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1 **Development and evaluation of a novel geopolymer based on**  
2 **basalt rock waste and ground granulated blast furnace slag**

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## Abstract

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Increase in industrial and construction activities has led to an enormous rise in waste generation and its hazardous impacts on the environment. Quarrying of rocks and manufacturing of artificial sands for civil engineering projects leads to the dumping of rock waste dust, which is a source of landfill problems. Further, excessive energy requirements for cement manufacturing, higher greenhouse gas emissions and rapid depletion of natural resources have focused the research towards the development of environment friendly and sustainable materials such as geopolymers. In this paper, a novel geopolymer has been developed from industrial wastes such as basalt rock fines considering partial replacement with ground granulated blast furnace slag (GGBFS) up to 30%. After a detailed mix-design investigation, the optimum molarity (M) of the sodium hydroxide solution was found to be 8M whereas the optimum ratio (R) of sodium silicate to sodium hydroxide solution as 0.75. Unconfined compressive strength (UCS) evaluation showed 7-day strengths up to 34 MPa, comparable to geopolymers based on conventional precursor materials. The scanning electron microscopy (SEM) imaging of the specimens revealed a dense geopolymer gel formation resulting in a homogeneous and compact microstructure. As a result, this innovative material produced can be used as an alternative, sustainable and cost-effective construction material.

**Keywords:** geopolymers, basalt rock waste, blast furnace slag, unconfined strength, microstructure

## 67 **1. Introduction**

68 Basalt rocks, having a high alumina and silica content, can be a potential raw material  
69 for the manufacturing of geopolymers (Davidovits, 1989; Duxson et al., 2007; Khale and  
70 Chaudhary, 2007; Lahoti et al., 2017). Large quantities of quarry fines generated as an  
71 industrial waste are being deposited in landfills which can cause serious environmental issues  
72 (Eliche-Quesada et al., 2020). A quarry on an average produces 20,000-30,000 tons per annum  
73 of rock fines, which are mostly dumped into landfill sites, thus contributing to environmental  
74 hazards. Currently, the waste by-product associated with rock quarrying activities of basalt  
75 rocks (basalt rock fines or dust) has limited application in the agricultural industry as a soil  
76 amendment agent (Nunes et al., 2014).

77 Geopolymers are synthesized as a result of activation of aluminosilicate source by a  
78 highly concentrated alkali hydroxide or silicate solution (Ahmad et al., 2021; Azevedo et al.,  
79 2020; Davidovits, 1989; Mathew and Issac, 2020; Serag Faried et al., 2020; Xie et al., 2020).  
80 The aluminosilicate compound goes through the dissolution phase responsible for  
81 transformation of aluminosilicate particles (Duxson et al., 2007; Rios et al., 2019; Wong et al.,  
82 2020; Xie and Fang, 2019). High pH levels of the activator solution (generally ranging from  
83 10 to 13), dissolve the amorphous aluminate and silicate species and result in a highly saturated  
84 aluminosilicate solution thus forming a gel-like structure. The large oligomer networks  
85 promote the formation of a bi-phasic hydrated gel. The linkages of the gel network enhance  
86 with time and the system hardens to form a three-dimensional aluminosilicate network  
87 generally known as geopolymer.

88 The increase in construction activities worldwide in the past few decades has resulted  
89 in an increasing demand for Ordinary Portland Cement (OPC). It has been estimated that the  
90 global production of cement in 2016 was nearly 4 billion tonnes (Andrew, 2018; Phummiphan

91 et al., 2018). While enhanced strength and reduced shrinkage are achieved for concrete  
92 applications using cement (e.g. better bonding of aggregates), its manufacturing process is  
93 highly energy intensive leading to excessive greenhouse gases emissions especially carbon  
94 dioxide (CO<sub>2</sub>) (Akbar et al., 2021; Duxson et al., 2007; Hassan et al., 2020; Hoppe Filho et al.,  
95 2021; Nawaz et al., 2020; Shobeiri et al., 2021; Sontia Metekong et al., 2020; Zhang et al.,  
96 2013). For instance, approximately, one tonne of cement produces around 900 kg of CO<sub>2-e</sub> and  
97 consumes about 5 Gigajoules of energy (Stafford et al., 2016). The need for alternative more  
98 sustainable cementitious binders, such as geopolymers has been receiving significant attention  
99 in the past decade. The geopolymer raw materials are often by-products from industrial  
100 processes (e.g. fly ash and blast furnace slag) and their usage helps to promote sustainable  
101 construction practices both financially (cost reduction up to 30%) and environmentally  
102 (greenhouse gas emission reduction up to 80%) (Erfanimanesh and Sharbatdar, 2020; Kolovos  
103 et al., 2013; Shobeiri et al., 2021; Zhang et al., 2013).

104 Past studies have focused on the development of geopolymers based on various natural  
105 materials (e.g. metakaolin, kaolin, rice husk ash, and bagasse) as well as industrial wastes (e.g.  
106 fly ash, blast furnace slag) in order to supplement the requirement for environment friendly  
107 construction materials (Arulrajah et al., 2017; Chindaprasirt et al., 2007; Hardjito et al., 2005;  
108 Izquierdo et al., 2009; Malkawi et al., 2020; Palomo et al., 1999; Ramlochan et al., 2000;  
109 Samarakoon et al., 2021). For instance, geopolymers derived from metakaolin, sedimentary  
110 rock powders, fly ash, blast furnace slag and other materials have shown comparable  
111 compressive strengths (Görhan and Kürklü, 2014; Lahoti et al., 2017; Nath and Kumar, 2019;  
112 Top and Vapur, 2018) as well as enhanced the chloride and sulphate resistance characteristics  
113 to that of conventional cementitious binders (Kwasny et al., 2018; Reddy et al., 2013; Sata et  
114 al., 2012; Sturm et al., 2018; Wasim et al., 2021).

115 In this study, a novel approach to produce a sustainable geopolymer based on two  
116 industrial wastes (basalt rock fines and ground granulated blast furnace slag) has been  
117 proposed. The main objectives of the research were to find optimum values of the synthesis  
118 parameters required in the formation of basalt fines and ground granulated blast furnace slag  
119 (GGBFS) geopolymer. In the first phase of testing, several combinations of synthesis  
120 parameters were used in order to prepare different mix designs of the basalt fines geopolymer.  
121 In the second phase, the ground granulated blast furnace slag (GGBFS) was used to partially  
122 replace the basalt rock fines by 10, 20 and 30% by mass. The optimum values of all the  
123 parameters were determined in order to achieve higher compressive strength and better  
124 workability characteristics. The possible application of the proposed geopolymer can result in  
125 substantial economic and environmental benefits as the source materials are waste by-products  
126 and their disposal currently involves stockpiling in landfills. The reprocessing and reuse of  
127 these waste materials to generate a cleaner value added product will contribute to enhanced  
128 sustainable construction practices, therefore making a step towards zero waste initiative.

129

## 130 **2. Experimental program**

### 131 *2.1 Raw materials*

132 The proposed geopolymer has four main materials as its components. The two precursor  
133 materials used were basalt rock fines and ground granulated blast furnace slag. The basalt rock  
134 fines were obtained from a quarry in the Illawarra region of New South Wales (Australia)  
135 whereas ground granulated blast furnace slag (GGBFS) was provided by Australasian Slag  
136 Association (ASA). Both are classified as industrial wastes and result from the manufacturing  
137 processes of sands and steel, respectively. The alkaline activator solution was prepared using  
138 98% pure sodium hydroxide pellets manufactured by Bondall (Australia) and D-grade sodium

139 silicate solution (specific gravity= 1.53 and SiO<sub>2</sub>/Na<sub>2</sub>O modulus ratio=2.0) which was supplied  
140 by PQ Corporation (Australia).

141

## 142 *2.2 Material characterisation*

143 To investigate the general geotechnical characteristics of basalt fines, several  
144 identification and characterisation tests such as particle size analysis, Atterberg limits analysis,  
145 specific gravity analysis and standard Proctor compaction tests were performed. The particle  
146 size analysis of the basalt fines was carried out using Mastersizer 3000 laser diffraction particle  
147 size analyser (manufactured by Malvern panalytical Ltd.) having a measurement range from  
148 0.01 to 3500 µm. Figure 1 shows the particle size distribution curve for the basalt fines. The  
149 D<sub>10</sub>, D<sub>30</sub> and D<sub>50</sub> are 3.5µm, 17.5µm and 40.5µm, respectively.

150 The compaction characteristics were determined using standard compaction tests in  
151 accordance with AS 1289.5.1.1-2017. The maximum dry unit weight and optimum moisture  
152 content obtained were 18.8 kN/m<sup>3</sup> and 13.5%, respectively as shown in Figure 2. The specific  
153 gravity of basalt fines (AS 1289.3.5.1-2006) was found to be 2.76. The basalt fines showed a  
154 liquid limit of 24.0%, plastic limit of 17.6% and a plasticity index of 6.4% (AS 1289.3.1.1-  
155 2009 and AS 1289.3.2.1-2009). Thus, the basalt fines may be classified as clayey silt or CL-  
156 ML (AS 1726-2017).

157 Scanning electron microscopy (SEM) technique was employed to investigate the  
158 microstructure of basalt fines using JEOL JSM-6490LV scanning electron microscope. Low  
159 vacuum environment was maintained in the chamber at a pressure of 40 Pa, accelerating  
160 voltage of 16 kV and the spot size was kept at 63 nm during the sample analysis. The images  
161 taken at different magnifications (x250, x500, x1000 and x2000) are shown in Figure 3. The  
162 scanning electron micrographs revealed that the basalt fines particles are sharp-edged having

163 irregular and angular surfaces that would assist in greater interlocking and thus denser  
164 geopolymer gel formation. A combination of different particle sizes is expected to assist in  
165 pore size reduction of geopolymer matrix. Indeed, the voids in the sample imaged under the  
166 microscope, were around 1 to 4  $\mu\text{m}$  and were likely to be reduced during the geopolymerisation  
167 process. The energy dispersive spectroscopy (EDS) analysis showed high Si and Al peaks, as  
168 highlighted in Figure 4, which was a confirmation of basalt fines being a suitable  
169 aluminosilicate source for geopolymerisation.

170 The chemical composition determined through X-ray fluorescence (XRF) analysis of  
171 the basalt rock fines and ground granulated blast furnace slag is presented in Table.1 which  
172 shows that the basalt rock fines are composed mainly of aluminosilicate compounds i.e. silica,  
173  $\text{SiO}_2 = 51.15\%$ , aluminium oxide,  $\text{Al}_2\text{O}_3 = 15.89\%$  and calcium oxide,  $\text{CaO} = 7\%$  by mass. In  
174 contrast, the ground granulated blast furnace slag composition showed a high percentage of  
175 calcium oxide,  $\text{CaO} = 42.71\%$  which indicates that GGBFS is highly reactive and can assist in  
176 the enhancement of calcium aluminate silicate hydrate (C-A-S-H) gel linkages, therefore  
177 contributing to the formation of a strong geopolymeric gel.

178

### 179 *2.3 Mix design ratios*

180 A total of 27 samples were prepared in the first phase of geopolymer mix preparation.  
181 Different combinations of varying parameters such as molarity (M) of sodium hydroxide  
182 solution, sodium silicate to sodium hydroxide ratio (R) and alkaline activator solution to basalt  
183 fines ratio were taken into consideration while preparing the test matrix as shown in Table 2.  
184 In the first phase of geopolymer mix preparation, the sodium hydroxide (NaOH) solutions were  
185 prepared at different molar concentrations (M) such as 4M, 8M and 12M to investigate the  
186 effect of concentration of alkali activator. Several past studies have reported using sodium



187 hydroxide solutions with molarities ranging from 2M to 14M for fly-ash and slag based  
188 geopolymers (Görhan and Kürklü, 2014; Jafari Nadoushan and Ramezani pour, 2016; Lahoti  
189 et al., 2017; Laskar and Talukdar, 2017; Lee et al., 2019; Nath and Kumar, 2019; Reddy et al.,  
190 2013; Williamson and Juenger, 2016). In this study, 4, 8 and 12M solutions were selected  
191 because they cover a wide range of molar concentrations for which maximum efficiency in  
192 geopolymerization process and larger strength gains are achieved.

193 To evaluate the role of the sodium silicate ( $\text{Na}_2\text{SiO}_3$ ) to sodium hydroxide (NaOH)  
194 solutions weighted ratios (R) in the geopolymer strength, specimens having different ratios (i.e.  
195 0.25, 0.5 and 1.0) were tested. The sodium hydroxide solution was kept at ambient conditions  
196 for 24 hours to achieve equilibrium (Lahoti et al., 2018; Mehta and Siddique, 2017). The alkali  
197 activator solution to basalt fines ratio was kept constant at 0.135. This value was selected  
198 because it corresponds to the optimum moisture content for basalt fines, whereas the quantity  
199 of binder solids was maintained as per the maximum dry unit weight. A summary of the tests  
200 conducted in the first phase is shown in Table 2.

201 The main objective was to determine the optimum synthesis parameters for the  
202 preparation of basalt fines geopolymer. As the unconfined compressive strength obtained for  
203 the specimens prepared in the first phase was smaller (i.e. around 2 MPa) than more established  
204 geopolymers, partial replacement with a richer aluminosilicate source (e.g. ground granulated  
205 blast furnace slag) was considered. In the second phase, the basalt fines were partially replaced  
206 at 10, 20 and 30% by mass with ground granulated blast furnace slag and 81 specimens were  
207 prepared as shown in Table 3.

208

209

210

211 *2.4 Specimen preparation and curing*

212 In the first phase, the sodium silicate and sodium hydroxide solutions were mixed in a  
213 glass beaker at specific ratios (R), i.e. 0.25, 0.5 and 1.0 for about 30 to 40 minutes. The solution  
214 prepared was then added to the basalt fines at the ratios mentioned in Table 2 and mixed for 5  
215 to 7 minutes to ensure homogeneous mixing. Cylindrical specimens, 38 mm in diameter and  
216 76 mm in height were prepared in steel moulds and compacted statically in 3 layers using a  
217 compression frame as per the max dry density i.e.  $18.8\text{kN/m}^3$ . Triplicate specimens were  
218 prepared for each specific ratio to ensure repeatability of results.

219 Once compacted, the specimens prepared were then wrapped in polyethylene films,  
220 sealed in polyethylene resealable bags, and cured at ambient conditions for the required  
221 duration under relatively constant temperature (approximately  $22^\circ\text{C}$ ) and humidity  
222 (approximately 95%). Similar procedure was carried out for the specimen preparation in the  
223 second phase as per the mix design ratios mentioned in Table 3.

224

225 *2.5 Unconfined compressive strength tests*

226 The basalt fines geopolymer specimens prepared in the first phase were tested for  
227 unconfined compressive strength at 7 days. The tests were carried out in accordance with AS  
228 5101.4-2008 using Instron 30 kN universal testing machine adopting a strain rate of 0.5  
229 mm/min. In the second phase, when the basalt rock fines were partially replaced with ground  
230 granulated blast furnace slag (GGBFS), the tests were carried out using 500 kN Instron  
231 universal testing machine adopting the same strain rate to avoid any rate dependency effects.  
232 The measurement accuracy of the load cell and deformation transducers of both testing systems  
233 used was 0.01 N and 0.01  $\mu\text{m}$ , respectively.

234

### 235 **3. Results and discussion**

#### 236 *3.1 Effect of molar concentration of alkali activator solution*

237           The alkaline activator concentration plays a major role in the reactivity, pore  
238 microstructure, aluminosilicate gel matrix formation as well as other mechanical properties of  
239 geopolymers (Ma et al., 2012; Rashad and Zeedan, 2011; Ruiz-Santaquiteria et al., 2012). The  
240 effect for the molar concentration (M) of the sodium hydroxide solution was investigated for  
241 the basalt fines geopolymer as well as for the basalt and ground granulated blast furnace slag  
242 geopolymer mixes. From the findings of the initial trial mixes, it was observed that the  
243 unconfined compressive strengths (UCS) of the basalt fines geopolymer increased with  
244 increase in molarity (M) of sodium hydroxide activator from 4M to 8M, for all sodium silicate  
245 to sodium hydroxide ratios (R). For instance, for the specimens with sodium silicate to sodium  
246 hydroxide ratio (R) of 0.25, the UCS values increased from 0.81 MPa to 1.25 MPa as the  
247 molarity of alkali activation solution was increased from 4M to 8M. However, a drop in UCS  
248 value to 1.05 MPa was observed when the molarity was increased further to 12M as shown in  
249 Figure 5. While these results may not be intuitive at first sight, they are consistent with the  
250 results reported in past studies for other geopolymer sources, in which a decrease in  
251 compressive strengths beyond optimum value of alkaline concentration was attributed to  
252 increased viscosity of activator solution and the presence of unreacted silica and alumina  
253 (Barbosa et al., 2000; Hardjito et al., 2008; Jafari Nadoushan and Ramezani-pour, 2016;  
254 Williamson and Juenger, 2016).

255           This is likely associated with the fact that higher molarity favours better reactivity with  
256 the aluminosilicate source but only up to an optimum value of 8M. This indicates that the  
257 aluminosilicate content would exhibit a higher extent of dissolution to complete the process of  
258 geopolymerisation. Once an optimum value is exceeded, the precipitation of dissolved species

259 inhibits geopolymerisation. For the second phase, the basalt fines and GGBFS geopolymer  
260 mixes were tested with 8M and 12M concentrations of the sodium hydroxide solution as 4M  
261 concentration was not found sufficient to facilitate the dissolution process.

262 Figure 6 (a) shows the change in peak compressive stresses of the basalt rock waste  
263 and ground granulated blast furnace slag geopolymer prepared using 8M and 12M NaOH  
264 concentrations,  $\text{Na}_2\text{SiO}_3/\text{NaOH}$  ratio of 0.25 and different slag percentages. It can be seen that  
265 as the molarity of NaOH solution increased from 8M to 12M, the compressive strength of the  
266 geopolymer specimens decreased. For instance, the compressive strength of 8M-0.25R-  
267 10%BFS sample was 11.1 MPa and dropped to 7.9 MPa for 12M-0.25R-10%BFS. Similar  
268 trend was observed for all the specimens prepared with varying slag content. This is due to the  
269 reason that molarities higher than the optimum value can hinder the complete geopolymeric  
270 gel formation. (Görhan and Kürklü, 2014; Jafari Nadoushan and Ramezaniyanpour, 2016;  
271 Williamson and Juenger, 2016).

272 Figure 6 (b) shows the normalised compressive stress-axial strain behaviour of basalt  
273 rock waste and ground granulated blast furnace slag geopolymer prepared using 8M and 12M  
274 NaOH concentrations,  $\text{Na}_2\text{SiO}_3/\text{NaOH}$  ratio of 0.25 with varying slag content. It can be seen  
275 that the 8M specimens exhibited higher strains at peak axial stresses and the values dropped as  
276 the molarity of NaOH solution increased to 12M. Further, the 8M specimens for all the varying  
277 slag percentages failed in a brittle manner showing a vertical drop in the post-peak curve,  
278 whereas the 12M specimens revealed a more ductile behaviour. This could be due to the  
279 formation of a monolithic geopolymer gel related to the complete geopolymerisation of the  
280 aluminosilicate source at 8M concentration of sodium hydroxide solution. As the material  
281 became stiffer at 8M concentrations, it tended to be more brittle whereas the incomplete  
282 reactions and free silica in the system at 12M concentration samples resulted in a more ductile

283 behaviour. Furthermore, 12M samples were found to set quicker probably due to accelerated  
284 geopolymerisation reactions, which hindered the compaction process and workability.

285 The ductility ( $\mu$ ) of geopolymer specimens was calculated using Eq. (1) shown below.

$$286 \quad \mu = \frac{\varepsilon_2}{\varepsilon_1} \quad (1)$$

287 The same method has been used for analysing the ductility of geopolymer concrete by various  
288 researchers (Ali et al., 2020; Reed et al., 2014). The strain  $\varepsilon_1$  represents the value when a  
289 tangent to the ascending branch of the stress-strain curve touches the horizontal line at the peak  
290 stress and  $\varepsilon_2$  shows the strain level at 85% of peak stress value on the descending branch of the  
291 curve as shown in Figure 7 (a). As the molarity of specimens increased from 8M to 12M, due  
292 to inhibited geopolymerisation process, the gel formed was weaker and thus the ductility of the  
293 samples increased. For instance, the ductility of 12M-0.25R-10%BFS was increased by almost  
294 29% as compared to 8M-0.25R-10%BFS. Similar trends were witnessed for all specimens  
295 having different slag percentages as shown in Figure 7 (b).

296 Based on the results reported herein, the optimal molarity of sodium hydroxide solution  
297 (M) for the basalt fines geopolymer as well as basalt fines and ground granulated blast furnace  
298 slag geopolymer was found to be 8.0 M.

299

### 300 *3.2 Effect of sodium silicate to sodium hydroxide ratio*

301 The role of sodium silicate to sodium hydroxide ratios (R) was studied for both basalt  
302 fines as well as the basalt and blast furnace slag based geopolymers. From the initial mixes, it  
303 was found that the UCS values increased with the increase in the sodium silicate to sodium  
304 hydroxide ratios (R) from 0.25 to 1.0R. For instance, for 8M solution, the UCS values were  
305 found to be 1.25 MPa at 0.25R, then increased to 1.65 MPa at 0.5R and further increased to

306 around 2.25 MPa for 1.0R, as shown in Figure 8. However, for the mixes with R=1.0, a reduced  
307 workability was detected due to accelerated reactions, thus making the mixtures difficult to  
308 compact. Figure 9 (a), 9 (b) and 9 (c) illustrate the stress-strain curves for basalt fines and blast  
309 furnace slag geopolymer with 8M NaOH concentration and Na<sub>2</sub>SiO<sub>3</sub>/NaOH ratio (R) of 0.25,  
310 0.5 and 0.75 respectively.

311 It was observed that the unconfined compressive strength values increased with the  
312 increase of these ratios for 8M concentrations of sodium hydroxide solutions as well as for the  
313 samples with 12M concentration of sodium hydroxide shown in Figure 10 (a), 10 (b) and 10  
314 (c). For instance, the peak axial stress value for 8M-0.25R-30% BFS specimens was 21 MPa,  
315 increased to 32 MPa for 0.5R and further increased to 34 MPa for 0.75R ratios. The peak axial  
316 stress value for 12M-0.25R-10% BFS samples was about 7 MPa, increased to 8.2 MPa for 0.5R  
317 and further increased to 9.4 MPa for 0.75R ratios. The peak axial stress value for 12M-0.25R-  
318 20% BFS samples was 12.5 MPa, increased to 15.1 MPa for 0.5R and further reached 19.4  
319 MPa against 0.75R. The peak axial stress value for 12M-0.25R-30% BFS samples was 17.5  
320 MPa, increased to 18.7 MPa for 0.5R and further reached 20.4 MPa against 0.75R. The elastic  
321 moduli of all the samples increased with the increasing sodium silicate to sodium hydroxide  
322 ratios.

323 This trend is consistent with the results reported for geopolymers developed from other  
324 established source materials where the increase in R values have contributed to increase in  
325 compressive strengths (Paija et al., 2020). However, it was also observed that with the increase  
326 in R values, the workability for compaction of samples decreased. The samples with the R  
327 value 0.25 were relatively easier to compact. As the values increased to 0.5 and then 0.75, the  
328 rate of geopolymerisation was accelerated due to higher content of sodium silicate solution  
329 causing earlier setting of the mix, thus making it difficult to compact.

330           Based on the results reported herein, the optimal ratio of sodium silicate to sodium  
331 hydroxide solution (R) for the basalt fines geopolymer as well as basalt fines and ground  
332 granulated blast furnace slag geopolymer was found to be 0.75.

333

### 334 *3.3 Effect of silicon to aluminium (Si/Al) ratio*

335           Along with other synthesis parameters, the silicon to aluminium (Si/Al) molar ratio of  
336 the geopolymer influences the mechanical strength characteristics of geopolymeric materials.  
337 To evaluate the optimum ratio for the aluminosilicate sources considered in this study, i.e. the  
338 basalt fines and ground granulated blast furnace slag, geopolymer samples were also prepared  
339 in different Si/Al ratios (2.81, 2.91 and 3.10) and their 7-day unconfined compressive strengths  
340 were investigated. Figure 11 shows that the peak axial strength values decreased with  
341 increasing Si/Al ratios (21 MPa for 2.81, 19MPa for 2.91 and 17 MPa for 3.10). This is likely  
342 caused by the higher quantity of unreacted silica present in the system. Another contributing  
343 factor could be increased viscosity of the alkali solution contributing to inhomogeneous mixing  
344 and a more porous structure.

345           In the past, optimum ratios have been determined for geopolymers based on various  
346 natural and industrial waste sources (He et al., 2016; Lahoti et al., 2018; Lizcano et al., 2012;  
347 Timakul et al., 2015). The optimum Si/Al ratio investigation for metakaolin and fly ash based  
348 geopolymers (Asif et al., 2015; Lizcano et al., 2012; Thokchom et al., 2012; Timakul et al.,  
349 2015) revealed that as the Si/Al ratios tend to reach closer to 3.0, the strength values for that  
350 geopolymer mix would be lowered as reported in the past literature and shown in Table 4. The  
351 novel basalt and slag based geopolymer proposed in this study follows a similar principle of  
352 currently existing metakaolin, fly-ash and slag based geopolymers thus confirming its potential  
353 usage as an alternative cementitious material to be considered in construction industry.

### 354 3.4 Effect of ground granulated blast furnace slag (GGBFS)

355 The strength gain in many geopolymer materials is occasionally slow due to lack of  
356 calcium content in various precursors, which may result in a larger setting time (Cho et al.,  
357 2017). In such conditions, addition of high calcium additives such as ground granulated blast  
358 furnace slag can assist in accelerating the chemical reactions and facilitate the attainment of  
359 high early strength (Chindaprasirt et al., 2011; Deb et al., 2014). The basalt content in the  
360 geopolymer precursor mix was partially replaced by 10, 20 and 30% by mass to analyse the  
361 compressive strength behaviour. Figure 12 shows the stress strain behaviour of basalt  
362 geopolymer prepared at 8M NaOH concentration and  $\text{Na}_2\text{SiO}_3/\text{NaOH}$  ratio of 0.75 with  
363 varying levels of blast furnace slag. As expected, the peak axial stresses increase for higher  
364 content of blast furnace slag i.e. 11 MPa, 28 MPa and 34 MPa for 10, 20% and 30%  
365 replacement, respectively. The Young's moduli also increased with the increase in percentage  
366 of ground granulated blast furnace slag. The values increased with higher content of blast  
367 furnace slag i.e. 2.65 GPa, 3.47 GPa and 4.30 GPa for 10, 20% and 30% replacement,  
368 respectively.

369 Figure 13 shows a comparison chart of compressive strength of basalt fines geopolymer  
370 with 8M NaOH concentration and various  $\text{Na}_2\text{SiO}_3/\text{NaOH}$  ratios at different percentages of  
371 GGBFS. As expected, an increase in strength was observed in the specimens having higher  
372 percentage of GGBFS. For instance, for all the different R ratios considered, specimens having  
373 a larger GGBFS replacement percentage exhibited larger compressive strengths, often resulting  
374 in an increase of over 15MPa (e.g. 8M – 0.5R and 8M – 0.75R) for an increase from 10% to  
375 30% replacement of GGBFS. This is likely associated with the accelerated geopolymerisation  
376 reactions which are facilitated by calcium content in the system derived from higher percentage  
377 of GGBFS. This in turn led to the formation of a denser, monolithic and a less porous  
378 microstructure geopolymer which achieved high early compressive strengths. These results



379 show that the proposed geopolymer could prove to be a promising alternative to already  
380 established geopolymers while adding value to the product that is being currently landfilled.  
381 However, some workability issues could arise due to the decrease in setting time, which may  
382 require some further research.

383         Considering the strength, workability and microstructure characteristics reported in  
384 previous sections, the minimum GGBFS replacement percentage required to achieve  
385 comparable performance to more established geopolymers is 30%. However, it should be noted  
386 that smaller replacement percentages may be considered for applications where compressive  
387 strengths exceeding 30 MPa are not required.

388

### 389 *3.5 Effect of aging*

390         Figure 14 shows the stress-strain behaviour for basalt fines and 30 % blast furnace slag  
391 geopolymer with respect to curing time or aging. As expected, larger peak axial stresses and  
392 Young's moduli were achieved for specimens having more curing time, however the rate of  
393 change varied considerably in the first week (Figure 15). For instance, the peak axial stresses  
394 increased from 15.5 MPa (1 day) to 34.2 MPa (7 days) whereas the Young's moduli increased  
395 from 1.70 GPa (1 day) to 5.60 GPa (7 days) during the first week. After the first 7 days, the  
396 increase in the peak axial stresses was slower. The peak axial stress at 28 days was 42 MPa  
397 while the Young's modulus rose to 6.08 GPa. The variation in compressive strengths of basalt  
398 fines and 30% blast furnace slag geopolymer with respect to time is shown in Figure 15.

399         The gain in strength is drastic in the first 7 days and reaches 34.2 MPa at an average of  
400 4.88 MPa per day. This is possibly due to the geopolymeric chains being formed in the early  
401 curing after which the material hardens and the rate of strength gain decreases to an average of  
402 0.38 MPa per day reaching a 28 day strength of 42 MPa. This rapid gain in strength in the first

403 week allows the basalt and slag based geopolymer to be used as a promising material where  
404 high early compressive strengths are desired. Typically, the strength gain of geopolymers  
405 occurs at a much lower rate after 7 days (Deb et al., 2014; Khale and Chaudhary, 2007). This  
406 behaviour was confirmed by the scanning electron microscopy (SEM) images, which showed  
407 that the microstructure of basalt fines and blast furnace slag geopolymer in the 7 day and 28  
408 day exhibited similarity.

409

### 410 *3.6 Microstructure analysis*

411 The scanning electron microscopy (SEM) micrographs were taken for both basalt fines  
412 as well as basalt fines-blast furnace slag geopolymers. Figure 16 shows the SEM micrographs  
413 for basalt fines geopolymer at different molar concentrations of sodium hydroxide activator  
414 solution. It can be observed that as the molarity of the alkaline solution increased from 4M to  
415 12M, it enhanced the dissolution of silica particles in the gel matrix, thus reducing the amount  
416 of free silica due to consumption in the geopolymerisation reaction. As a result, the gel becomes  
417 denser and the material exhibits higher compressive strength.

418 The SEM micrographs for basalt fines geopolymer with partial replacement of basalt  
419 dust with 10, 20 and 30% of ground granulated blast furnace slag are illustrated in Figure 17.  
420 It is evident from the images that as the GGBFS content increases from 10 to 30%, the larger  
421 amount of reactive calcium in the system results in a denser microstructure. The gel formed is  
422 monolithic in nature and the pore size is estimated to be reduced to around 3 $\mu$ m as compared  
423 to the 9 $\mu$ m exhibited in the original basalt fines microstructure. Figure 18 shows the SEM  
424 micrographs of basalt fines and 30% blast furnace slag geopolymer (8M-0.75R-30%BFS in  
425 particular) at 7 days and 28 days, respectively. It is observed that a well compacted and dense  
426 gel formation occurs in the first seven days of geopolymerisation when the material has

427 undergone most of the chemical reactions and entered into its hardening stage. After the first  
428 week, the geopolymer gel gains strength at a much lower rate. This phenomenon is confirmed  
429 from the SEM images at 28 days of formation, as significant difference in the geopolymer gel  
430 is not observed also at the microstructure level. The microstructural characteristics of this new  
431 basalt and ground granulated blast furnace slag based geopolymer substantiate that the  
432 geopolymeric gel development mechanism and behaviour of this material is comparable to  
433 already established geopolymers from other precursor sources (Duxson et al., 2007; Izquierdo  
434 et al., 2009; Ma et al., 2013; Schmücker and MacKenzie, 2005) thus validating its potential  
435 application as a supplementary cementitious material.

436

#### 437 **4. Cost Analysis**

438 One of the major factors contributing to the usage of any material in the construction  
439 industry is its cost. In this study, quotations from different supplier companies such as PQ  
440 Corporation Australia, Bondall Australia, Boral Australia and Australasian Slag Association  
441 were obtained in order to carry out cost analysis. The average cost of some conventional  
442 precursor materials used in the synthesis of geopolymers are for instance, fly-ash costs AU\$  
443 600/ton, metakaolin around AU\$ 550/ton and GGBFS around AU\$ 80/ton. The alkaline  
444 activator like sodium hydroxide solution can cost around AU\$ 9000/ton whereas sodium  
445 silicate solution may cost around AU\$ 6500/ton. Using basalt dust waste and ground granulated  
446 blast furnace slag (GGBFS) geopolymer in the mix proportions suggested above, the per cubic  
447 meter cost will range from AU\$ 1000 to AU\$ 1200 thus reducing the cost by up to 50% as  
448 compared to conventional fly-ash based geopolymers. It should be noted that the cost exercise  
449 presented here is an example and assumes the raw materials are readily available in the vicinity  
450 of where the geopolymer is being produced. In contrast, in more remote locations where

451 quarried fines waste may not be available, the cost of transport may tip decision towards the  
452 use of more established precursors materials. Thus, a detailed analysis should be undertaken  
453 on a project to project basis.

454

## 455 **5. Conclusions**

456 In this study, an attempt was made to utilise basalt rock fines currently categorised as  
457 landfill wastes from quarry sites in combination with steel industry waste by-product, i.e.,  
458 ground granulated blast furnace slag to develop a series of new geopolymer mixes suitable for  
459 usage as an alternative, environment-friendly construction material. The main conclusions that  
460 can be drawn from this study are summarised as follows.

- 461 1. While the geopolymer based on basalt fines alone has shown potential, the compressive  
462 strengths obtained were small (around 2 MPa) compared to other conventional  
463 geopolymers developed from metakaolin, fly-ash etc. (30-50 MPa).
- 464 2. An addition of a richer aluminosilicate source, i.e. partial replacement up to weight  
465 ratios of 30% of ground granulated blast furnace slag (GGBFS), has enhanced the  
466 mechanical properties of the geopolymer mixes obtained and unconfined compressive  
467 strengths (UCS) up to 34 MPa were achieved with varying GGBFS ratios.
- 468 3. The optimum molarity (M) of the sodium hydroxide activator solution was found to be  
469 8M whereas the optimum weighted ratio of the sodium silicate to sodium hydroxide  
470 solution (R) was found to be 0.75. The experimental results revealed that the  
471 compressive strength values would increase even at higher ratios but at the cost of  
472 workability of the geopolymer mix. At values more than 0.75, the chemical reactions  
473 are accelerated which result in the decrease in setting time of the mix, thus making the  
474 samples difficult to compact. Sodium silicate is one of the major components of

475 geopolymers contributing towards strength; however, it also significantly results in  
476 excessive greenhouse gas emissions. Although higher ratios (approximately 2.5) have  
477 been utilised in geopolymer concrete production in literature, the lower ratios in this  
478 innovative basalt and slag based geopolymer mix can prove to reduce the carbon  
479 footprint as well as the production cost.

480 4. The basalt and GGBFS geopolymer prepared showed 7 days compressive strengths up  
481 to 34.2 MPa and exhibited a slower strength gain up to 28 days curing  
482 (i.e. maximum peak strength of 42 MPa). The SEM micrographs revealed that the  
483 microstructure developed in the first week was highly monolithic, dense with a reduced  
484 pore size, therefore contributing to high early strengths.

485 5. The basalt and GGBFS geopolymer prepared shows a reduction in cost by up to 50%  
486 and may prove to be a cost effective alternative to alkali-activated geopolymers  
487 prepared from other conventional precursors such as metakaolin and fly ash.

488

489 In light of the above, the proposed basalt rock fines and GGBFS based geopolymer can  
490 prove to be a value added product having a potential usage as an alternative cementitious  
491 material in the construction industry. The utilisation of landfill and industrial waste  
492 products such as the ones used in this research can act as a major step towards greener  
493 construction practices.

494

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502

### 503 **CRedit Author statement**

504 The paper is a joint contribution of all authors but specific contributions can be recognised as  
505 follows,

506 **Mohsin Nawaz:** Methodology, Laboratory experiments, Writing - Original Draft,

507 **Ana Heitor:** Conceptualization, Writing - Review & Editing, Resources, Supervision, Project  
508 administration

509 **Muttucumaru Sivakumar:** Conceptualization, Writing - Review & Editing, Resources,  
510 Supervision

511

### 512 **Declaration of Interest Statement**

513 The authors declare that they have no known competing financial interests or personal  
514 relationships that could have appeared to influence the work reported in this paper.

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738 Table. 1 Chemical composition of basalt rock fines and ground granulated blast furnace slag

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<b>Component</b>	<b>Basalt fines (mass %)</b>	<b>GGBFS (mass %)</b>
SiO <sub>2</sub>	51.15	34.46
Al <sub>2</sub> O <sub>3</sub>	15.89	12.78
CaO	7.00	42.71
Fe <sub>2</sub> O <sub>3</sub>	8.37	0.39
Na <sub>2</sub> O	3.36	0.75
MgO	2.79	5.32
P <sub>2</sub> O <sub>5</sub>	0.73	0.08
SO <sub>3</sub>	0.11	1.68
K <sub>2</sub> O	3.71	0.27
Mn <sub>2</sub> O <sub>3</sub>	0.18	0.39
TiO <sub>2</sub>	1.00	0.82
Loss on Ignition (LOI)	5.42	0.14

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Table 2. Summary of test matrix for phase-I

<b>Molarity of NaOH (M)</b>	<b>(NaOH+Na<sub>2</sub>SiO<sub>3</sub>)/Basalt (by weight)</b>	<b>Na<sub>2</sub>SiO<sub>3</sub>/NaOH (R) (by weight)</b>	<b>Number of specimens</b>
4M	0.135	0.25	3
	0.135	0.5	3
	0.135	1.0	3
8M	0.135	0.25	3
	0.135	0.5	3
	0.135	1.0	3
12M	0.135	0.25	3
	0.135	0.5	3
	0.135	1.0	3
			<b>Total: 27</b>

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Table 3. Summary of test matrix for phase-II

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Molarity of NaOH (M)	Na <sub>2</sub> SiO <sub>3</sub> /NaOH (R) by mass	GGBFS percentage (%) by mass	No. of Specimens
4	0.25	10	3
		20	3
		30	3
	0.5	10	3
		20	3
		30	3
	0.75	10	3
		20	3
		30	3
8	0.25	10	3
		20	3
		30	3
	0.5	10	3
		20	3
		30	3
	0.75	10	3
		20	3
		30	3
12	0.25	10	3
		20	3
		30	3
	0.5	10	3
		20	3
		30	3
	0.75	10	3
		20	3
		30	3
			<b>Total: 81</b>

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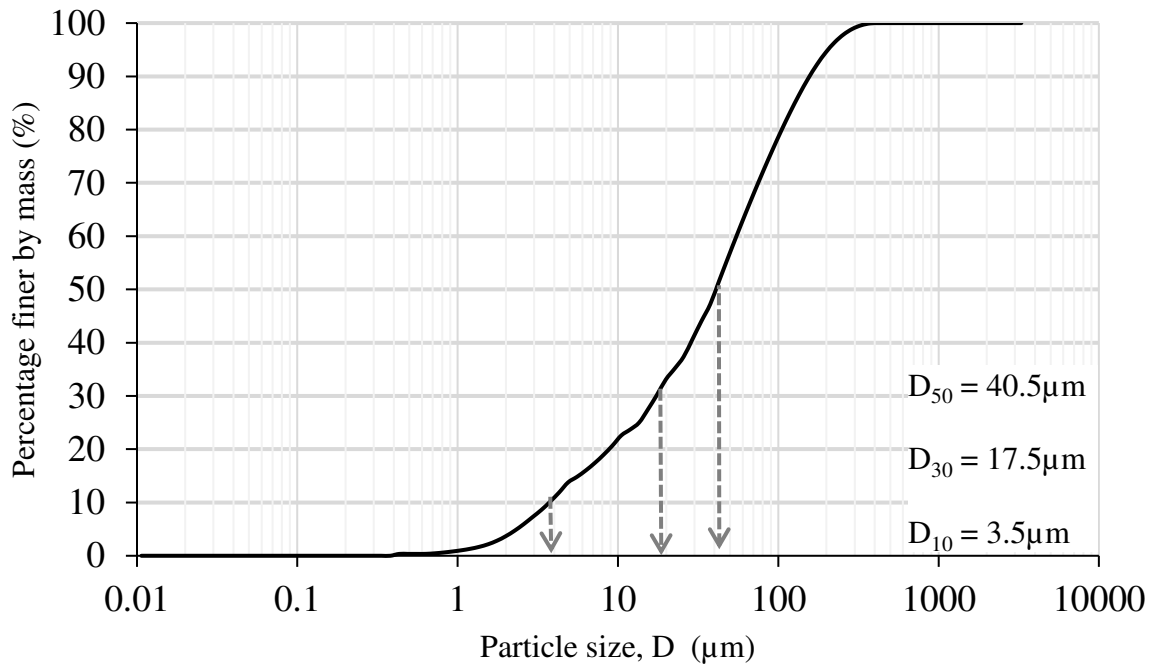
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Table 4. Comparison table for effect of Si/Al ratio on strength of geopolymers in past literature

<b>Authors</b>	<b>Si/Al ratios</b>	<b>Comments</b>
Asif et al. (2015)	1.85, 2.0, 2.5 and 3.0	The compressive strengths increased till a ratio of 2.0 after which they decreased
Timakul et al. (2015)	2.6, 2.65, 2.8 and 3.0	The compressive strengths increased till a ratio of 2.65 after which a decrease was witnessed
He et al. (2013)	1.68, 2.24, 2.80 and 3.35	The compressive strengths increased till a ratio of 2.80 after which the values lowered
Lizcano et al. (2012)	1.25, 1.5, 2.0 and 2.5	The compressive strengths increased till a ratio of 1.5 after which they decreased
Zhang et al. (2011)	1.89 to 7.78	The compressive strengths were highest for a ratio of 1.89 after which they started to decrease

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Figure 1 Particle size distribution of basalt fines

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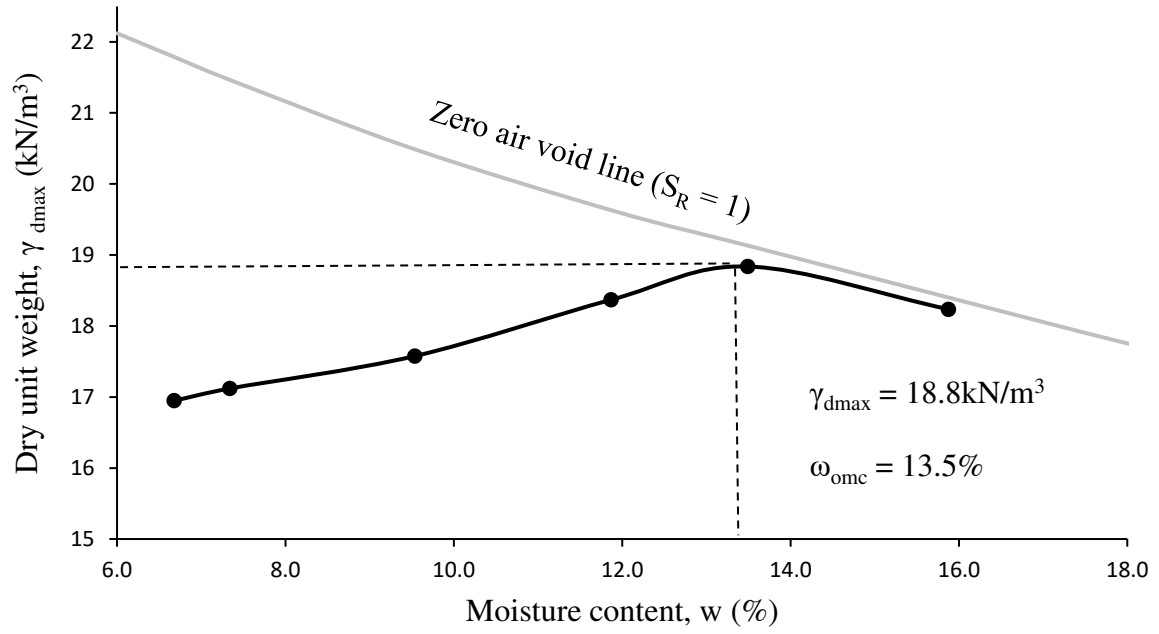
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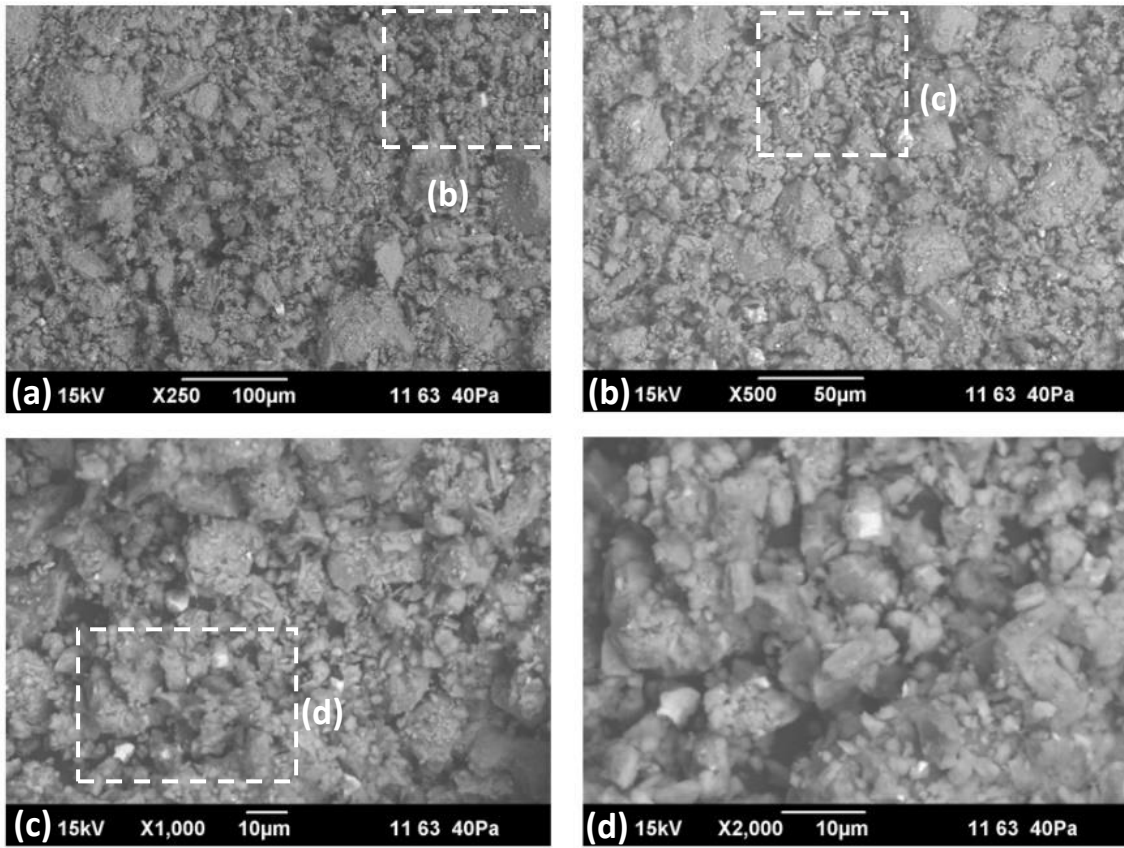
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Figure 2 Compaction curve for basalt rock fines



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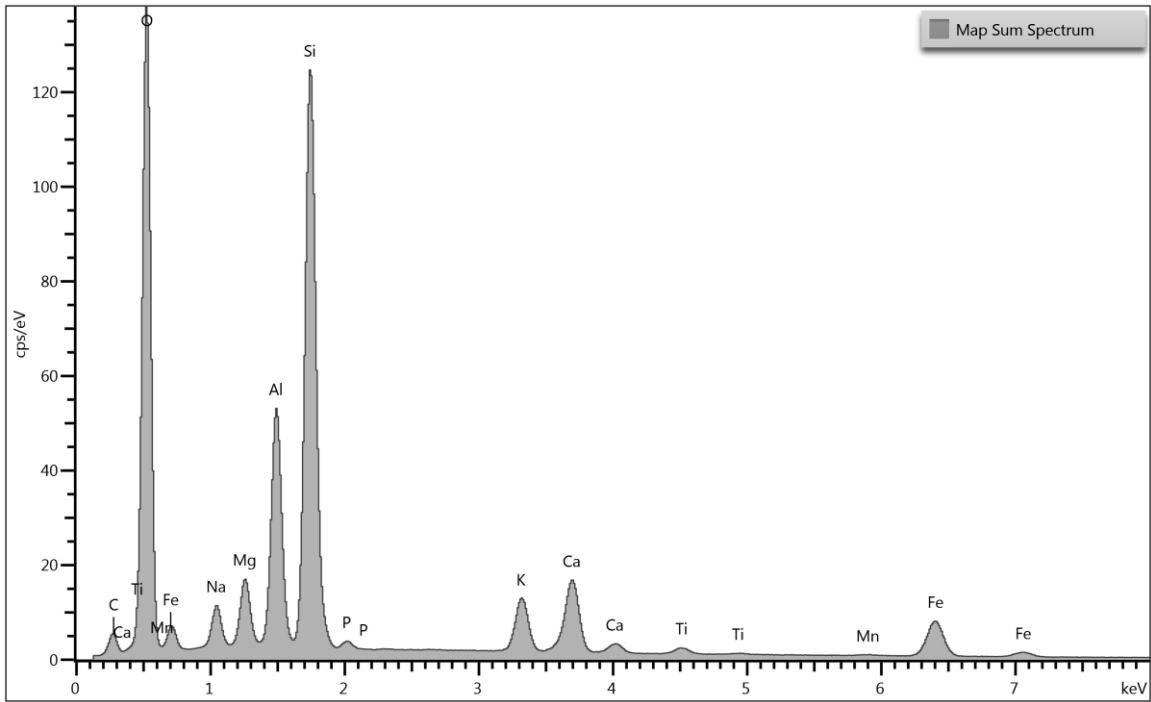
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Figure 3 SEM images of basalt fines at various magnifications (a) x250 (b) x500 (c) x1000 and (d) x2000 (Micrographs taken by Mohsin Nawaz) (squares represent the magnified regions)



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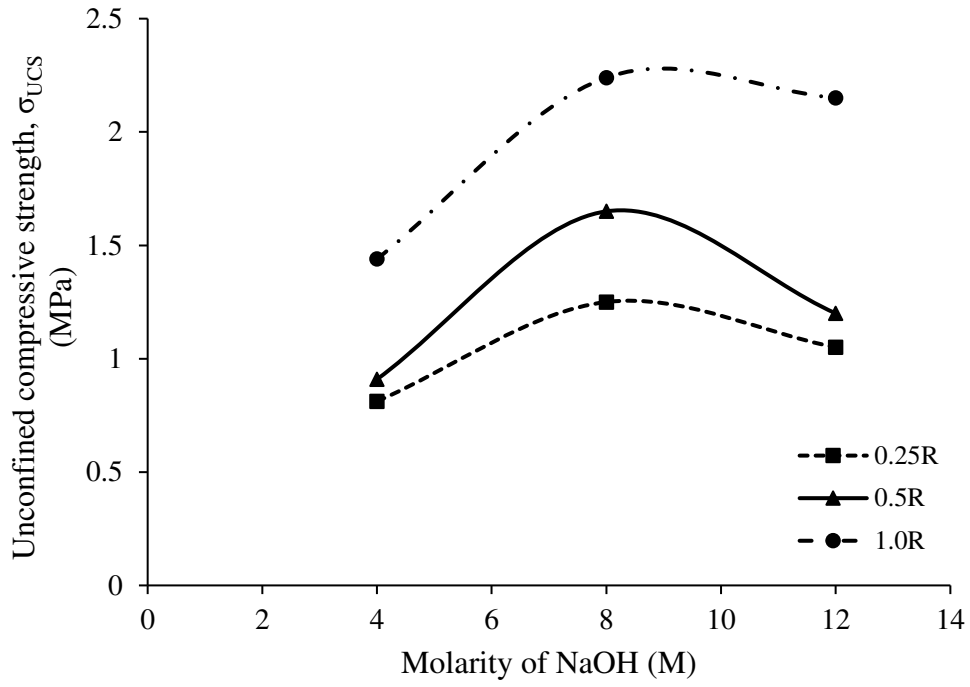
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868 Figure 5 Compressive strength variation for basalt fines geopolymer w.r.t. molarity (M) of  
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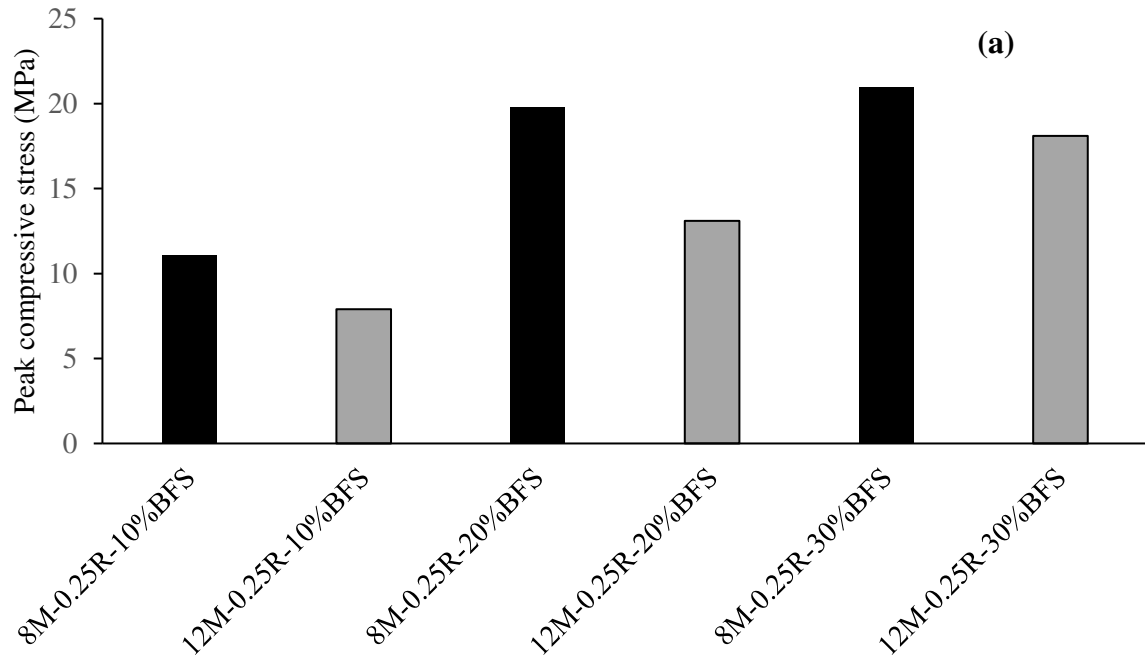
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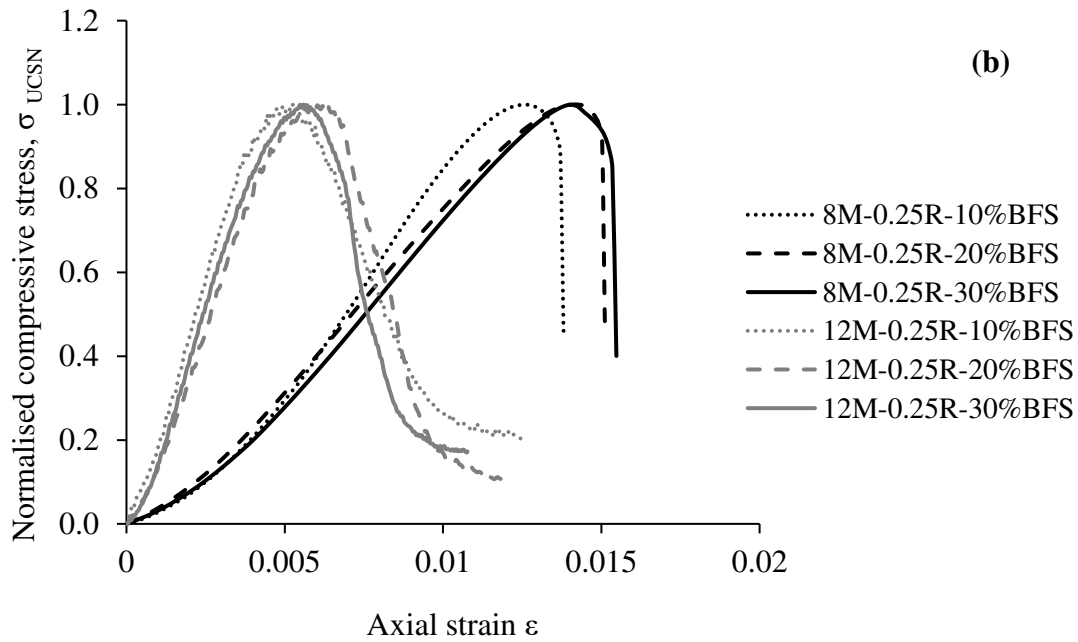
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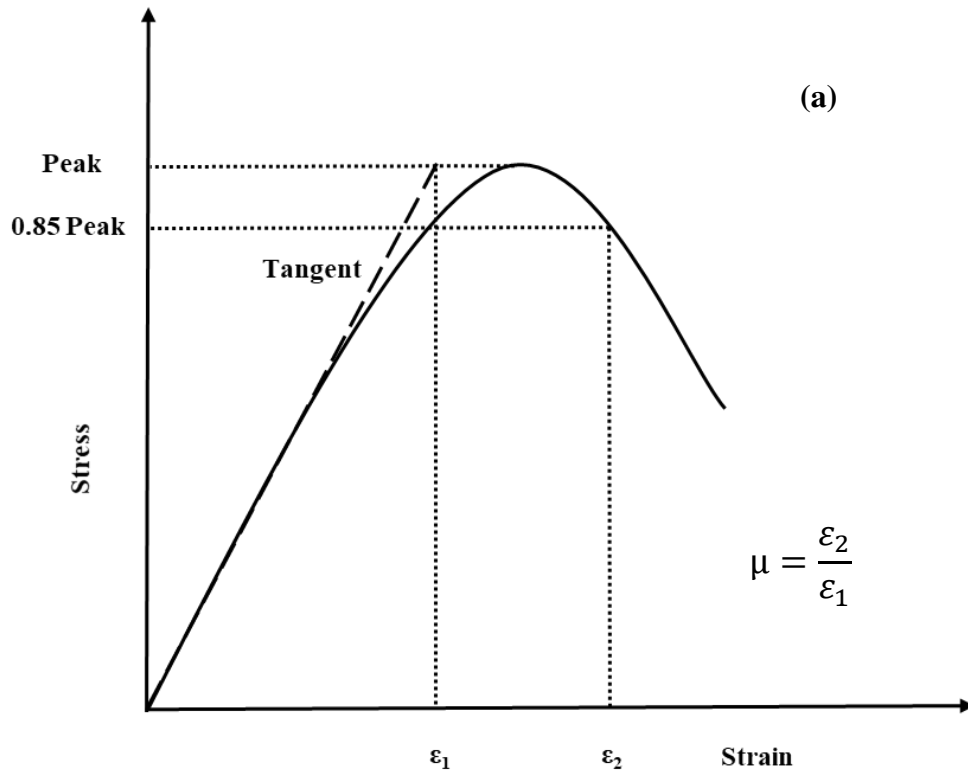
889 Figure 6 (a) Peak compressive stress of 8M and 12M geopolymer samples with varying slag  
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 891 samples with varying slag content

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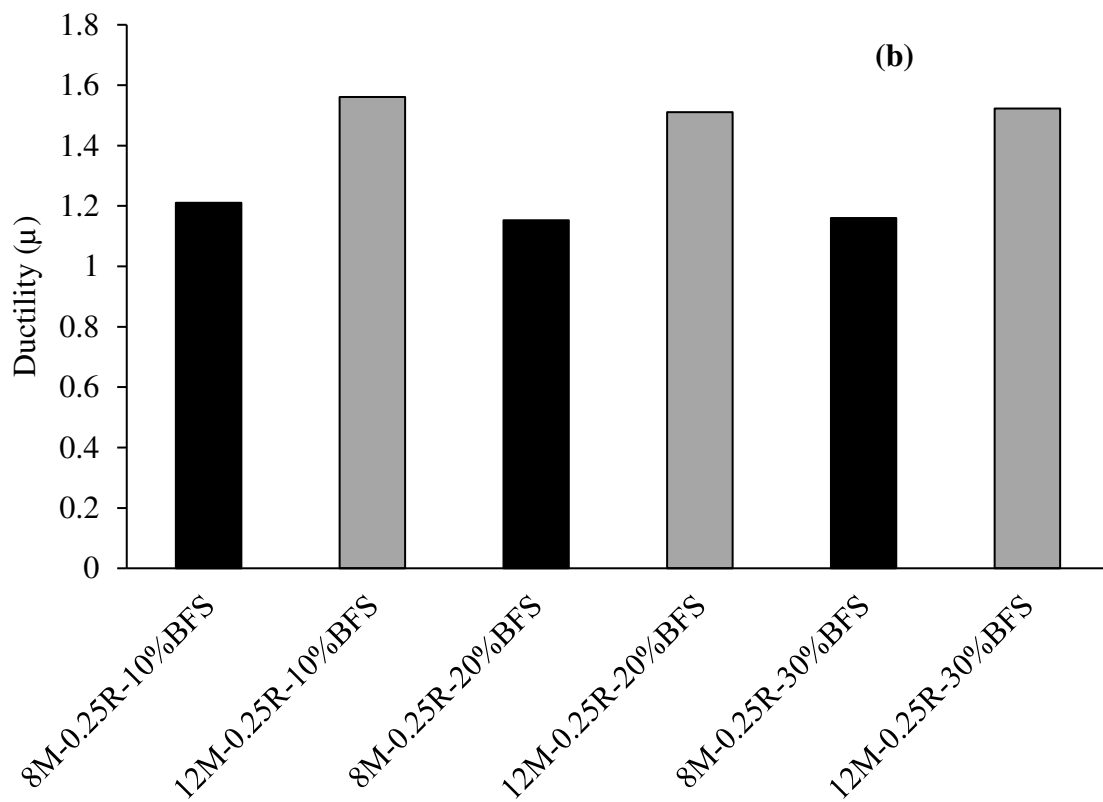
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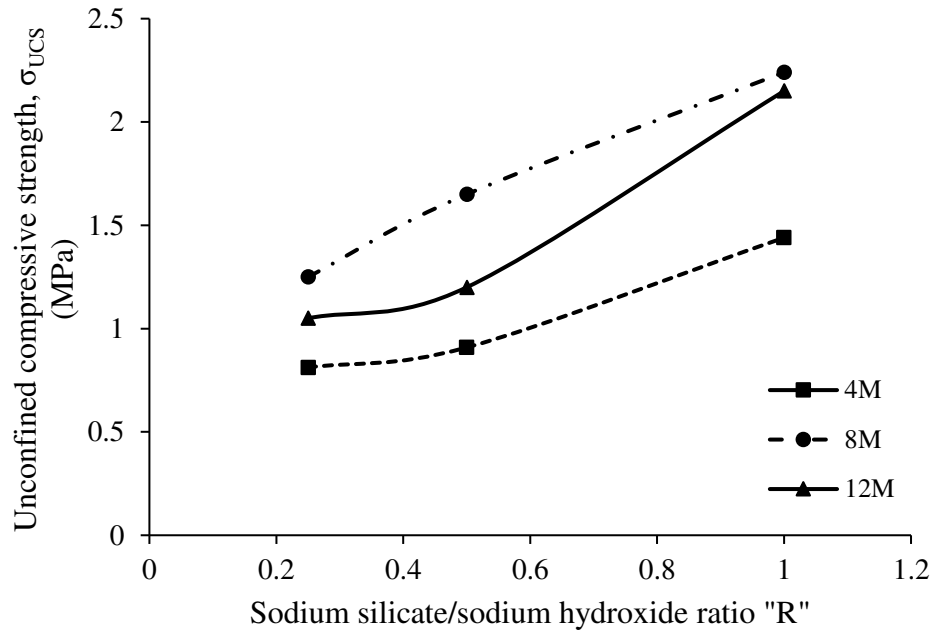


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Fig. 7 Ductility of geopolymer samples (a) Measurement (b) Effect with change in molarity for varying slag content specimens



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903 Figure 8 Compressive strength variation for basalt fines geopolymer w.r.t.  $\text{Na}_2\text{SiO}_3/\text{NaOH}$   
 904 ratios (R)

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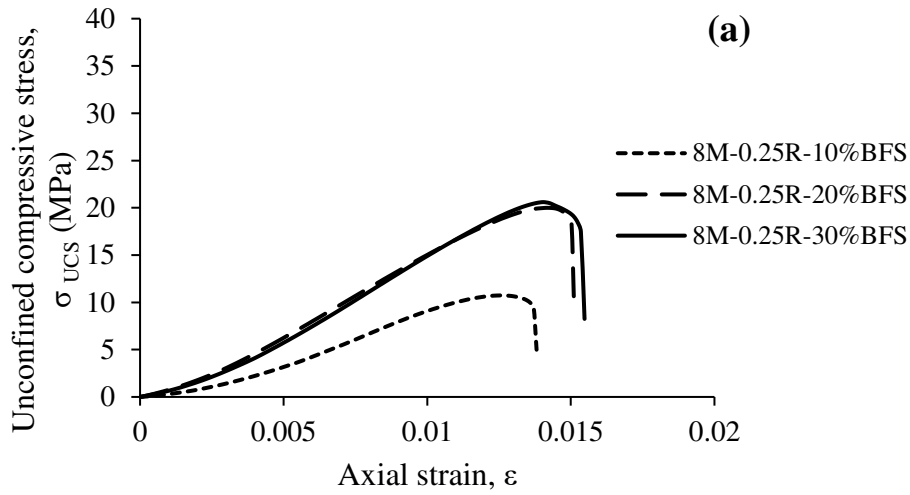
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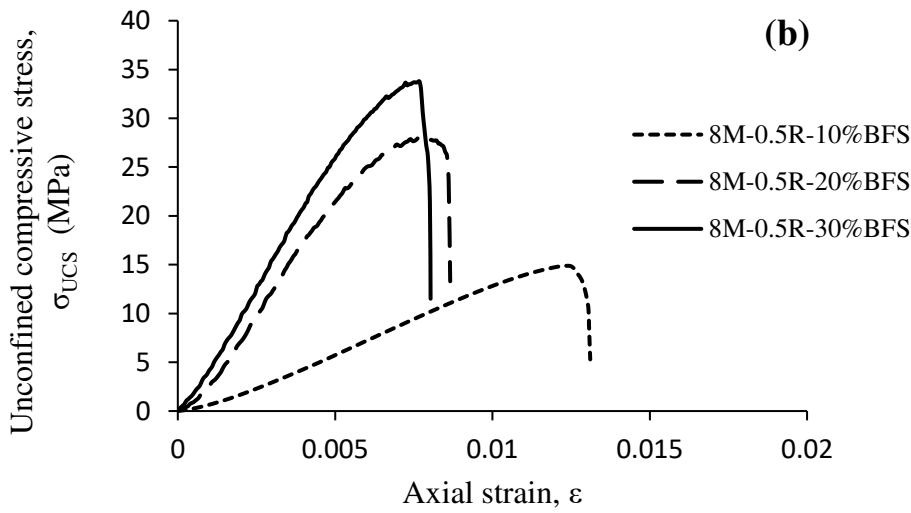
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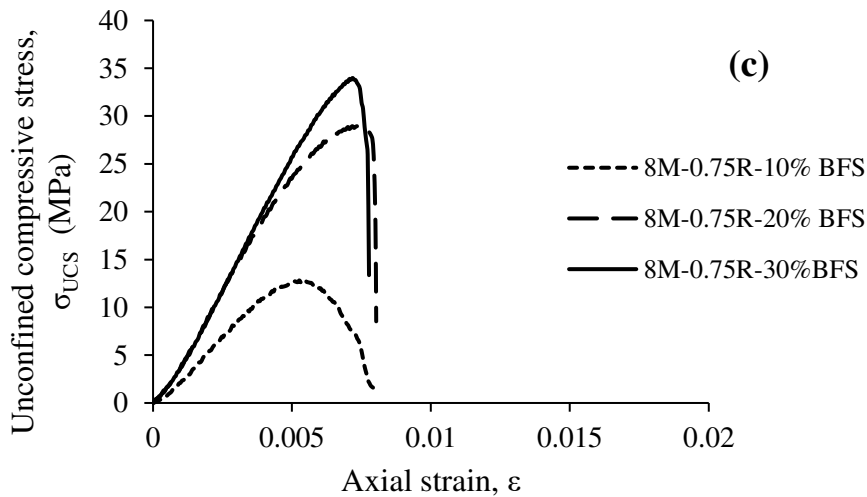
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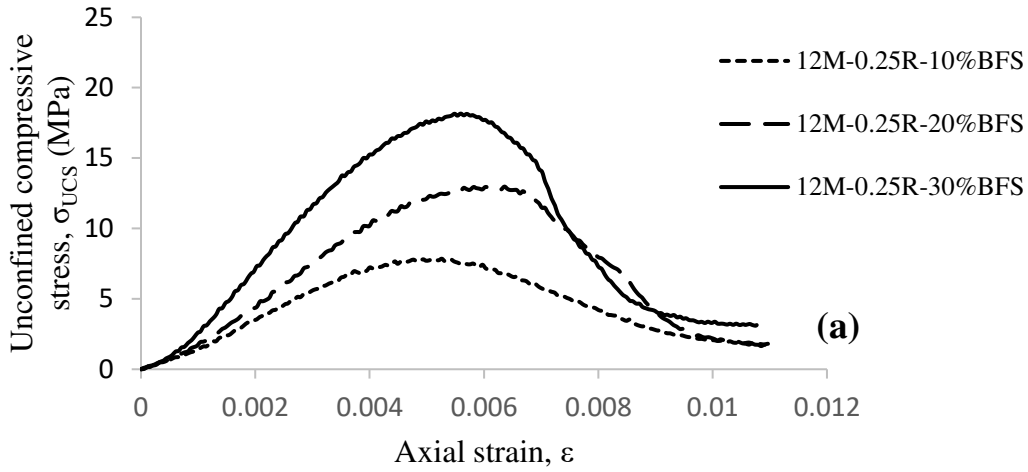
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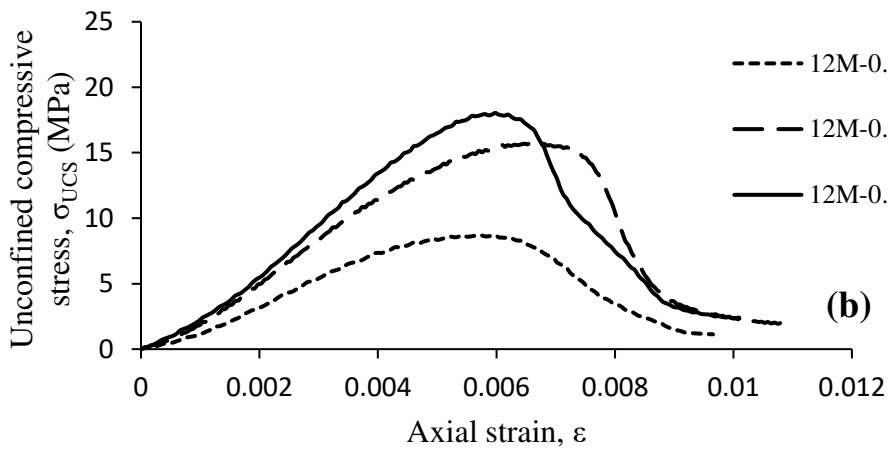
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924 Figure 9 Stress-strain curves for basalt fines and different percentages of blast furnace slag  
 925 geopolymer with 8M NaOH concentration and  $\text{Na}_2\text{SiO}_3/\text{NaOH}$  ratio of (a) 0.25 (b) 0.5 and (c)  
 926 0.75

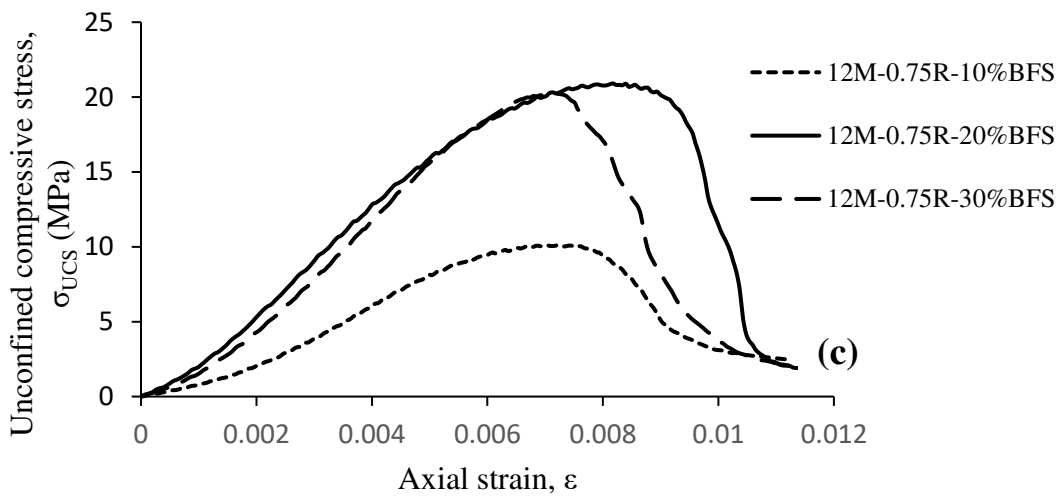




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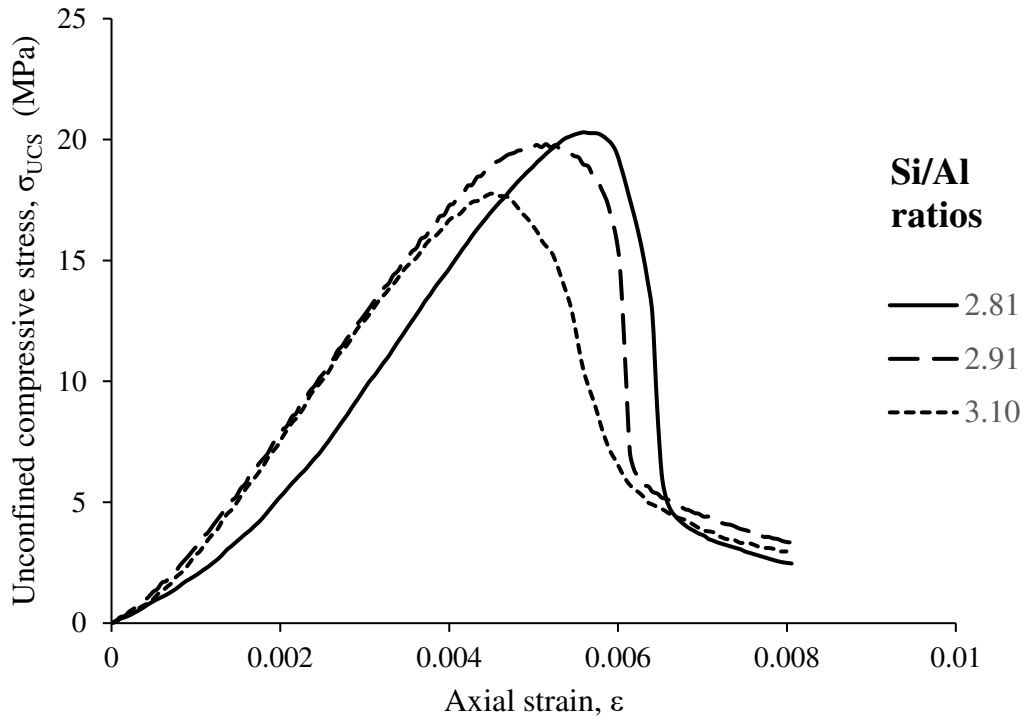


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Figure 10 Stress-strain curves for basalt fines and different percentages of blast furnace slag geopolymer with 12M NaOH concentration and  $Na_2SiO_3/NaOH$  ratio of (a) 0.25 (b) 0.5 and (c) 0.75



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Figure 11 Stress-strain curves for basalt fines and blast furnace slag geopolymers with varying Si/Al ratios

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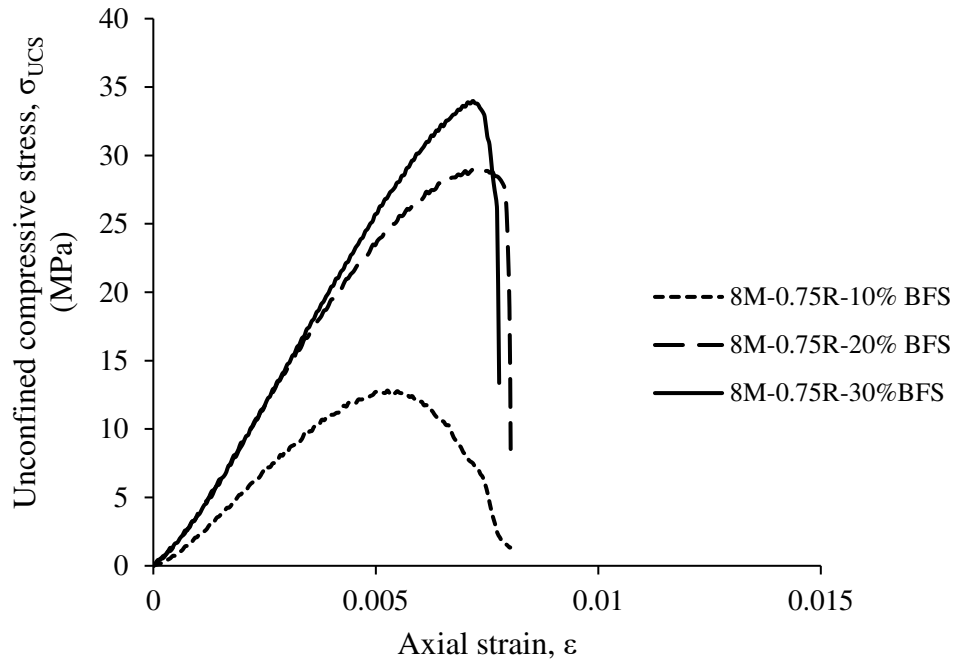
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955 Figure 12 Stress-strain curves for basalt fines and different percentages of blast furnace slag  
 956 geopolymer with 8M NaOH concentration and  $\text{Na}_2\text{SiO}_3/\text{NaOH}$  ratio of 0.75

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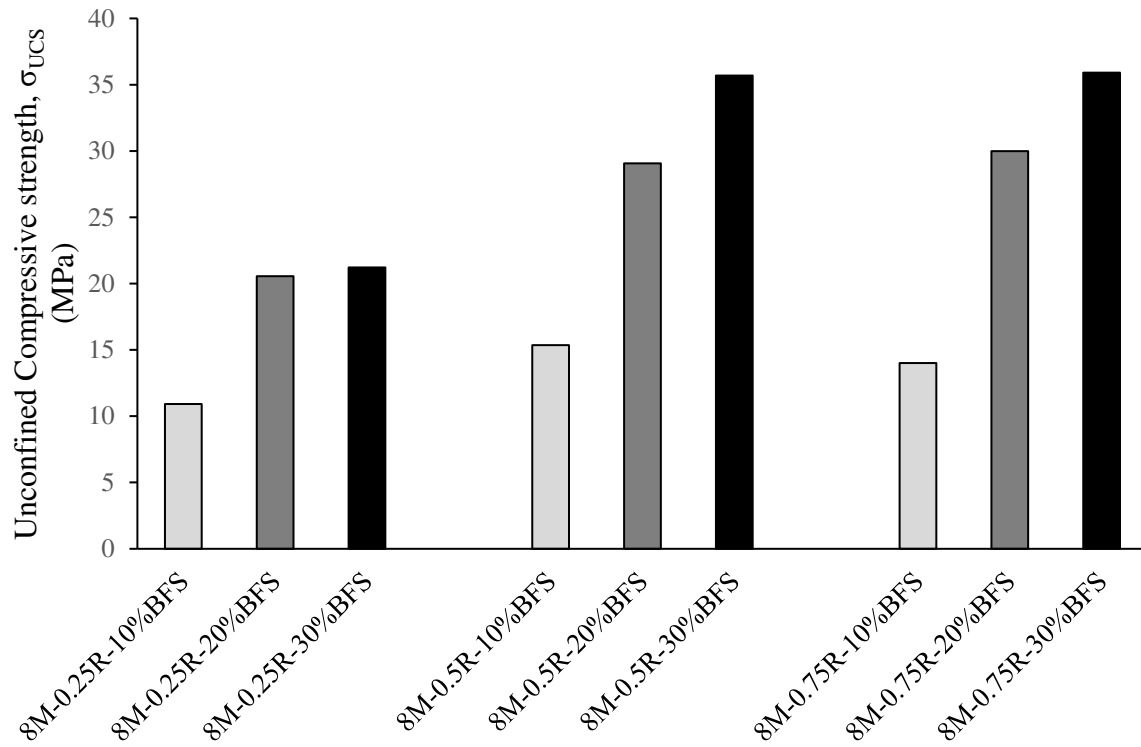
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975 Figure 13 Comparison chart of compressive strength of basalt fines geopolymer with 8M  
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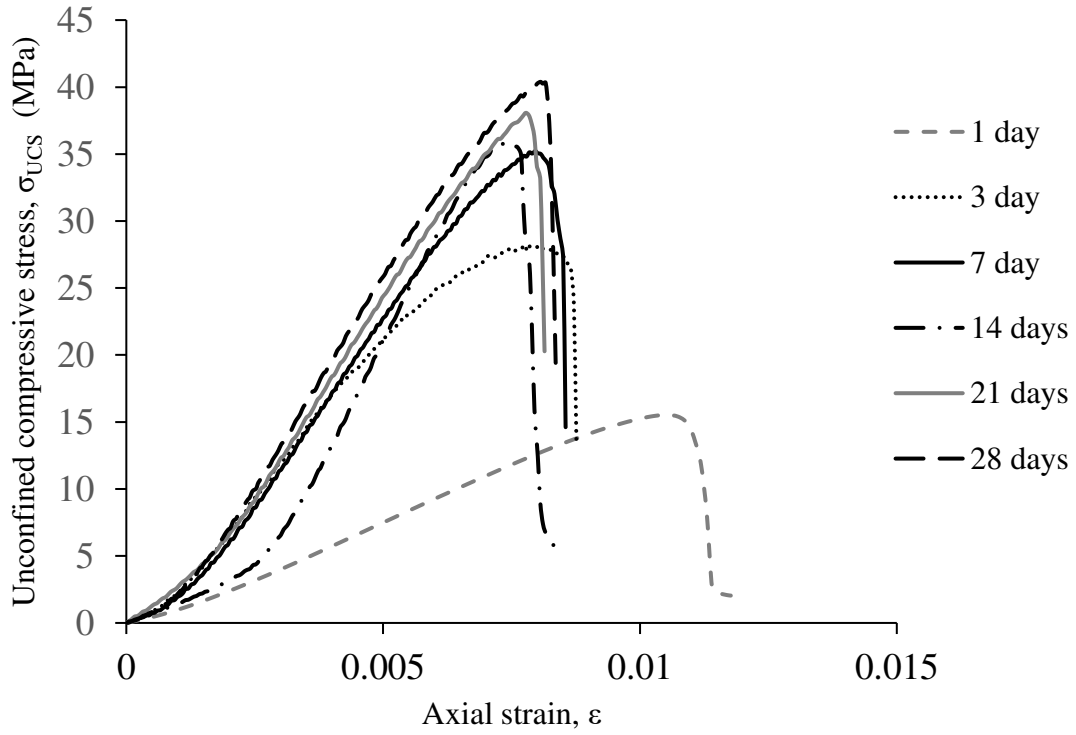
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994 Figure 14 Stress-strain variation for basalt fines and 30% blast furnace slag geopolymer with  
 995 respect to curing time

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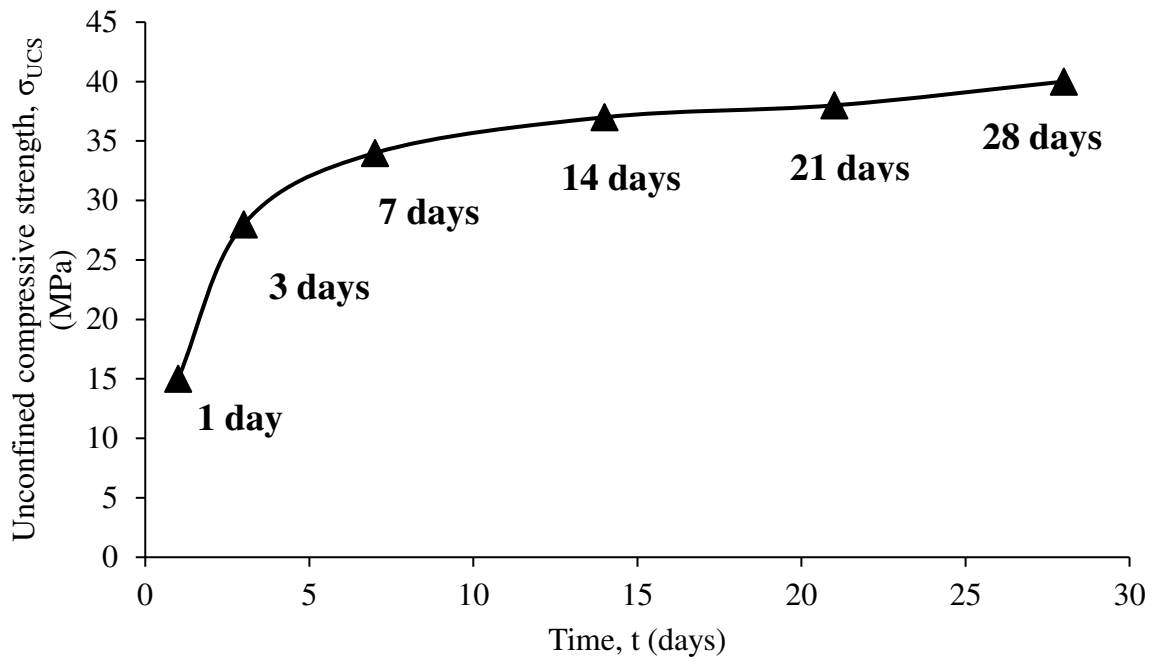
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1014 Figure 15 Variation in peak axial stress of basalt fines and 30% blast furnace slag  
 1015 geopolymer with time

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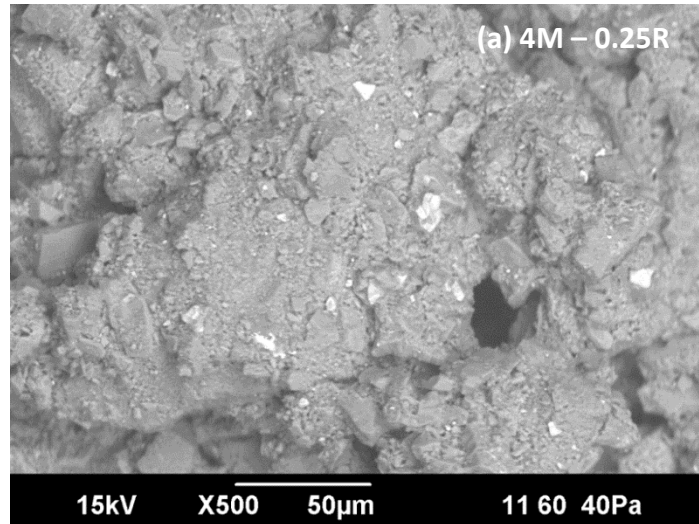
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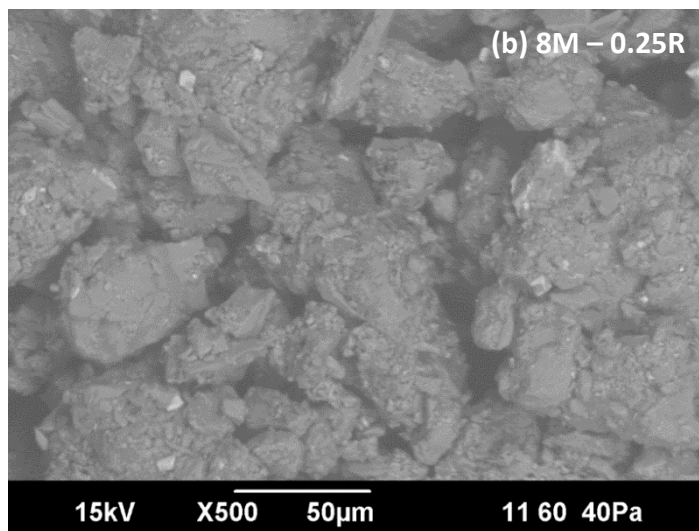
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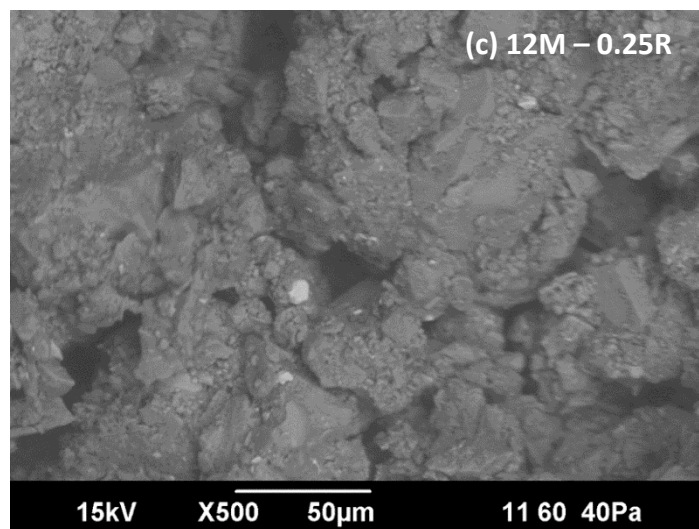
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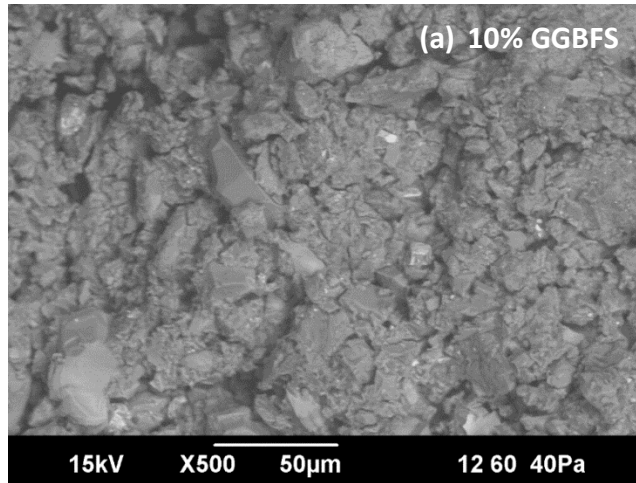
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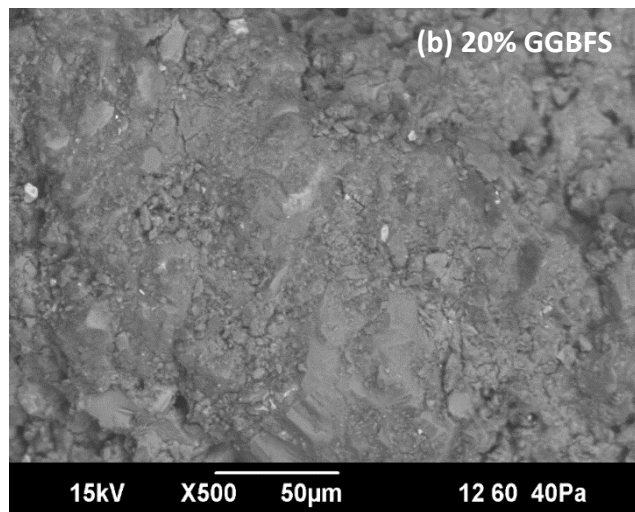
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1035 Figure 16 SEM images for basalt fines geopolymer at different molar concentrations of  
1036 alkaline activator (a) 4M-0.25R (b) 8M-0.25R and (c) 12M-0.25R (Micrographs taken by  
1037 Mohsin Nawaz)

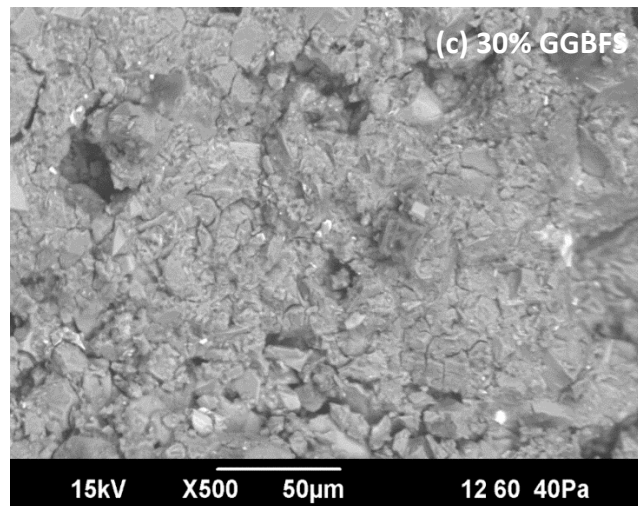
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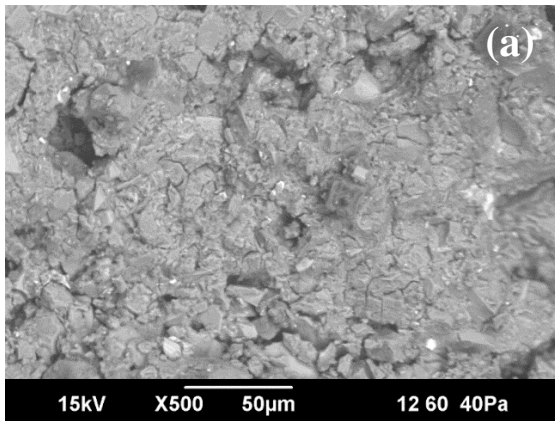
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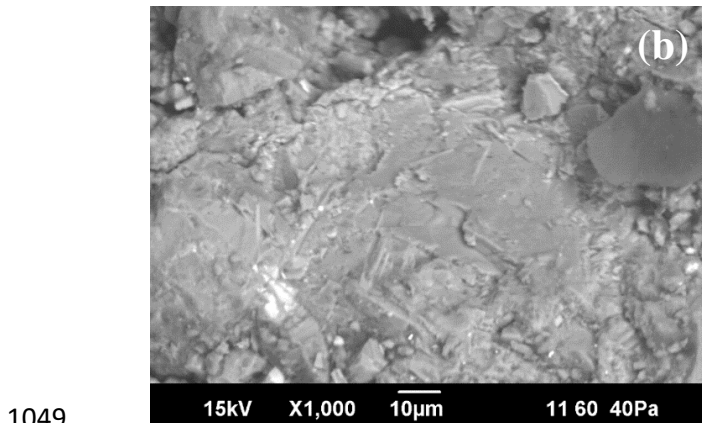
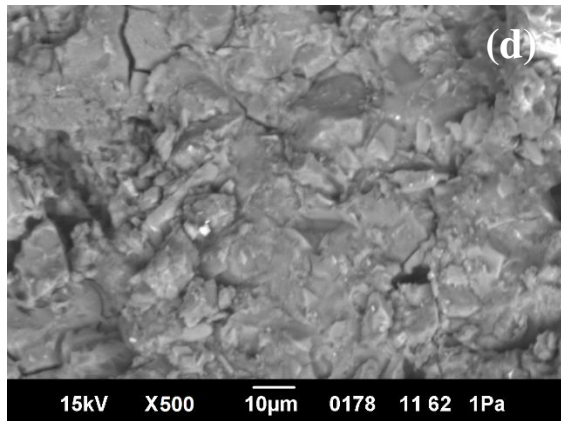
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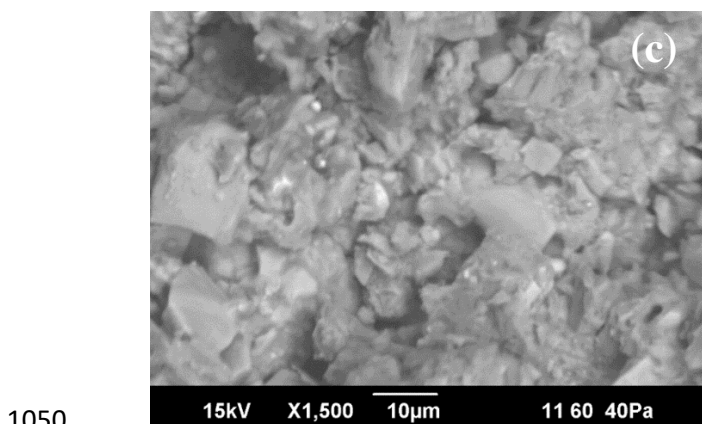
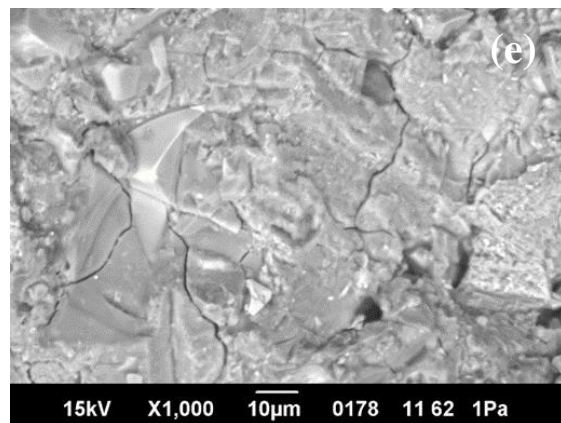
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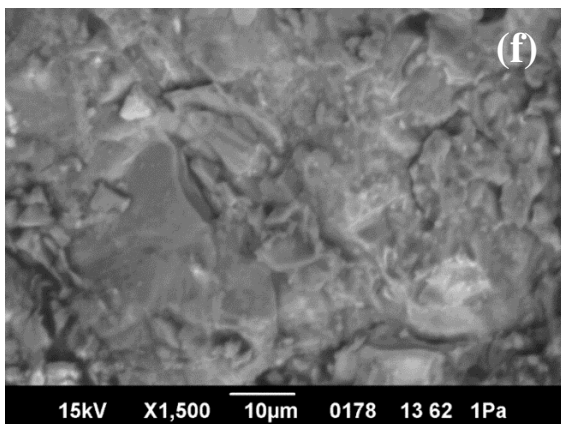
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Figure 18 Microstructure of basalt fines and blast furnace slag geopolymer (8M-0.75R-30%BFS) after 7 days (a) x500 (b) x1000 (c) x1500 and 28 days (d) x500 (e) x1000 and (f) x1500 (Micrographs taken by Mohsin Nawaz)

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