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Characterisation of temporal variations of alkali-activated slag cement property using microstructure features and electrical responses

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9 Abstract

10 Relatively few studies currently exist concerning temporal variation of the overall performance of 11 alkali-activated slag (AAS) cement; a topic essential for quality control in practice. This study uses 12 microstructural characteristics and electrical responses to reflect performance variation of AAS cement. 13 To achieve this, four key performance parameters were assessed, pore formation factor, chloride 14 transport coefficient, permeability coefficient and compressive strength. Classical theories used for 15 PC-based materials were applied to estimate the performance of AAS paste. It is found that the 16 formation factor of AAS can be assessed by combining bulk conductivity and pore solution 17 conductivity and the permeation properties of AAS cement cannot be reliably assessed by the Katz-Thomoson (KT) model, while the Millington-Quirk (MQ) model works better. To assess the 18 19 relationship between compressive strength and permeable porosity, the Balshin and Ryshkevitch 20 equations performed best in terms of describing the relationship between these two parameters. 21 Meanwhile, AAS cement is more sensitive to the mix proportions and additional care is needed to 22 ensure its quality. It is the first time to use electrical response to predict the chloride transport 23 coefficients of AAS cement, which would provide a powerful tool for its quality control and service 24 life prediction.

- Keywords: alkali-activated slag cement, microstructure features, electrical responses, pore formation
 factor, permeation properties, strength development
- 27

1

1. Introduction

Owing to remarkable economic growth and high speeds of construction in the last 40 years, China has
become the world's largest cement producer, responsible for nearly 60% of Portland cement (PC)
manufactured worldwide [1]. With cement production closely related to key environmental concerns
such as energy consumption and CO₂ emission, several viable alternatives that provide supplementary
choice and/or improvement to conventional PC systems are the focus of a vibrant research area.

Use of industry-by-products is an established way to reduce uses of PC clinker [2-4]. Reflecting
the size of China's steel industry [5, 6], ground granulated blast-furnace slag (GGBS) has emerged as
a main supplementary cementitious material (SCM) used both as a partial replacement for PC to
improve the performance of conventional concrete, and also to manufacture alkali-activated slag
(AAS) cement-based materials [7, 8].

12 AAS cement was formally invented in 1957 by Glukhovsky at the Kiev Institute of Civil 13 Engineering, Ukraine, although the possibility of using alkaline activation of slag can be traced back 14 earlier [9-11]. Previous research [12-15] has shown that AAS binding systems have several advantages 15 over PC, such as rapid and high compressive strength, low heat of hydration, high temperature and 16 corrosion resistance against chemical attacks and/or from high chloride concentration situations. While 17 practical AAS applications are being reported worldwide [16-18], its widespread adoption requires in-18 depth understanding of its disadvantageous properties, including controlled setting behaviour, 19 occurrence of efflorescence [19], shrinkage and micro-cracking [20-23], and suitable water reducing 20 techniques [9]. In addition, urgently needed are established techniques to control and predict long-term 21 AAS properties.

It is known [2,17,19] that standard core analysis is not always able to predict the overall performance of AAS-based materials, with high statistical variance of results failing to provide sufficient information concerning all processes which may be occurring. Microstructural features and electrical responses can be used to assess and predict transport and mechanical properties of PC and relevant techniques can provide satisfactory results [24]. Despite this, special care is still needed when using these approaches to assess AAS performance [17], due to the significant differences in physical and chemical characteristics that exist between the two binder systems.

Therefore, four key performance parameters, including formation factor, permeability, diffusivity and compressive strength, were used to characterise the performance of AAS cement pastes. To achieve this, compressive strength, bulk conductivity, pore solution conductivity, accessible porosity and pore size distribution were all measured. Further comparisons were made to examine if established techniques for PC-based systems were applied to AAS cement-based materials.

2. Theoretical consideration

2 2.1 Estimation of pore formation factor

1

3 An essential parameter describing transport properties, pore formation factor is the ratio of fluid path 4 length to the length of the sample, which is a function of porosity and pore connectivity. Determination 5 of formation factor is not straightforward as the micro-geometry of cementitious materials are 6 characterised by mesoscale heterogeneities. As such, fluid molecules are unable to sample the 7 representative volume of pore space in the experimentally accessible measurement time. It has been 8 observed [24] that when measuring the flow of electric current through fully saturated samples, path 9 connectivity is dependent on the conducting fluid and the microstructure characteristics, while 10 electrical resistivity is a simple function of porosity and average length of path. As the flow of fluid 11 and electric current in many aspects are analogous, electrical measurements in cementitious materials 12 can be described using:

13
$$\rho_T = \rho_O \cdot \frac{1}{\beta \cdot \phi}$$
 Eq 1

14 where: ρ_T is the total resistivity (Ω); ρ_O is the resistivity of the pore solution, which is a function 15 of ion composition and concentration in solution (Ω); ϕ is the porosity of the system accessible to 16 fluids; and β is the connectivity of the pores in the system, which characterises how capillary pores are 17 connected in a three-dimensional system. The resistivity of a saturated sample solely depends on the 18 two microstructural parameters (porosity ϕ , and connectivity β). As it is often difficult to differentiate 19 between these two terms, the literature [24] presents their product, which inversely is referred to as the 20 formation factor, *F*, where:

$$\beta \cdot \phi = \frac{1}{F}$$
 Eq 2

Formation factor is a material property that is independent of specimen size or shape, but relies on the characteristics of the medium. Further, the formation factor is also referred to as a transport property, as it can be used to analyse the diffusivity of porous materials. As such, an accurate estimate of formation factor is of particular significance.

26 2.2 Estimation of ion diffusion coefficient

27 Diffusion is the process by which ions move through pore solution due to differences in concentration, 28 and is often considered as the main transport mechanism for chloride ions in saturated cementitious 29 materials. According to Brownian motion, the mean-square displacement of molecules is linearly 30 proportional to time in bulk fluids. The proportionality constant is the bulk diffusion coefficient, D_o . 31 The formation factor is related to the diffusion coefficient by the established Nernst-Einstein equation, 32 as follows:

$$1 \qquad \frac{\rho_T}{\rho_O} = F = \frac{D_O}{D}$$
 Eq 3

where: D_O is the self-diffusion coefficient describing how different ionic species move through dilute solutions (m²/s); and *D* is the bulk diffusion coefficient (m²/s). The self-diffusion coefficient, D_O , can be determined for different ionic species. For chloride ions at 25 °C, a typical value in pure water is 2.03×10^{-9} m²/s [25, 26].

6 Through the formation factor, the diffusion coefficient is closely connected to many transport
7 properties of fluids confined in AAS cement samples, including permeability, electrical conductivity
8 of electrolytes, and velocity of sound.

9 2.3 Estimation of permeability coefficient

Permeability is a key material property of durability assessment related to pore geometry rather than 10 11 the material's pore fluid. Modelling this parameter is a long-standing challenge as the permeability of 12 cementitious materials can vary by over five orders of magnitude. Even for concretes with the same 13 mixture proportions, variation of permeability can span three orders of magnitude [4, 27, 28]. Several 14 methods can be used to estimate permeability using microstructure properties. The first, derived from 15 Carman, is based on tortuosity and shape factor [29-31]. As this model contains pore radius and fluid 16 path length and some not directly measurable properties, it has not been considered in this study. The 17 second equation to describe saturated porous systems is the Katz-Thomoson model [32-34], which 18 introduces the term critical pore diameter, d_c (m), to assess permeability, K_{KT} (m²), through formation 19 factor (*F*) and the critical pore diameter as follows:

$$20 K_{KT} = \frac{1}{226 \cdot F} \cdot d_c^2 Eq 4$$

The third method uses the concept of series parallel arrangements of pores arising from random
 apposition of planes within cementitious materials. Millington and Quirk [35] describe the
 permeability, K_{MQ} (m²), of rock using the formation factor (*F*) and the average pore radius:

$$24 K_{MQ} = \frac{r^2}{8F} Eq 5$$

As shown in Eq (4) and (5), both critical pore diameter and average pore radius are used to 25 26 estimate permeability coefficients. It has been noted that X-ray computed micro-tomography (µCT) 27 offers three-dimensional imaging of the pore structure in building materials, while at present most 28 instruments are unable to resolve components of porosity smaller than 5 µm routinely [36, 37]. 29 Moreover, direct measurement by quantitative image analysis in principle is a good way to estimate 30 the internal distribution of pores, but in practice, agreement between porosity estimated by image 31 analysis and other methods is often poor [37, 38]. One general reason for this is that it is difficult to 32 detect pores of widely different sizes in images of fixed magnification. In other words, the method has

4 page

1 limited dynamic range. As such, mercury intrusion porosimetry (MIP) was used to obtain these two 2 parameters. While MIP has received criticism for its accuracy [39, 40], it can provide useful 3 information about threshold diameter; a parameter representative of cement permeability [34, 41]. As 4 such, \bar{r} , pore structure of AAS cement was characterised in this study by MIP.

5 2.4 Estimation of strength development from microstructure characteristics

6 Compressive strength is the most sought-after property during the fabrication of concrete and 7 numerous works have been carried out to analyse different influencing parameters [42-45]. It is 8 generally recognised that the presence of pores affects strength adversely, despite different geometries 9 of pores having different influences. Researchers [43,44] have attempted to establish links between 10 porosity and compressive strength of cementitious materials, and the effectiveness of the following 11 four equations is examined in this study.

12	Balshin's equation:	$\sigma = \sigma_0 \cdot (1 - \phi)^a$	Eq 6-1
13	Ryshkevitch's equation:	$\sigma = \sigma_0 \cdot e^{-b\phi}$	Eq 6-2
14	Schiller's equation:	$\sigma = c \cdot \ln(\frac{\phi_{CR}}{\phi})$	Eq 6-3
15	Hasselman's equation:	$\sigma = \sigma_0 \cdot (1 - A\phi)$	Eq 6-4
10			·

16 Where: σ_0 is the compressive strength at zero porosity (MPa); σ is the compressive strength 17 at porosity ϕ (MPa); ϕ is the porosity (% by volume); ϕ_{CR} is the porosity at zero strength (% by 18 volume); a, b, c, k are empirical constants. Although σ_0 can be interpreted as compressive strength 19 at zero porosity, its value may not provide reliable response as strength can be affected by many other 20 microscopic flaws not considered explicitly in these four models.

21

22

3. Experiment programme

23 3.1 Raw materials

The raw material used to prepare the AAS samples in this study was GGBS from Chongqing Iron and
Steel Group. Its specific gravity and Blaine fineness were 2.80 g/cm³ and 430 m²/kg respectively.
Conventional PC manufactured from Chongqing Fuhuang Cement Plant was used for the reference
samples. The chemical composition of the GGBS and PC used are given in Table 1.

5 page

The activating alkaline was a commercial sodium silicate solution with a modulus (SiO₂/Na₂O
molar ratio) of 2.46, a chemical composition of 29.03 wt.% SiO₂, 12.07wt.% Na₂O and 51.40% H₂O.
Chemically pure NaOH was used to adjust the modulus of sodium silicate to 1.2. The binders were
prepared at activator (Na₂O) concentrations of 5 wt.% of GGBS. Alkaline solutions were prepared and
allowed to cool to room temperature prior to mixture preparation.

6 3.2 Sample manufacture and curing regime

7 Paste samples with W/B ratios of 0.35 and 0.50 were prepared for both the AAS and PC samples. 8 Selection of these two W/B ratios is mainly based on previous experiences [14,17], as these parameters 9 could cover the compressive strength from C30 to C60. Binder powder was added to the liquid and 10 mixed for 3 mins in a planetary-type mixer. Pastes were cast into 40 mm cubes and $\Phi 40 \times 100$ mm 11 cylinders (for exacting pore solution) and immediately covered with plastic sheeting to minimise 12 superficial drying. All samples were demoulded after 1 day and placed in a water bath at constant 13 temperature (20 ± 2 °C). Three samples were withdrawn for subsequent testing at 1, 3, 7, 14 and 28 14 days.

15 3.3 Test methods

16 **3.3.1** Accessible porosity

17 Accessible porosity was examined via measurement of capillary porosity. It was determined on mass 18 differences between water-saturated and dried pastes at different temperatures according to previous 19 studies [46, 47]. After 3, 7, 14 and 28 days, samples were crushed to particle sizes around 10 mm and 20 immersed in a solution of absolute ethyl alcohol to stop hydration. Samples were dried in a 40 °C 21 vacuum oven until constant mass was achieved, which was recorded as $M_{40}{}^{o}C$ (g). Dried samples were 22 then immersed in deionised water for 24 hours, after which the saturated surface dry mass was recorded 23 as Ms (g). The volume of crushed samples was determined using Le Chatelier Flask's method [48] and 24 the capillary porosity computed using the following equation:

25
$$\phi = \frac{M_s - M_{40^{\circ}C}}{\rho_W \cdot v} \times 100\%$$
 Eq 7

where: \emptyset is the capillary porosity (% by volume); M_s is the mass of saturated sample (g); M_{40} c is the constant mass of samples dried at 40 °C (g); ρ_w is the density of water (g/cm³); and v is the bulk volume of cement paste (cm³).

29 3.3.2 Bulk electrical conductivity

Resistivity of paste samples was tested with a two-point uniaxial method via an LCR bridge. Prior to
 measurements, excess surface moisture was removed and specimens were placed between two thin
 parallel metal plates with moist sponges added to ensure effective electrode connection. Alternating
 (AC) voltage with a frequency of 1 kHz was applied in order to reduce effects of polarisation [49, 50].

The conductivity (S/m) of each specimen was calculated as the inverse of electrical resistivity, which
 was assessed using the following equation:

$$3 \qquad \rho = R \frac{A}{l} \qquad \qquad \text{Eq 8}$$

where: *ρ* is the electrical resistivity (Ω·m); *R* is the resistance of a uniform specimen (Ω); *A* is
the cross-section area of a specimen (m²); and *l* is the length of the specimen (m).

6 3.3.3 Conductivity of ion liquid

7 Pore fluids were extracted using the steel die press method following the protocol outlined by 8 Vollpracht et al [51] and Barneyback and Diamond [52]. On the day before each test, specimens were 9 removed from the aforementioned water tank and pre-conditioned in a standard environment (20±1 °C, 10 $RH \ge 95\%$) for 24 hours to achieve a saturated surface-dry state. Samples were loaded into the 11 apparatus and the equipment was used in conjunction with a compression machine as shown **Figure** 12 1. Pore solution was extracted under a constant pressure of 407.6 MPa (800 kN over 1962.5 mm²); a 13 process taking around 45 minutes. The pore fluid was collected in 10-ml plastic syringes and then 14 transferred to 50ml plastic bottles to minimise the potential carbonation effects until next analysis. 15 Electrical conductivity of the pore solution was tested by a conductivity probe (Company: INESA, 16 DDS-11a).

17 3.3.4 Mercury intrusion porosimetry

18 The pore size distribution of AAS and PC paste samples was determined using MIP. After curing for 19 3, 7, 14 and 28 days, a sample was crushed to obtain the particle size of around 2 mm. Before MIP 20 measurements, these samples were immersed in ethyl alcohol for 1 day and were moderately dried in 21 a vacuum oven at 40 °C until constant weight was achieved. The porosimeter (MicromeriticsI, mode: 22 Autopore 9500) was capable of minimum and maximum intruding pressures of 0.53 psi (26.90 mmHg) 23 and 60000 psi (413.69 MPa) respectively. All these operations were automated by microcontroller and 24 conducted within a fully enclosed pressure chamber. Incremental pore volume of each measured pore 25 diameter, d, was calculated by assuming cylindrical pore and Washburn equation Eq (9),

$$26 \qquad d = -4 \cdot \frac{\gamma \cos \theta}{P} \qquad \qquad \text{Eq 9}$$

27 where: γ is the surface tension of mercury, 0.485 N/m at 25 °C; *P* is the intrusion pressure 28 (N/m²); θ is the wetting angle for mercury, assuming to be 130°. The threshold pore size, 29 corresponding to the peak in the pore size-differential volume relationship, is found to be effective in 30 the comparative studies [34, 53] and it has been used for permeability predictions [32-34, 54].

31 3.3.5 Compressive strength

1 Compressive strengths were determined in accordance with the National Standard GB/T 17671-1999

2 [55] at ages of 3, 7, 14 and 28 days. All the results of compressive strength reported are the average

3 value of three specimens.

4

4. Results and discussion

5 4.1 Temporal property changes

6 4.1.1 Compressive strength and porosity

7 Figure 2 summarises the compressive strength results obtained for the AAS and PC paste samples, 8 and clearly indicates that decreasing W/B ratio and increasing age led to an increase in compressive 9 strength for both binders. For all samples, rapid increase of compressive strength occurred for the first 10 14 days, after which values stabilised. While AAS samples with a W/B ratio of 0.35 exhibited the most 11 rapid rate of early age strength gain and the highest recorded values among the four sample types 12 considered, an opposite trend was observed for the 0.50 W/B ratio AAS samples. More specifically, 13 compressive strengths recorded for the 0.50 W/B ratio AAS before 7 days were significantly lower 14 than for the corresponding PC samples. This observation contradicts commonly reported conclusions 15 that AAS has fast hydration reaction rates and homogeneous hydration products [14, 56, 57].

16 The strength development values and mechanical behaviour of AAS may generally be attributed 17 to the degree of porosity, hydration rate and distribution of hydration products. It is known that early-18 age reaction of AAS cement is based on alkali-mediated dissolution and precipitation. Monitored by 19 backscattered electron mode scanning electron microscope, microstructural evolution occurs as rapid 20 product formation occurs on the surface of GGBS grains in sodium silicate-activated systems, followed 21 by a slow formation processes occurring internally. In this process, nucleation of AAS from the 22 solution is dominated by the speed of precipitation of a gel like CSH phase, which fills the former pore 23 space. This is different in nature from the needle-like outer product CSH in PC systems and, in contrast, 24 forms bridging among the clinker grains [14, 58-60]. During the hydration of AAS, the molarity, concentration and type of alkali activators are key influential parameters. When the W/B ratio is low, 25 silicate ion dissociation gains a higher alkalinity along with the liberation of Ca²⁺, and in turn results 26 27 in fast precipitation of CSH hydration at early stages [61, 62]. This phenomenon accounts for the more 28 rapid strength gains noted for the AAS samples relative to the PC. Increase of W/B ratio from 0.35 to 29 0.50 yielded a lower concentration of alkaline for a given alkaline content and it appears that the 30 dissolution of GGBS needed more time and a lower alkali concentration to occur naturally. This means 31 that a longer duration is needed for AAS hydration product and skeleton formation, as products and 32 the rate of formation around slag grains rely on the availability of ions in the pore solution [63-65]. 33 The work certainly suggests that the effect of W/B ratio is more notable for AAS paste in comparison 34 to PC.

1 Figure 3 shows the variation of porosity for the AAS and PC paste samples with time. It can be 2 seen that the porosity for all samples decreases significantly as age increases, especially before 7 days. 3 Relative to all other samples, the porosity of W/B ratio 0.50 AAS samples remained high until 3 days, 4 dropped by 30% between 3 and 14 days, and stabilised thereafter at a value of around 42%. In 5 comparison, all other samples behaved similarly after 28 days, with porosities around 30%. 6 Unsurprisingly, porosity was affected by extra water in the matrix, as higher W/B ratios tend to produce 7 weaker matrices with higher capillary porosities; a trend particularly apparent for AAS mixes [66, 67]. 8 For the AAS-0.50 samples, porosity reduced rapidly after 3 days suggesting that in contrast to the slow 9 dissolution process, precipitation of hydration products in AAS is relatively fast after saturation is 10 reached. The reason for higher porosity in the AAS-0.50 mixes is that there is a substantially lower 11 amount of gel formed. Similarly, Al-Otaibi [68] reported that decreasing Na₂O dosages in AAS 12 concrete resulted in an increase in porosity. When considering compressive strength and porosity 13 results in combination, while the AAS-0.35 and PC-0.35 mixes had similar porosities after 28 days, 14 the AAS pastes achieved higher strength, indicating stronger hydration products for the AAS systems.

15 Figure 2 and 3 highlight the fact that the kinetics of chemical reaction between GGBS and 16 activator is a complicated process, and that the influence of W/B ratio on strength development and 17 porosity is more notable for AAS. High variability of AAS properties has been widely reported by 18 different researchers [9, 14, 56, 63, 64, 69, 70], with influencing factors including the source, 19 composition and fineness of the GGBS used, the type and dosage of the alkali activator, W/B ratio and 20 the curing regime used. The results obtained in this study suggest that in order to control the quality of 21 AAS, a deep and comprehensive consideration of raw materials, mix proportions and manufacturing 22 process must be given.

23 4.1.2 Electrical responses

Electrical conductivity of bulk samples and pore solutions at different ages for both AAS and PC
samples are shown in Figure 4. As seen in Figure 4 (a), the bulk electrical conductivity for AAS and
PC progressively decreased as age increased. Bulk conductivity of samples saturated with conducting
fluid is controlled by the saturating medium and pore structure parameters including overall porosity
and pore geometry [64, 71, 72]. Ongoing hydration reactions make pores finer and less continuous,
thereby naturally lowering bulk electrical conductivity.

Two distinguishable features can be seen in **Figure 4** (a). The first is that AAS samples had significantly higher conductivity than PC samples at equivalent ages, particularly before 7 days and that AAS sample porosity continuously decreased with curing process until 28 days; at which point similar values to the PC samples were achieved. This trend was particularly true for the AAS-0.50 samples, which exhibited the highest bulk conductivity before 14 days and then similar values as the PC samples at 28 days. Justification for this is a combination of pore structure changes and higher pore solution conductivity contributed by the high Na⁺ and OH⁻ concentrations in the pore solution. Equally,

1 CSH in the AAS exhibits different chemical and morphological characteristics compared with PC, 2 specified with a lower Ca/Si ratio, higher incorporation of Na⁺ and substitution of Al³⁺ for Si⁴⁺ [59, 3 73]. This means the conductivity of hydration products in AAS can vary greatly according to the 4 activator alkalinity and the means through which the alkalis are incorporated into the hydration 5 products. In this study, sodium silicate gels with a lower Ca/Si ratio, where some of the Si⁴⁺ is 6 substituted by Al³⁺ and Na⁺, could play either a charge balancing role or be absorbed into the CSH 7 structure. Hence, the current movements in AAS samples could be accelerated. This is in contrast to 8 PC systems, where hydration products have a relatively insignificant contribution to conductivity [64]. 9 Against this, the electrical conductivity of AAS is much higher than PC at early ages and with 10 increasing time the solid phases and pore tortuosity increases, leading to electrical conductivity 11 decreases [46, 74]. These findings agree well with findings in our previous results [17] and other 12 researchers [74,75].

The second contributing feature is that W/B ratio serves as the key factor affecting the conductivity of both AAS and PC mixes. More liquid will be present in a given sample as its W/B ratio increases, which in turn will result in higher conductivity [76, 77]. In the case of AAS, increasing W/B ratio also restricts the concentration of alkali-ions as the total amount of alkali content is kept constant [78, 79]. Thus, the degree of hydration for AAS with a high W/B ratio diminishes, leading to coarse pores, higher porosity and, therefore, higher conductivity [80].

19 It is well established that the bulk electrical conductivity itself does not adequately represent 20 pore structure characteristics of porous materials, although the relative rate of hydration can be 21 accessed using this parameter. To fully interpret this data, it is necessary to additionally consider pore 22 solution conductivity. Figure 4 (b) shows the conductivity of pore solution extracted from the 23 hardened paste samples. It is apparent that for a given W/B ratio, the pore solution conductivity of 24 AAS was much higher than that observed in PC samples. Due to the differences in pore solution 25 between AAS and PC, it is not surprising to observe this feature, as the pore solution in AAS contains 26 a high concentration of alkali metal cations and hydroxide ions, which is significantly higher than the 27 PC samples, resulting in high conductivity [10, 81-83]. It is also apparent that, in comparison to the 28 PC samples, the bulk conductivity of the AAS samples appeared to vary somewhat with curing 29 duration; albeit that prolonged curing led to decreasing conductivity of the pore solution. For PC 30 samples, alkali concentrations (Na^+ and K^+) increase during the first few days reflecting decreasing 31 amounts of liquid water, and then stabilise to form hydration products including CSH gel, AFm, AFt 32 and calcium hydroxide [27, 84, 85]. For AAS systems, hydration reaction is virtually between the slag 33 particles and the alkali solution, and the initial ion concentration remains extremely high [64, 71, 72]. 34 As GGBS continues to hydrate, the ions in the alkali solution keep reacting with the slag and combining with the hydration products. The sodium silicate solution provides the essential $[SiO_4]^{2-}$ and Na⁺ in C-35 36 (N)-A-S-H gel, yielding large and rapid changes in the ion concentration in pore solution [81, 86]. A 37 strong dependence of W/B ratio 0.50 AAS over the first 7 days provides further evidences of hydration

in AAS, as a low concentration of alkali-activator cannot initiate early polymerisation. Corresponding
 delayed hydration processes were also confirmed by the compressive strength results in Figure 2.

In addition, from **Figure 4** (b) it is clear that the conductivity of pore solution is inversely proportional to the W/B ratio for both AAS and PC samples. This trend correlates well with the variation of ion concentration caused by W/B ratio.

6 4.1.3 Pore size distribution

7 Cumulative pore volumes for AAS and PC samples at different ages (3, 7 and 28 days), as measured 8 by MIP, are provided in Figures 5(a) and (c) respectively. In order to identify pore structure variations, 9 pores were classified into size categories <10, 10-150 and >50 nm as shown in Figure 5(c). It can be 10 seen that cumulative volumes of mercury intruded (porosity) for both AAS and PC samples decreased 11 as the curing duration increased, and both paste types had similar 28-day cumulative pore volumes. 12 Further, another three interesting features can be identified: 1) from 3 to 7 days, pores becomes finer, 13 especially for those between 5 and 20 nm, while the overall porosity did not change significantly; 2) 14 between 7 and 14 days an overall decrease in porosity was found, especially for pores of 10 nm; 3) 15 from 14 to 28 days the overall porosity was roughly the same and the pores become finer, particularly 16 for pores beyween 4 and 8 nm.

17 From early ages, uniform gels fill initially water-filled spaces in AAS and the material gradually 18 densifies as reactions proceed. In comparison to AAS, pores in PC systems start at around 100 nm, 19 which are much coarser than that for AAS. With that said, no significant variations in porosity between 20 AAS and PC samples were found beyond 14 days. The main change occurred in the 2-50 nm pore 21 range, while no reduction of pores between 50 and 100 nm was found at the age of 28 days. The data 22 sets in Figure 5 clearly demonstrate that PC systems have a continuous pore size distribution within 23 the range 5 to 1000 nm at 28 days, whereas AAS systems predominantly contain pores either in the 24 <10 or greater than 200 nm ranges. This indicates that in terms of microstructure, AAS systems have 25 a much denser pore structure, albeit that their total porosity within the capillary range studied are 26 similar to PC systems. Existence of increased amounts of smaller pores is related to the fact that no 27 visible inner product ring formation occurs in AAS systems, but rather a dense mass matrix that leads 28 to high strengths, as shown in Figure 2.

29 The peak in the differential pore volume-pore size relationship suggests where the critical 30 (percolating) pore size is in the system. It should be clarified that the indication of threshold pore 31 diameter is ideally not an absolute value, but a parameter to facilitate comparisons between specimens, 32 and which can also be derived from the differential pore volume-pore diameter relationships [87-90]. 33 Beneath this size, the pore system will be de-percolated and, hence, this parameter is very useful for 34 assessing transport properties [31, 34, 91]. A comparison of threshold pore sizes between AAS and PC 35 systems is shown in Table 2. Clearly, the critical pore radii for AAS-0.35 samples are considerably 36 smaller than for PC-0.35 samples. It was also found that both AAS and PC systems have shown a decreasing trend as curing duration increased and barely any variation can be detected in the threshold pore diameter after 14 days, suggesting that the microstructure is less affected beyond this point. Further investigation into pore structure characteristics for AAS and PC was focused on examining the influence of W/B ratio. **Figure 6** compares the 28-day MIP results for AAS and PC samples at W/B ratios 0.35 and 0.50. It can be observed from **Figure 6** that porosity and pore size distribution are significantly affected by W/B ratio and binder type, with higher porosity and coarser pores associated with the higher W/B ratio AAS and PC samples.

Another noteworthy feature is that the porosity of the AAS samples increased from 22.9 to 44.2%
as the W/B ratio increased from 0.35 to 0.50. Analysis of pore size distribution for AAS-0.50 samples
highlights a marked increase in the number of pores with diameters between 10 and 100 nm, while the
number of fine pores (diameter between 2 and 10 nm) remained similar to the AAS-0.35 samples.
Moreover, the 28-day porosity of AAS-0.50 samples was much higher than the reference PC samples.
A continuous pore size distribution over the measured 2-1000 nm range was detected for AAS-0.50
samples, while only <10 nm pores were observed in the reference PC samples.

As explained before, an increase in W/B ratio dilutes the activator concentration which slows the hydration action rate and precludes high strength at very early ages (lower strength at 3 days for both AAS samples compared to PC equivalents). Combined with the above AAS-0.35 findings where AAS samples were found to have a fine pore structure albeit similar porosity compared with reference PC samples, it can be concluded that W/B ratio plays a more profound role in microstructure formation for AAS than for PC systems.

21 4.2 Estimation of property variation using microstructural characteristics

22 4.2.1 Pore formation factor and chloride diffusion coefficient

23 Pore formation factors and chloride diffusion coefficients were assessed using Eq (2) and (3) and the 24 corresponding results are plotted in Figure 7. For both AAS and PC samples, the pore formation factor 25 increased over the entire curing period considered, reflecting the fact that as capillary pores decrease 26 in volume and size, they start to disconnect because of increases of hydration products [4, 27]. Another 27 interesting point is that the performance of AAS samples was strongly dependent on W/B ratio. AAS-28 0.35 samples returned similar or even lower pore formation factors than corresponding PC samples, 29 but a much higher value was found when W/B ratio increased to 0.50. Several reasons can be employed 30 to explain this phenomenon, including the amount of reaction products and difference in pore 31 structures. The MIP results shown in Figure 6 suggest that the AAS-0.35 samples produced a denser 32 microstructure. A comparative analysis of previously obtained SEM results indicate that the hydration 33 products in AAS are amorphous and uniformly distributed throughout space inside specimens [92, 93]. 34 Thus, it is perhaps not surprising to obtain high formation factors for AAS-35 samples. However, this 35 is not always valid for the mix with high W/B ratio. This is partially attributed to differences in porosity variation, as shown in Figure 3, and partially because of pore connectivity (β in Eq (1)) [41, 94, 95]. 36

A detailed analysis of this phenomenon has been presented in a previous related study [78]. It is also evident in **Figure 7** that AAS samples exhibited different behaviour in the changing rate of pore formation factor compared with PC, which is affected by the W/B ratio. The pore formation factor for AAS samples still had not stabilised after 28 days, whereas for PC samples no significant variation was detected after 7 days. This discrepancy clearly reflects differences in the chemical reactions occurring in AAS and PC systems

7 Figure 8 shows the chloride diffusion coefficients determined using the pore formation factor 8 via Eq (3). As expected, diffusion coefficients for AAS and PC samples and their evolution over the 9 curing period were generally similar; consistently decreasing with increasing curing duration. AAS 10 pastes were clearly more sensitive to changing W/B ratio, with only minor decreases in chloride 11 diffusion observed for the AAS-0.35 samples and no significant variations observed after 14 days. In 12 contrast, the AAS-0.50 samples exhibited more significant and constant decreases in chloride diffusion 13 coefficient over 28 days. For the PC samples, at both W/B ratios the diffusion coefficients stabilised 14 after 7 days. This observation shows consistency with results obtained from other researchers [74, 96] 15 and is similar to the influence of the pore formation factor explained above.

16 According to work published by McCarter et al [97], more in-depth analysis can predict temporal 17 variations of chloride diffusion coefficient, with the following equation applied to model obtained 18 diffusion coefficients at time *t* (in days), denoted by $D_i(t)$:

19
$$D_i(t) = C \times \left(\frac{D_0}{F_{ref}}\right)^n \times \left(\frac{t_{ref}}{t}\right)^m$$
 Eq 10

20 where: C, n and m are constants; D_o is the diffusion coefficient of the ion (e.g. Cl⁻) in the free electrolyte at 25°C, 2.03×10⁻⁹ m²/s; F_{ref} is the formation factor at a reference time (d); t_{ref} is the 21 22 reference time in days; and m is an exponent related to hydration and pozzolanic reactions. The 23 reference time for the current work was taken as 3 days ($t_{ref} = 3$ days) and F_{ref} values are as shown in 24 Figure 8. The three unknown numbers were obtained through multi-parameter nonlinear regression 25 analysis. During analysis of the diffusion coefficient results, three factors needed to be taken into 26 account; namely the influence of ions in pore solution, the pore geometry and the binding capacity of 27 hydration products [96, 98, 99]. These are discussed further in the following paragraph.

28 For AAS systems, more ions are inherently present in the pore solution. In particular, Na⁺ ions 29 are easily leached from specimens during their service life [100]. Within this process, negatively 30 charged ions would naturally move to maintain electro neutrality; a process that potentially could slow 31 down Cl⁻ ion ingress into the specimen. The second factor to be considered are the microstructural 32 differences between AAS and PC systems. It has been reported [67, 101, 102] that microcracks 33 frequently occur within AAS-based materials, with the shape of pores reflecting the oval to slot like 34 shape of microcracks. The results indicate that it is unlikely for AAS systems with a W/B ratio of 0.50 35 to reach the de-percolation threshold under the exposed curing conditions due to limited hydration. Lastly, differences in physical and chemical binding between PC and AAS systems is the third factor
 that requires attention. Established results [58,75,68] have shown that the binding capacity of AAS
 samples is much higher than for those made with PC due to a lower Ca/Si ratio and relatively higher
 Al content; characteristics beneficial to resist chloride ingress. Therefore, it is reasonable to observe
 different regression constants for both AAS and PC samples.

6 To verify the reliability of the proposed equation, correlations between calculated and measured 7 chloride diffusion coefficients for AAS and PC samples at different curing durations were compared 8 (see **Figure 9**). As exhibited, a strong correlation was found at different ages, despite the 9 aforementioned additional influences of ions in pore solution, pore geometry and binding capacity of 10 hydration products [67, 96, 98-102]. As such, the evaluation of diffusion coefficients using this 11 methodology was concluded to provide a good estimate of likely performance in practice.

12 4.2.2 Permeability coefficient

13 Plotted in Figure 10 are permeability coefficients for W/B 0.35 mixes estimated using the Katz-14 Thomoson (KT) and Millington-Quirk (MQ) models according to Equations (4) and (5) respectively. 15 Clearly, permeability coefficients for both AAS and PC samples generally reduced gradually over 16 curing time. For both sample types the majority of reductions occurred during the first 14 days, after 17 which performance stabilised. This finding agrees with the outputs of previous research [28, 103] and 18 may be justified due to the fact that variation of permeability is related to the pore structure of reaction 19 products and pore connectivity. As previously noted, for both PC and AAS samples beyond 14 days 20 there was no significant variation in porosity (see Figure 3), pore structure (see Figure 5), critical pore 21 size (see Table 2) or formation factor (see Figure 8). The key feature evident from Figure 10 is that 22 the AAS samples exhibited significantly better performance in terms of permeability than the PC at 23 W/B ratio 0.35.

24 No constant conclusion can be drawn from previous studies as conflicting results in this regard are reported in the literature. Davidovits [104] found the permeability of PC system to be 10⁻¹² m/s 25 (10^{-19} m^2) in comparison to 10^{-11} m/s (10^{-18}m^2) , i.e. one order higher, for alkali-activated materials. 26 27 Sagoe-Crentsil et al [105] studied the water and gas permeability of AAS and PC concrete and found 28 that while gas permeability of AAS and PC were similar (6.19×10^{-17} and 6.32×10^{-17} m² respectively), AAS showed a 10-fold increase in water permeability $(1.56 \times 10^{-17} \text{ versus } 1.77 \times 10^{-18} \text{m}^2)$. In previous 29 30 work related to this study [67], sorptivity and air permeability were studied and it was found that AAS 31 concrete exhibited much higher sorptivity and air permeability for a given compressive strength grade. 32 On the other hand, Zhang et al. [106] and Nasvi et al [107, 108] conducted measurements of 33 permeability for AAS concrete and reported lower values than those for PC concrete. Similar findings 34 were also reported by Ko et al [109].

Paste AAS and PC samples have a primary porosity different to mixes comprising aggregates
 (mortar or concrete), which generally exhibit secondary porosity related to interfacial transition zones

1 [110]. As such, permeability results for paste samples should be closely linked with the microstructure 2 features determined by MIP. The influence of cracks also needs to be considered here. It has been 3 reported [67, 101, 102, 110] that considerable microcracks have been observed for AAS concrete, and 4 especially within aggregate-paste interfaces; a phenomenon undoubtedly influencing permeability. 5 Meanwhile, the variability and properties of raw materials and mix proportions also affects 6 permeability results. As such, it is not hard to understand opposing trends being reported in the 7 literature. The observed results in this study highlight the fact that AAS can achieve low permeability; 8 a finding which has considerable practical significance. However, this cannot be the case for AAS with 9 high W/B ratios.

10 It is also worth noting that permeability coefficients derived from the two models cannot be equal. 11 More specifically, permeability coefficients estimated using the KT model are one order lower than 12 those using the MQ model. The KT model is built on the fact that the rate of flow between two pores 13 of different sizes is controlled by the pore of the smallest size, or the so-called neck. This presupposes 14 a perfect fit between the two pores, which rarely exists in reality. It is assumed that the pores and necks 15 are circular in cross section, although it is shown that they have a rather angular shape [30, 31]. Also, 16 another assumption is that only one neck arises in sequence with another neck in the same pore. Errors 17 generated from these assumptions at least partially offset against other errors by neglecting the 18 widening pores. The threshold pore method used in the KT model does permit a rather rapid calculation 19 of permeability, but the accuracy and reliability of this model should be questioned. As shown in this 20 study, the KT model could give misleading results when assessing the permeability of AAS with a 21 W/B ratio of 0.50. Figure 11 shows the estimated 28-day permeability values for AAS and PC samples 22 with W/B ratios 0.35 and 0.50. It can be seen that the two models yielded a higher permeability 23 coefficient as the W/B ratio increases, but a different trend was obtained when the W/B ratio was 0.50. 24 In other words, AAS is less permeable than PC in the case of KT model, while an opposite conclusion 25 is given if the MQ model is applied. As indicated in the above section, AAS with high W/B ratio has 26 a coarse and continuous pore structure and high porosity, which would yield a higher permeability 27 coefficient. Therefore, it may be concluded that the KT model may not be suitable for estimating the 28 permeability of AAS, as only threshold diameter considerations do not fully represent the complexity 29 of its pore structure in reality.

30 4.2.3 Compressive strength

In Figure 12, the porosity of AAS and PC samples is plotted against compressive strength. It can be seen that all pastes are porous bodies with porosities ranging between about 20% and 55%. As porosity levels mainly originate from the fraction of mixing liquids, it is expected that increasing compressive strength is a consequence of porosity and W/B ratio decreasing. Considering the variability of the data, the relationships between two parameters is good. Furthermore, a general trend between compressive strength and porosity can be found for AAS and PC samples. This is easy to understand because the

1 strength of both binders comes from the CSH [111, 112], although the ratio of crystalline and Ca/Si in 2 these hydration products is different. The four strength-porosity relationships discussed in section 2.4 3 were used to predict the experimental results, where non-linear regression analysis was applied to 4 obtain the coefficients for AAS and PC in Eq (6-1) to Eq (6-4). From Figure 12 it is clear that the 5 correlation coefficients of regression constants ranged from 0.662 to 0.948 for the four modules. 6 Compared to Balshin's and Ryshkevitch's equations, Schiller's and Hasselman's equation appear to 7 be less accurate. This conclusion differs with some studies, e.g. Shi [91], which has shown an opposite 8 observation. A possible reason explaining this situation is the type of GGBS used in the different 9 studies, as it has been shown that GGBS-type dominates responses of AAS cement materials. In 10 addition, different methods used to determine porosity may be another reason for this observation. 11 Porosity reported in this study was determined at 40 °C (selected to avoid additional changes in 12 microstructure and hydration products during drying [27, 113, 114]), instead of at 105 °C for the 13 aforementioned study.

14

5. Conclusion

In this study, the microstructural characteristics and electrical responses were used to assess and predict the performance of AAS cement. The compressive strength, permeable porosity, bulk conductivity, pore solution conductivity and pore structure of AAS and corresponding reference PC paste samples were measured. Based on the results obtained, and following conclusions can be drawn:

- It has been found that classical theories used for predicting the performance of PC through microstructural characteristics and electrical responses can be applied to assess AAS cement. Pore formation factor, chloride diffusivity and permeability were estimated along with an evaluation of different porosity-compressive strength models
- 2. The results show that the chloride transport coefficient can be reliably predicted using electrical
 responses. According to the regression analysis, no empirical equation can be established between
 AAS cement and PC, and hence it is recommended to establish an equation for the material for
 the specific condition.
- Previous studies have highlighted that the AAS cement has refined pore structure with a relatively
 narrow pore size distribution. It is found that the general trend is only valid when the W/B ratio is
 low, while this feature cannot be observed for the cement paste with a high W/B ratio. It suggests
 that the assumption of AAS will have a denser pore structure than that of PC for a given W/B is
 not safe.
- Analysis of temporal variation of formation factor indicates that the AAS cement has a relatively
 constant growth rate until 28 days, while the formation factor of PC intends to increase rapidly
 before 7 days and after this, the variation is relatively small. This feature highlights the importance
 of continuous curing for AAS cement, as normal curing regime for PC would not be sufficient to

ensure its quality. Meanwhile, the high sensitivity to the W/B ratio is clearly seen from the
 performance range obtained. It means that AAS has the potential to achieve super durability, but
 this binding system would require more care on manufacturing techniques.

4

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12

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1	Figure and Table
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Table 1 Chemical composition of GGBS and PC (% by mass)

Composition	SiO ₂	Al_2O_3	Fe ₂ O ₃	MgO	CaO	Na ₂ O	K ₂ O	Loss
GGBS	31.63	13.42	1.32	9.12	36.35	0.34	0.46	0.61
PC	21.33	5.80	2.57	2.41	60.21	0.70	0.21	

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Table 2 Temporal variation of threshold diameter of AAS and PC

Curing duration	AAS-35	PC-35
(day)	(nm)	(nm)
3	11.0	62.4
7	9.1	62.4
14	6.0	50.3
28	5.2	50.3

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Figure 3 Accessible porosity of AAS and PC pastes





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and PC pastes; (d) comparison of pore proportion based on the classification in (c).



Figure 6. Comparison of the pore size distribution of AAS and PC pastes with 0.35 and 0.50 W/B ratios at 28 days



Figure 7. Temporal variation of formation factors for AAS and PC pastes



Figure 8. Temporal variation of Cl⁻ diffusion coefficient for AAS and PC pastes



Figure 9 Comparison between measured and estimated Cl⁻ diffusion coefficients



Figure 10 Comparison of estimated permeability coefficients for AAS and PC pastes



Figure 11 Permeability coefficients of AAS and PC pastes with different W/B ratios estimated by MQ and KT models



Porosity, φ
 Figure 12. Relationship between porosity and compressive strength of AAS and PC pastes

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