



This is a repository copy of *Prediction of the curing time to achieve maturity of the nano-cement based concrete using the Weibull distribution model : a complementary data set.*

White Rose Research Online URL for this paper:
<http://eprints.whiterose.ac.uk/167382/>

Version: Published Version

Article:

Jo, B.W., Chakraborty, S. orcid.org/0000-0002-1387-0385 and Kim, H. (2015) Prediction of the curing time to achieve maturity of the nano-cement based concrete using the Weibull distribution model : a complementary data set. *Data In Brief*, 4. pp. 285-291. ISSN 2352-3409

<https://doi.org/10.1016/j.dib.2015.05.018>

Reuse

This article is distributed under the terms of the Creative Commons Attribution (CC BY) licence. This licence allows you to distribute, remix, tweak, and build upon the work, even commercially, as long as you credit the authors for the original work. More information and the full terms of the licence here:
<https://creativecommons.org/licenses/>

Takedown

If you consider content in White Rose Research Online to be in breach of UK law, please notify us by emailing eprints@whiterose.ac.uk including the URL of the record and the reason for the withdrawal request.



eprints@whiterose.ac.uk
<https://eprints.whiterose.ac.uk/>



ELSEVIER

Contents lists available at ScienceDirect

Data in Brief

journal homepage: www.elsevier.com/locate/dib



Data Article

Prediction of the curing time to achieve maturity of the nano-cement based concrete using the Weibull distribution model: A complementary data set



Byung Wan Jo*, Sumit Chakraborty, Heon Kim

Department of Civil and Environmental Engineering, Hanyang University, Seoul 133791, South Korea

ARTICLE INFO

Article history:

Received 28 May 2015

Accepted 29 May 2015

Available online 12 June 2015

Keywords:

Nano-cement

Ordinary Portland cement

Weibull distribution

Curing time

Maturity of concrete

ABSTRACT

This data article provides a comparison data for nano-cement based concrete (NCC) and ordinary Portland cement based concrete (OPCC). Concrete samples (OPCC) were fabricated using ten different mix design and their characterization data is provided here. Optimization of curing time using the Weibull distribution model was done by analyzing the rate of change of compressive strength of the OPCC. Initially, the compressive strength of the OPCC samples was measured after completion of four desired curing times. Thereafter, the required curing time to achieve a particular rate of change of the compressive strength has been predicted utilizing the equation derived from the variation of the rate of change of compressive strength with the curing time, prior to the optimization of the curing time (at the 99.99% confidence level) using the Weibull distribution model. This data article complements the research article entitled "Prediction of the curing time to achieve maturity of the nano-cement based concrete using the Weibull distribution model" [1].

© 2015 Elsevier Inc. This is an open access article under the CC BY license (<http://creativecommons.org/licenses/by/4.0/>).

DOI of original article: <http://dx.doi.org/10.1016/j.conbuildmat.2015.03.037>

* Corresponding author. Tel.: +82 2 2220 1804; fax: +82 2 2292 0321.

E-mail address: joycon@hanmail.net (B.W. Jo).

<http://dx.doi.org/10.1016/j.dib.2015.05.018>

2352-3409/© 2015 Elsevier Inc. This is an open access article under the CC BY license (<http://creativecommons.org/licenses/by/4.0/>).

1. Specifications table

Subject area	Materials science
More specific subject area	Cement based material (Nano-cement and ordinary Portland cement)
Type of data	Table and graph
How data was acquired	The data were acquired using a universal testing machine (Schimadzu, CCM-200A; Shimadzu Corporation, Japan).
Data format	Raw and processed
Experimental factors	The compressive strength of the concrete fabricated in this investigation was estimated in accordance with the Korean standard KS F 2405 [2]. The compressive strength of cylindrical concrete sample of the dimension 10 cm × 20 cm was measured using a universal testing machine with a loading rate 0.06 MPa/min. To obtain an average compressive strength, six concrete samples from each batch of the concrete mix design were tested. During the test, maximum load obtained at the complete failure of the specimen was estimated.
Experimental features	In this investigation, concrete samples were fabricated using ordinary Portland cement using ten different mix design, followed by compressive strength measurement at four different curing times. Subsequently, the required curing time to achieve a particular rate of change of the compressive strength was predicted utilizing the equation derived from the variation of the rate of change of compressive strength with the curing time, prior to the optimization of the curing time (at the 99.99% confidence level) using the Weibull distribution model.
Data source location	Department of Civil Engineering, Hanyang University, Seoul, South Korea
Data accessibility	The data are supplied with this paper

2. Value of the data

- Evaluation of the compressive strength of ordinary Portland cement compared to nano-cement based concrete.
- Estimation of the rate of change compressive strength of ordinary Portland cement as a function of curing time.
- Optimization of the curing time to achieve a particular rate of change of compressive strength of OPCC using the Weibull distribution model.
- Weibull distribution analysis of the curing time to achieve maturity of the OPCC.

3. Data, experimental design, materials and methods

Strength data presented here are from ten different concrete samples (OPCC) fabricated to compare nano-cement based concrete (NCC) with ordinary Portland cement based concrete (OPCC).

4. Sample preparation method

In this investigation, concrete samples were fabricated using ordinary Portland cement, variable amounts fine and coarse aggregate and water (Table 1). Initially, required amount of cement was mixed with the required amount of fine and coarse aggregate, followed by mixing with quantified amount of water. Thereafter, the concrete samples were cast immediately into the mold of the dimension 10 cm × 20 cm. After complete setting, the samples were removed from the mold and allowed to cure for four different curing times such as 3, 7, 14 and 28 days.

5. Characterization and data analysis

The prime focus of this investigation was to optimize the curing time using Weibull distribution model. Initially, the compressive strength of concrete (OPCC) was measured using a universal testing machine with a loading rate 0.06 MPa/min in accordance with the Korean standard KS F 2405 [2]. The

Table 1

Formulations code and mixing proportion of the components for the fabrication of ordinary Portland cement based concrete.

Formulations code	Component weight (kg/m ³)			
	Ordinary Portland cement	Water	Fine aggregate	Coarse aggregate
OPCC-1	420	400	396	924
OPCC-2	420	400	436	884
OPCC-3	420	400	474	846
OPCC-4	420	400	516	804
OPCC-5	420	400	554	766
OPCC-6	420	380	500	820
OPCC-7	420	400	500	820
OPCC-8	420	420	500	820
OPCC-9	420	440	500	820
OPCC-10	420	460	500	820

Table 2

Compressive strength of ordinary Portland cement based concrete cured for different times (days).

Formulations code	Compressive strength (MPa) of the concrete samples cured for different curing time (days)			
	3 days cured	7 days cured	14 days cured	28 days cured
OPCC-1	11.3	18.5	25.4	31.6
OPCC-2	11.8	19.5	26.6	33.0
OPCC-3	14.7	24.8	34.2	42.6
OPCC-4	15.2	25.6	35.2	44.1
OPCC-5	12.9	21.5	29.5	36.8
OPCC-6	12.4	20.5	28.1	35.0
OPCC-7	13.4	22.4	30.8	38.2
OPCC-8	14.1	23.7	32.6	40.6
OPCC-9	14.4	24.3	33.4	41.6
OPCC-10	13.8	23.1	31.8	39.5

compressive strengths of ten different mix design of the ordinary Portland cement based concrete (OPCC) are presented in Table 2. The plot of compressive strength vs curing time as well as the plot of the rate of change of compressive strength (df_c/dt) vs curing time of the ten different mix design of OPCC is presented in Fig. 1. A trend line for the variation of compressive strength with curing time was predicted. Thereafter, a first order derivative of the data points of trend line w.r.t. curing time was calculated to obtain a rate of change of compressive strength. Additionally, a best fitted equation of the plot of the rate of change of compressive strength (df_c/dt) vs curing time of each type of concrete was estimated. The values of the various parameters of the best fitted equation are tabulated in Table 3. From this best fitted equation of the each type of concrete mix design, the times (t_{r1} , t_{r2} , t_{r3} , and t_{r4}) required to achieve a different rate of change of compressive strength ($(df_c/dt) = (df_c/dt)_{\max} \times 10^{-2}$, $(df_c/dt)_{\max} \times 10^{-3}$, $(df_c/dt)_{\max} \times 10^{-4}$, and $(df_c/dt)_{\max} = 0$) were estimated (Table 4). Analyzing the results, a range of the curing time is observed to achieve a particular rate of change of compressive strength of the ordinary Portland cement based concrete fabricated using ten different mix design. Therefore, to normalize this range of the data, a widely used statistical model (Weibull distribution) has been selected. Using this two parameter Weibull distribution model, we are trying to normalize the data at 99.99% probability.

The probability function of two-parameter semi-empirical distribution (Weibull distribution) is given by Barsoum [3]. Hence, to analyze the curing time such as t_{r1} , t_{r2} , t_{r3} , and t_{r4} of the OPCC using the Weibull distribution model, initially survival probability (S) was calculated. Determination of the survival probability (S) for each set of the data, such as t_{r1} , t_{r2} , t_{r3} , and t_{r4} leads to predict the m and σ_0

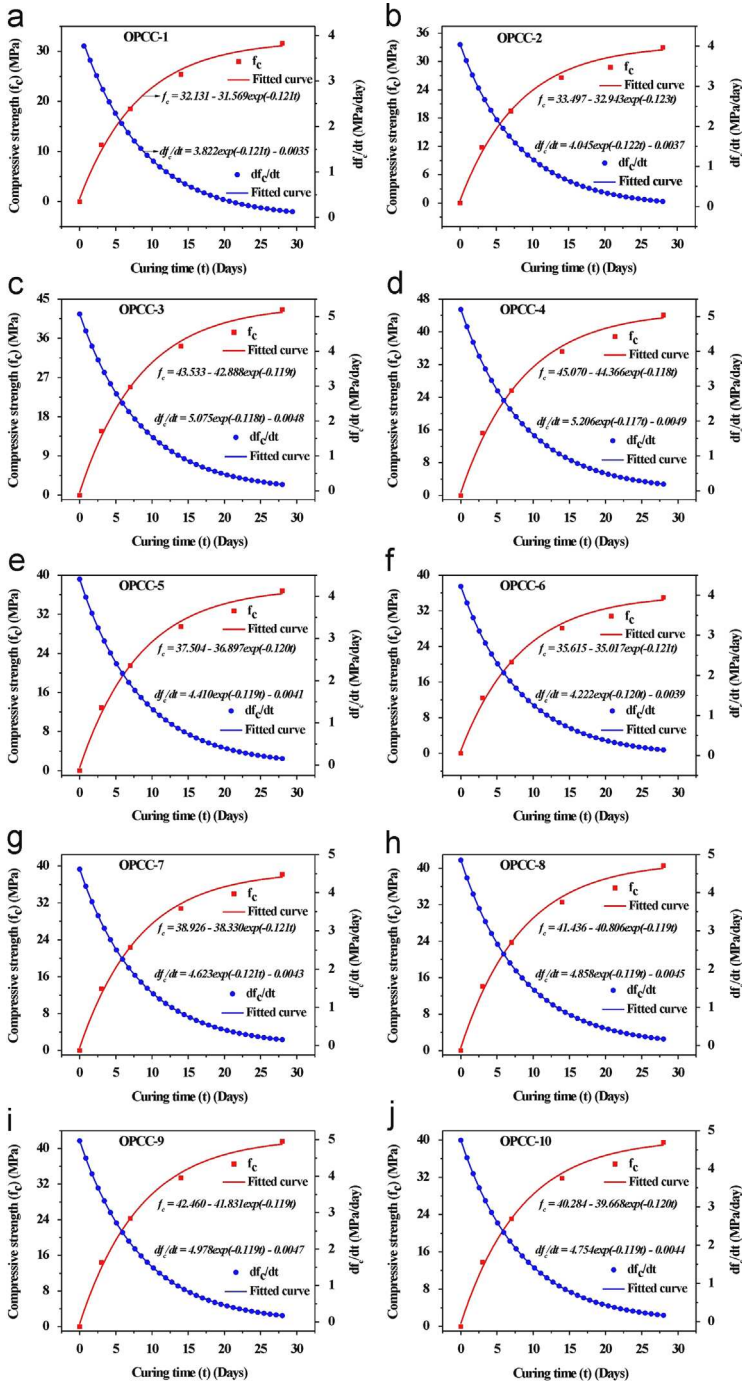


Fig. 1. Variation of the compressive strength (f_c) of ordinary Portland cement based concrete as a function of the curing time (t), fitting of the compressive strength data with an exponential equation to evaluate the trend for the increment of the compressive strength with curing time as well as the variation of the rate of change of the compressive strength (df_c/dt) of the ordinary Portland cement based concrete as a function of the curing time (t) and the fitting of the rate of change of compressive strength with an exponential equation to predict the trend for the decrement of the rate of change of the compressive strength with curing time, (a) OPCC-1, (b) OPCC-2, (c) OPCC-3, (d) OPCC-4, (e) OPCC-5, (f) OPCC-6, (g) OPCC-7, (h) OPCC-8, (i) OPCC-9, (j) OPCC-10.

Table 3

The values of the different parameters of the exponential equation for different concrete samples obtained by fitting of the rate of change of the compressive strength (df_c/dt) vs time (t) plot.

Sample code	Values of the different parameters of the equation $df_c/dt = (df_c/dt)_{28d} + (df_c/dt)_{max} \times \exp(R_0 \times t)$ from df_c/dt vs t plot			R^2
	$(df_c/dt)_{28d}$	$(df_c/dt)_{max}$	R_0	
OPCC-1	-0.00354	3.822	-0.1210	0.999
OPCC-2	-0.00371	4.045	-0.1220	0.999
OPCC-3	-0.00476	5.075	-0.1179	0.999
OPCC-4	-0.00491	5.206	-0.1170	0.999
OPCC-5	-0.00410	4.410	-0.1190	0.999
OPCC-6	-0.00392	4.222	-0.1200	0.999
OPCC-7	-0.00429	4.623	-0.1210	0.999
OPCC-8	-0.00454	4.859	-0.1186	0.999
OPCC-9	-0.00466	4.978	-0.1185	0.999
OPCC-10	-0.00443	4.754	-0.1194	0.999

Table 4

The estimated time to reach a different value of the (df_c/dt) for each concrete mix design using the equation represented in Table 3.

Sample code	Time (days) required to reach a different value of the rate of change of compressive strength (df_c/dt)			
	t_{r1}^a	t_{r2}^b	t_{r3}^c	t_{r4}^d
OPC-1	37.33	51.67	56.87	57.72
OPC-2	36.94	51.17	56.36	57.21
OPC-3	38.30	52.97	58.27	59.13
OPC-4	38.59	53.36	58.67	59.53
OPC-5	37.90	52.22	57.25	58.05
OPC-6	37.64	52.09	57.33	58.18
OPC-7	37.32	51.65	56.84	57.68
OPC-8	38.07	52.67	57.95	58.80
OPC-9	38.11	52.71	57.98	58.84
OPC-10	37.82	52.34	57.58	58.44

^a Time required to reach the value $(df_c/dt)_{max} \times 10^{-2}$, where $(df_c/dt)_{max}$ is the maximum value of the rate of change of compressive strength (df_c/dt).

^b Time required to reach the value $(df_c/dt)_{max} \times 10^{-3}$.

^c Time required to reach the value $(df_c/dt)_{max} \times 10^{-4}$.

^d Time required to reach the value of $(df_c/dt)=0$.

value. From this m and σ_0 , the design value (σ) of the curing time was calculated. Where m is a shape factor usually referred as Weibull modulus [3], σ is the design value of the curing time (at the survival probability equal to 99.99%) to achieve a particular rate of change of the compressive strength and σ_0 is a normalizing parameter (at the survival probability equals to $1/e$, i.e. 37%). In this study, σ refers to a minimum value of the t_{r1} , t_{r2} , t_{r3} , and t_{r4} at the 99.99% confidence level. It means that a minimum value of t_{r1} , t_{r2} , t_{r3} , and t_{r4} , which will be achieved in 99.99% case, if it is predicted for 100 times. Accordingly, σ_0 refers to a minimum value of the t_{r1} , t_{r2} , t_{r3} , and t_{r4} at the 37% confidence level. It indicates that a minimum value of t_{r1} , t_{r2} , t_{r3} , and t_{r4} , which will be achieved in 37% case, if it is predicted for 100 times. Table 5 represents the values of S_j and $\ln(t_{r1})$ of the OPCC. Likewise t_{r1} , the values of $\ln(t_{r2})$, $\ln(t_{r3})$ and $\ln(t_{r4})$ were calculated. The plot of $-\ln\ln(1/S_j)$ vs $\ln(t_{r1})$, $-\ln\ln(1/S_j)$ vs $\ln(t_{r2})$, $-\ln\ln(1/S_j)$ vs $\ln(t_{r3})$ and $-\ln\ln(1/S_j)$ vs $\ln(t_{r4})$ of OPCC are shown in Fig. 2. From this plot, σ_0 is calculated using the slope (m) and intercept values for the OPCC. The values of the Weibull modulus m , σ_0 (predicted curing time at survival probability=37%) and σ (predicted curing time at survival probability=99.99%) for

Table 5

Summary of the calculated data for t_{r1} of ordinary Portland cement based concrete for the prediction of Weibull modulus and σ_0 values by Weibull fitting.

Ordinary Portland cement based concrete (OPCC)				
Rank	t_{r1}	S_j	$-\ln(\ln(1/S_j))$	$\ln(t_{r1})$
1	36.94	0.933	2.663843	3.609
2	37.32	0.837	1.723263	3.620
3	37.33	0.740	1.202023	3.620
4	37.64	0.644	0.821667	3.628
5	37.82	0.548	0.508595	3.633
6	37.90	0.452	0.230365	3.635
7	38.07	0.356	-0.03292	3.639
8	38.11	0.260	-0.29903	3.640
9	38.30	0.163	-0.59398	3.645
10	38.59	0.067	-0.99269	3.653

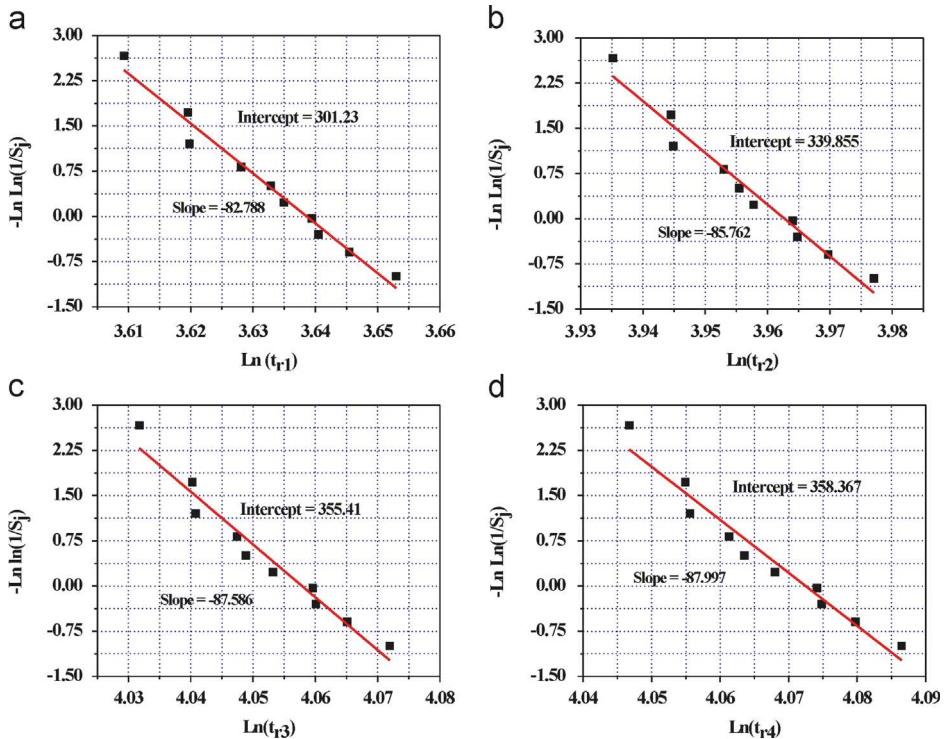


Fig. 2. Weibull fitting $-\ln(\ln(1/S_j))$ vs $\ln(t_r)$ for the ordinary Portland cement based concrete samples to predict the Weibull modulus, σ_0 (37% probability) and σ (99.99% probability). (a) $-\ln(\ln(1/S_j))$ vs $\ln(t_{r1})$ (b) $-\ln(\ln(1/S_j))$ vs $\ln(t_{r2})$ (c) $-\ln(\ln(1/S_j))$ vs $\ln(t_{r3})$ and (d) $-\ln(\ln(1/S_j))$ vs $\ln(t_{r4})$.

ordinary Portland cement based concrete (OPCC) are represented in Table 6. From the analysis, the values of t_{r1} , t_{r2} , t_{r3} , and t_{r4} at the 99.99% confidence level of ordinary Portland cement based concrete are estimated to be 37.3, 51.7, 56.8 and 57.7 days, respectively. Nonetheless, the values of the nano-cement based concrete were calculated to be 19.57, 20.91, 21.05 and 21.07 days, respectively [1]. Therefore, it is assessed that ordinary Portland cement requires more time (58 days) to be cured

Table 6

The statistically analyzed and optimized value of the curing time required to reach a particular rate of change of the compressive strength at 37% and 99.99% confidence level (probability).

Components	Intercept ^a	Slope or Weibull modulus (m) ^b	σ_0 Values at (1/e) 37% probability ^c	σ At 99.99% probability ^d
t_{r1} (days)	301.23	−82.788	38.0	37.3
t_{r2} (days)	339.86	−85.762	52.6	51.7
t_{r3} (days)	355.41	−87.586	57.8	56.8
t_{r4} (days)	358.37	−87.997	58.7	57.7

^a Intercept.

^b Slope for each component have been calculated from the respective figures of Fig. 2.

^c σ_0 is the value of the component like t_{r1} , t_{r2} , t_{r3} and t_{r0} at 37% confidence level.

^d σ is the value of the component like t_{r1} , t_{r2} , t_{r3} and t_{r04} at 99.99% confidence level.

completely as compared to that of the nano-cement based concrete (21 days). Although, it was reported by ACI Committee 308 [4] that different types of cement take different times to cure completely. Additionally, ACI 214R-02 [5] reported that usually 28 days are required to yield adequate curing of the Portland cement based concrete.

Acknowledgment

Author would like to acknowledge the BK 21, Government of Korea (Republic of) for their funding to pursue this research program.

References

- [1] B.W. Jo, S. Chakraborty, H. Kim, Prediction of the curing time to achieve maturity of the nano-cement based concrete using the Weibull distribution model, *Constr. Build. Mater.* 84 (2015) 307–314.
- [2] KSF 2405: 2010, Testing method for compressive strength of concrete. Bureau of Korean standard, Seoul, 2010.
- [3] M.W. Barsoum, *Fundamentals of Ceramics*, International ed. The McGraw Hill Companies; Series in material science and engineering, India, 1997.
- [4] ACI Committee 308R-01, Guide to Curing Concrete. American concrete Institute, Detroit: Michigan. ACI 308R-01. (http://civilwares.free.fr/ACI/MCP04/308r_01.pdf).
- [5] ACI 214R-02, Evaluation of Strength Test Results of Concrete, American concrete Institute, Detroit: Michigan 229.