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The
University
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Department of Civil and Structural Engineering

A Novel Friction-wall System for Seismic Strengthening of Substandard RC Buildings

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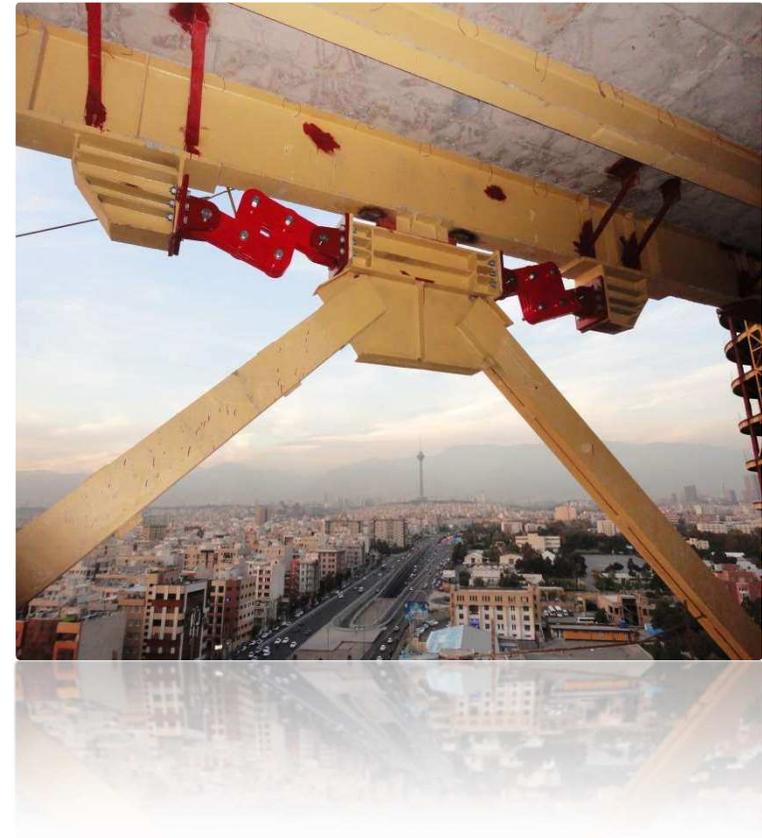
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February 2020



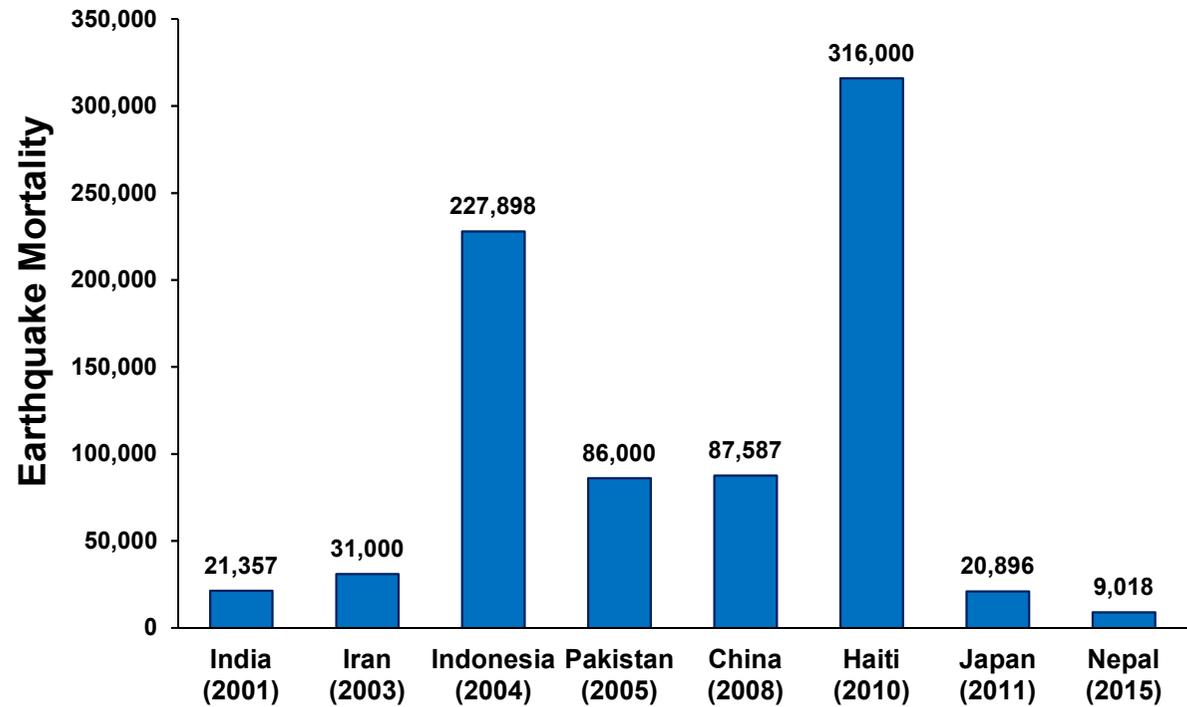
Progress Report



- Introduction
- Proposed Friction-Wall System
- Practical Design Solution
- Performance-based Optimisation
 - Single-objective optimisation
 - Multi-objective optimisation
- Summary and Conclusions



Bam Earthquake 2003, Iran



- Severe damage and extensive mortality
- Need for efficient and cost-effective strengthening solutions



Friction-based Energy Dissipation Systems

➤ Brace-type dampers

- Stress concentration
- Blocking the openings



➤ Wall-type dampers

- Less stress concentration
- Higher energy dissipation
- Larger Opening
- More adjustable





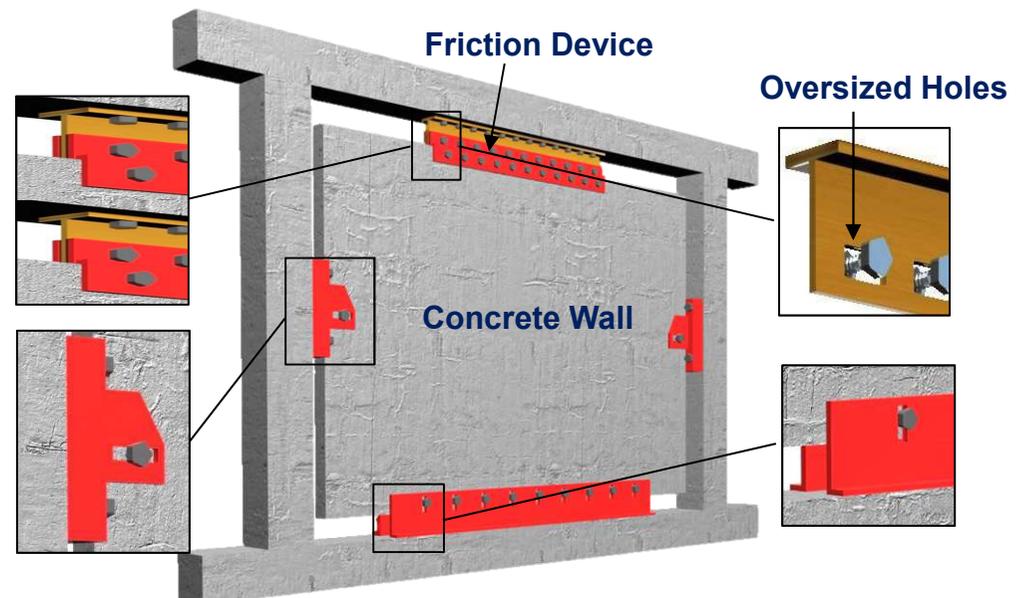
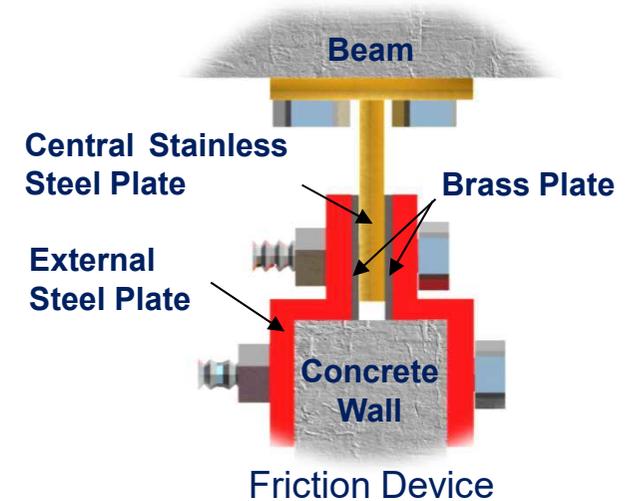
Proposed Friction Wall Damper:

➤ Components:

- Concrete wall panel
- Vertical supports to columns
- Horizontal support to lower floor beam
- Friction connection to upper floor beam

➤ Advantages:

- No brittle shear failure
- No stress concentration
- Easy assembly
- Easy adjustment





Geometry of the reference RC frames equipped with friction wall dampers



Prototype Slip Load Distribution Patterns:

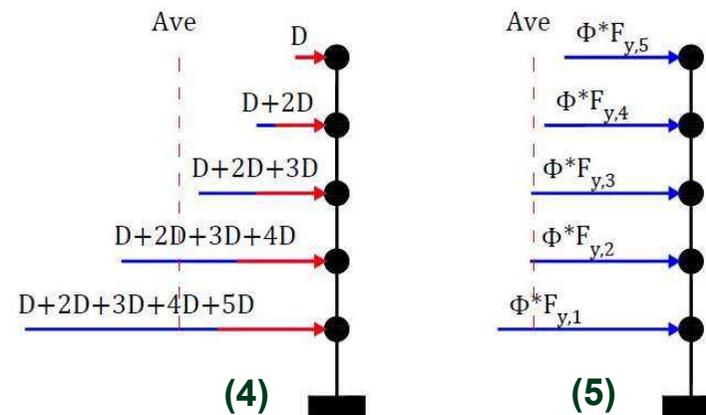
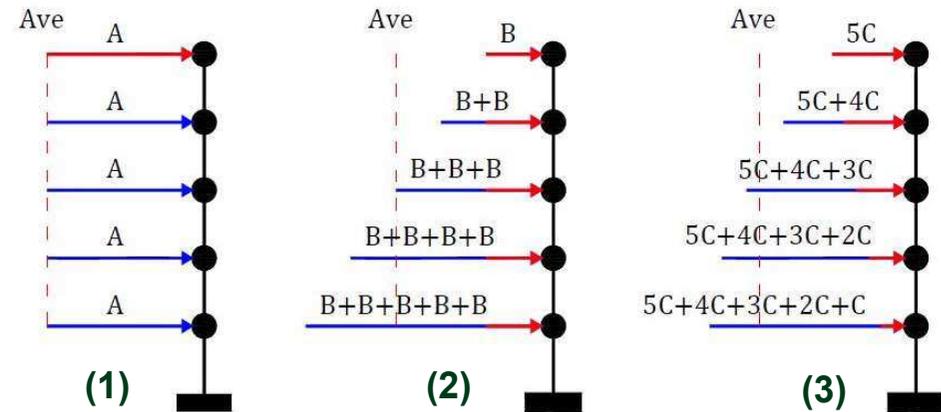
1) Uniform (Conventional design)

2) Uniform Cumulative

3) Triangular Cumulative

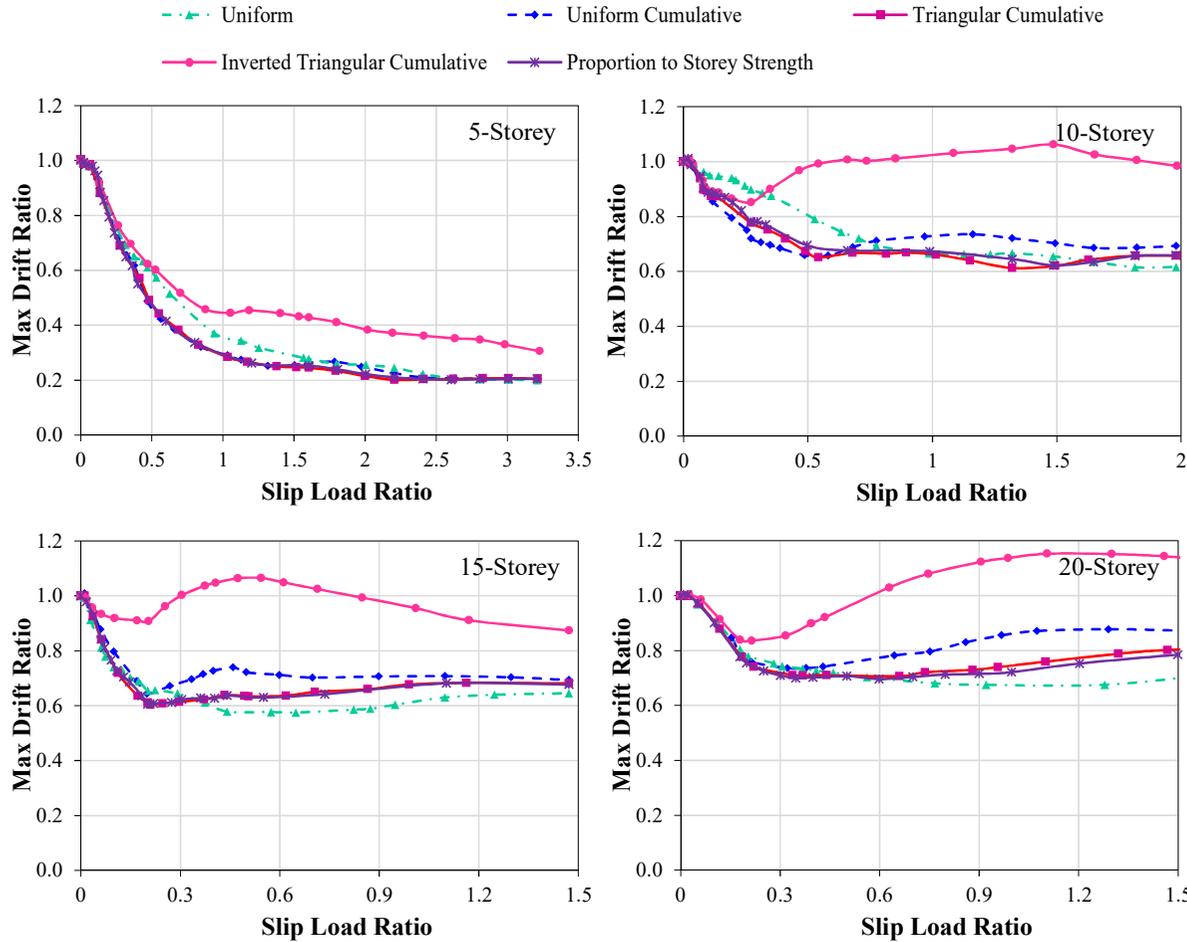
4) Inverted Triangular Cumulative

5) Proportional to the Storey Shear Strength ($F_{y,i}$)





Displacement Demand



Slip load ratio:

$$F_{SR} = \frac{\sum_{i=1}^n F_{S,i}}{\sum_{i=1}^n F_{y,i}}$$

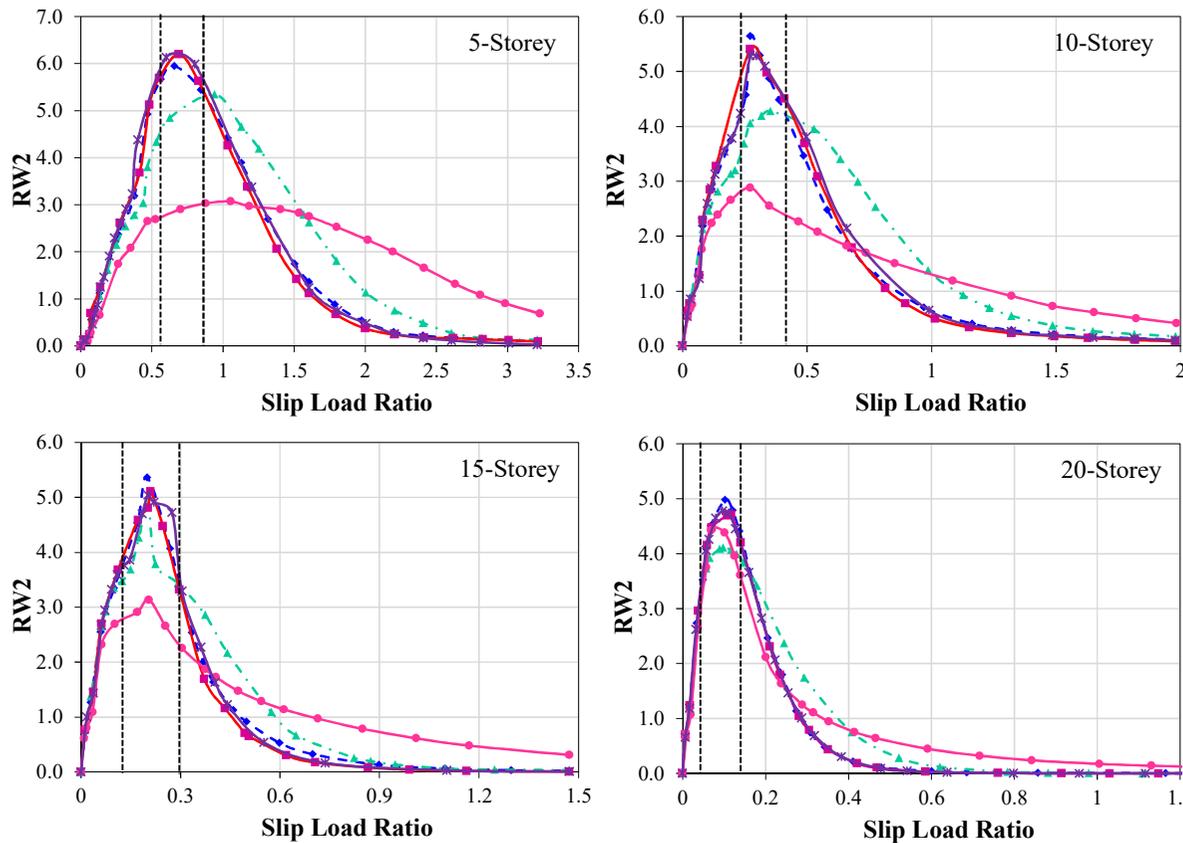
F_{SR} : slip Load Ratio
 $F_{S,i}$: slip load of the i^{th} storey
 $F_{y,i}$: storey shear strength

Optimum range?



Energy Dissipation Capacity

--△-- Uniform - - - Uniform Cumulative -■- Triangular Cumulative
-●- Inverted Triangular Cumulative -×- Proportion to Storey Strength



Energy parameter:

$$R_{w2} = \frac{W_{sf}}{(W_{sb} + W_{sc})_{cs}}$$

W_{sf} : work of friction devices
 W_{sb} : static work of beams
 W_{sc} : static work of columns

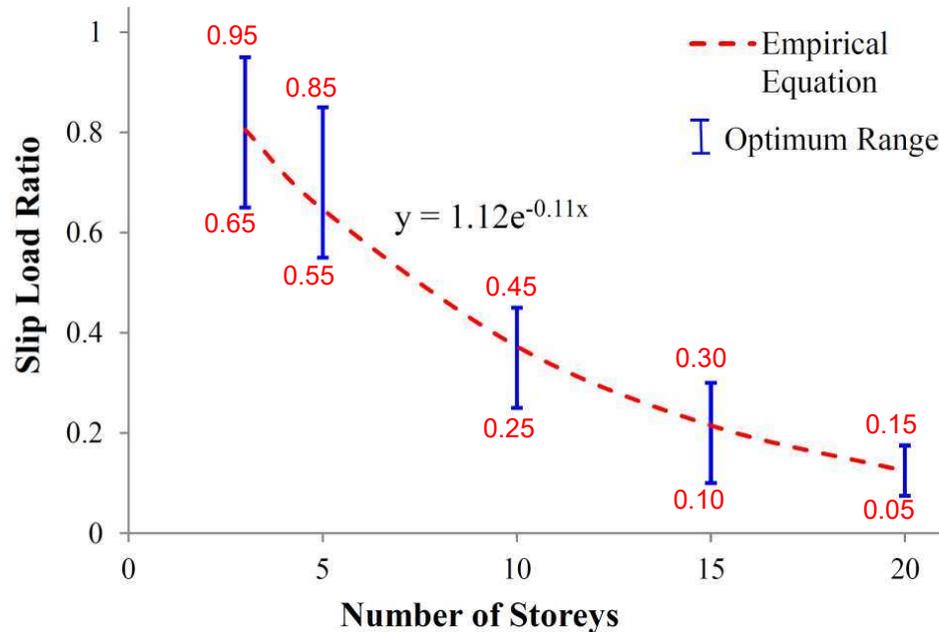
There is an optimum range for slip load ratios that, on average, leads to maximum energy dissipation.

Variation of R_{w2} as a function of slip load ratio, average of six selected earthquakes



Design Equation

Based on the optimum slip load ranges obtained from different seismic performance indices, an empirical equation is proposed for more efficient design of friction-based wall dampers.



Best Slip Load Ratio

$$R = 1.12e^{-0.11n}$$

R : best slip load ratio
 n : number of storeys

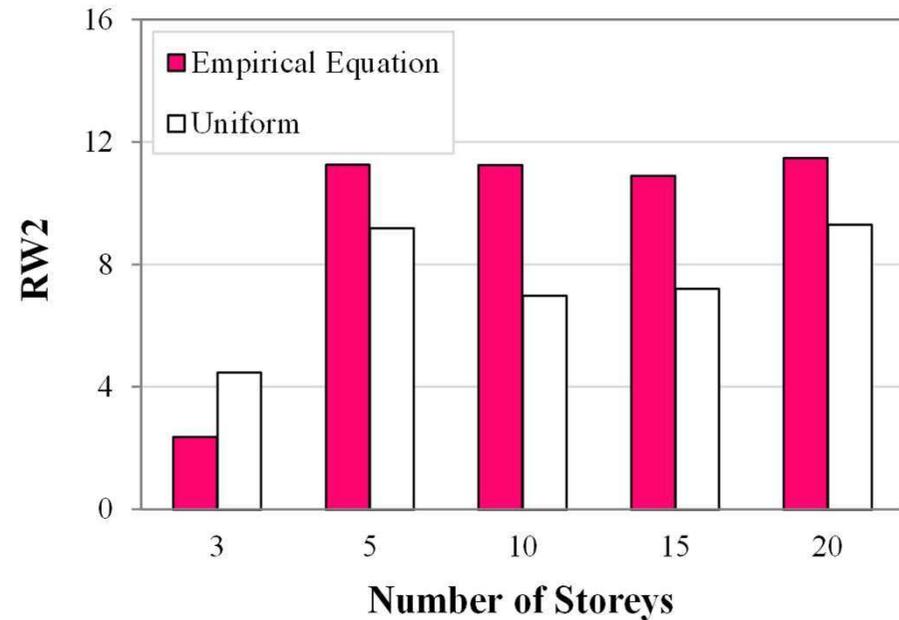
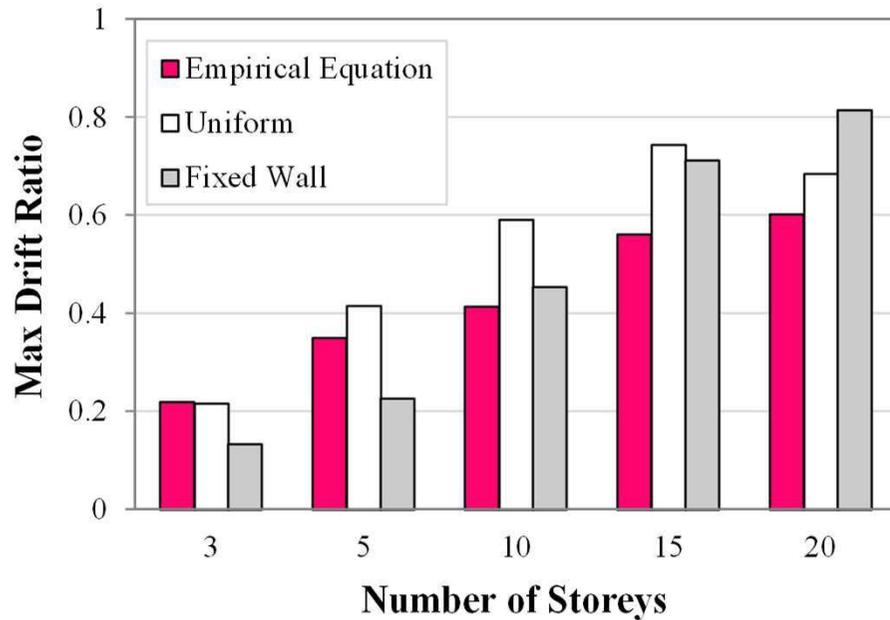
Load Distribution

$$F_{S,i} = \frac{1.12e^{-0.11n} \times (n+1-i)}{n(n+1)/2} \times \sum_1^n F_{y,i}$$

$F_{S,i}$: slip load of the i^{th} storey
 $F_{y,i}$: storey shear strength



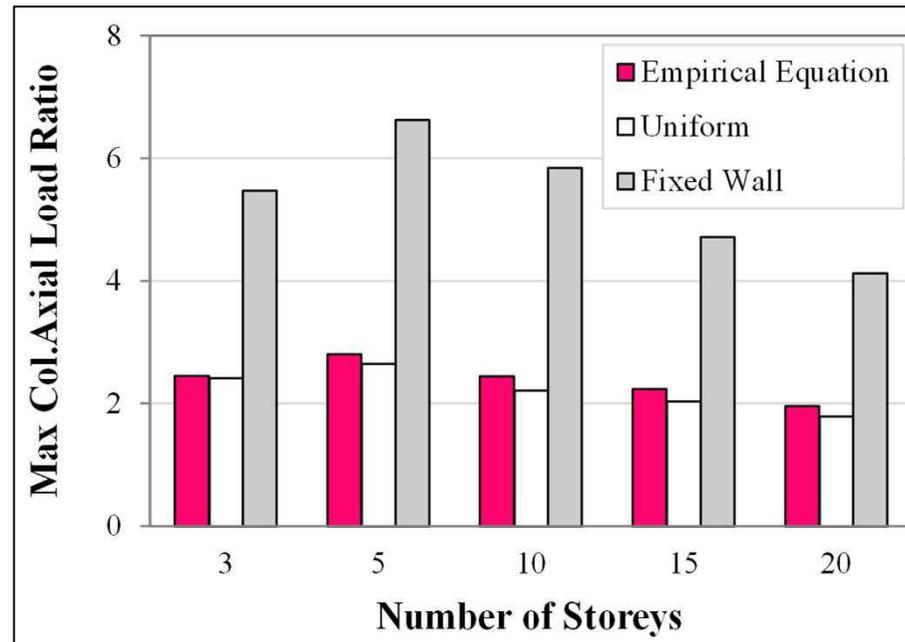
Efficiency of the proposed practical design solution:



By increasing the energy dissipation, maximum inter-storey drift is reduced compared to conventional design method



Efficiency of the proposed practical design solution:

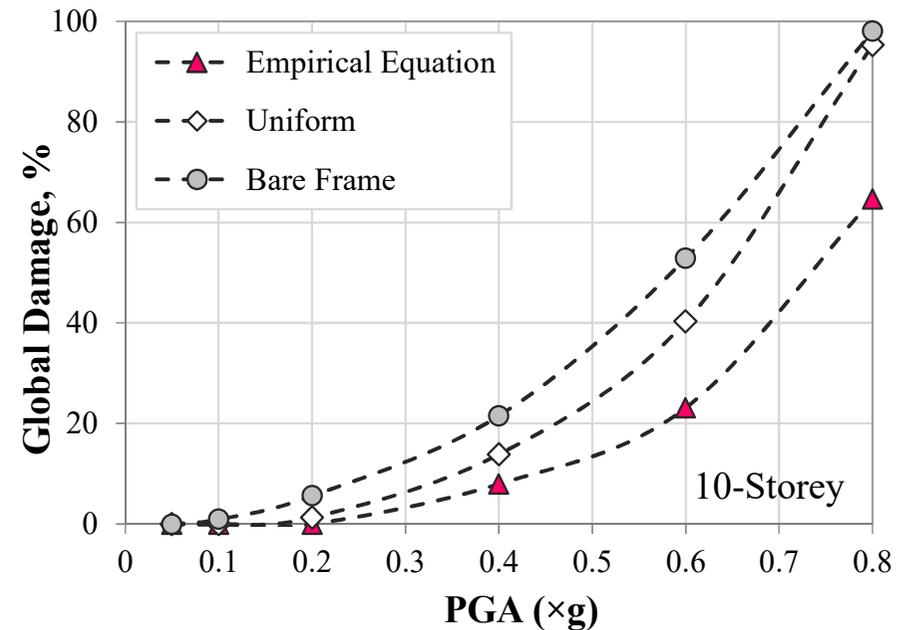
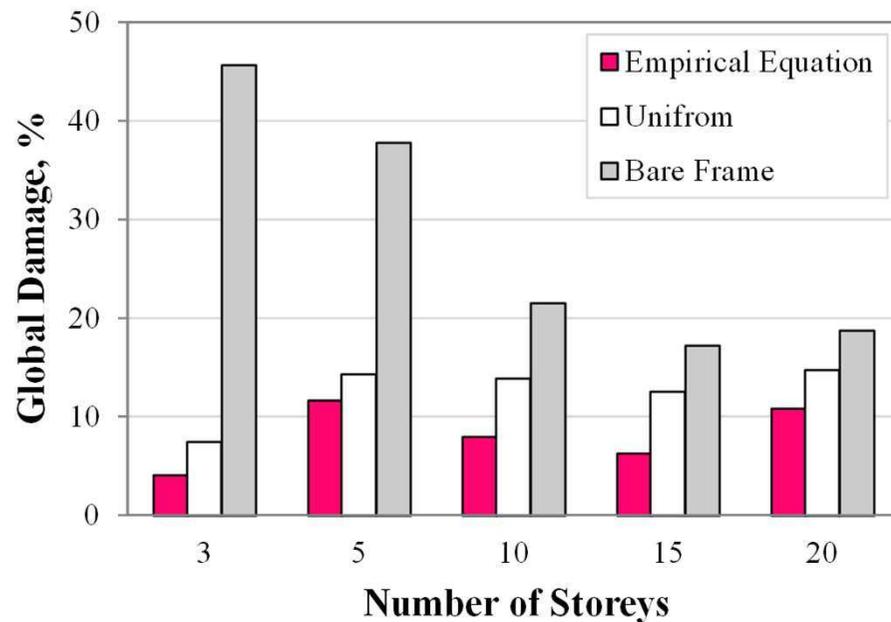


Axial load in the columns is significantly reduced



Cumulative Damage Index

- Using the proposed design solution leads to significant reduction (up to 83%) in the global damage index compared to the conventional design.
- The proposed equation is efficient at all PGA levels.



10-storey frame, A set of six synthetic spectrum compatible earthquakes



An optimisation method based the concept of **Uniform Damage Distribution** was adopted to obtain the best slip load distribution.

Proposed algorithm:

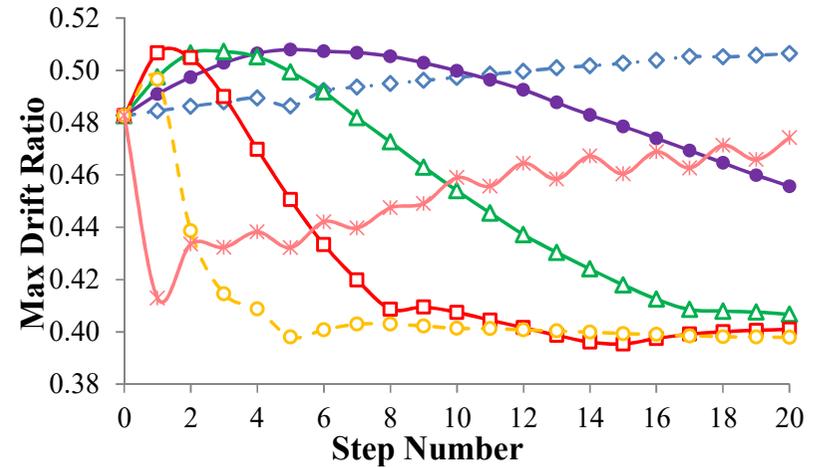
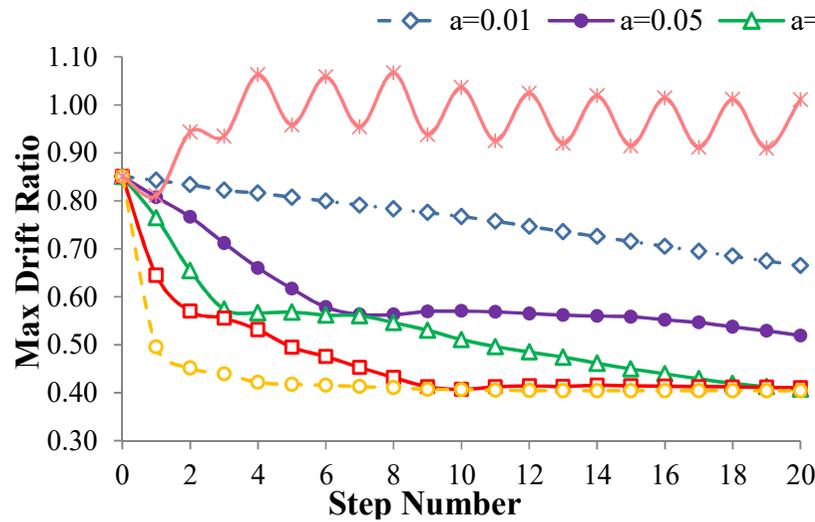
- 1) Initial slip load
$$F_{S,i} = \frac{1.12e^{-0.11n}}{n} \times \sum_1^n F_{y,i}$$

n: Number of storeys
i: Storey number
- 2) (m + 1)th slip loads
$$(F_{S,i})_{m+1} = (F_{S,i})_m \times \left(\frac{\Delta_i}{\Delta_{ave}} \right)_m^\alpha$$

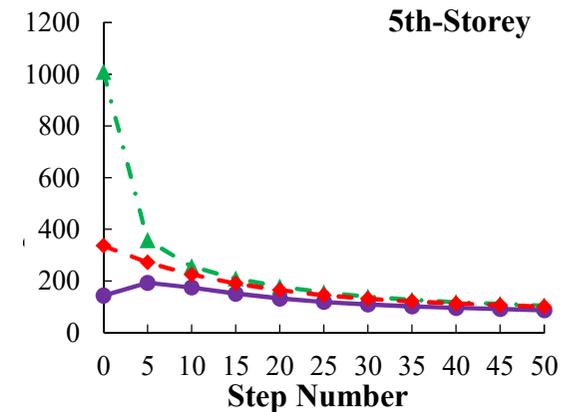
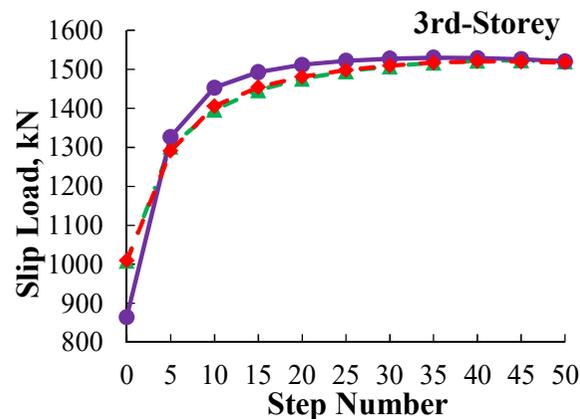
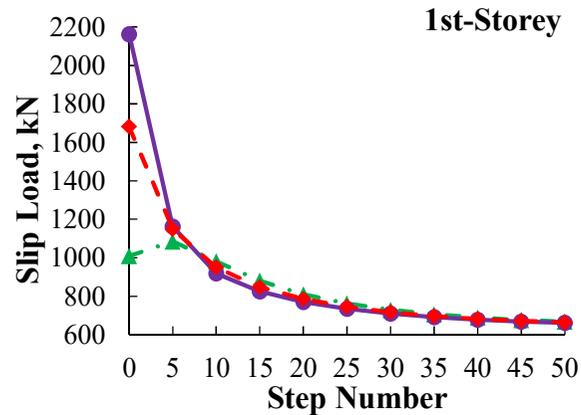
Δ : Maximum drift
m: Number of steps
 α : Convergence factor
- 3) Scaled to the average
$$\left[(F_{S,i})_{m+1} \right]_{scaled} = \left(\frac{\sum (F_{S,i})_0}{\sum (F_{S,i})_{m+1}} \right) \times (F_{S,i})_{m+1}$$
- 4) Covariance of drift
$$(COV_\Delta)_m = \left(\frac{\sqrt{(Var_\Delta)_m}}{(Ave_\Delta)_m} \right) \times 100$$



➤ Effect of convergence parameter:



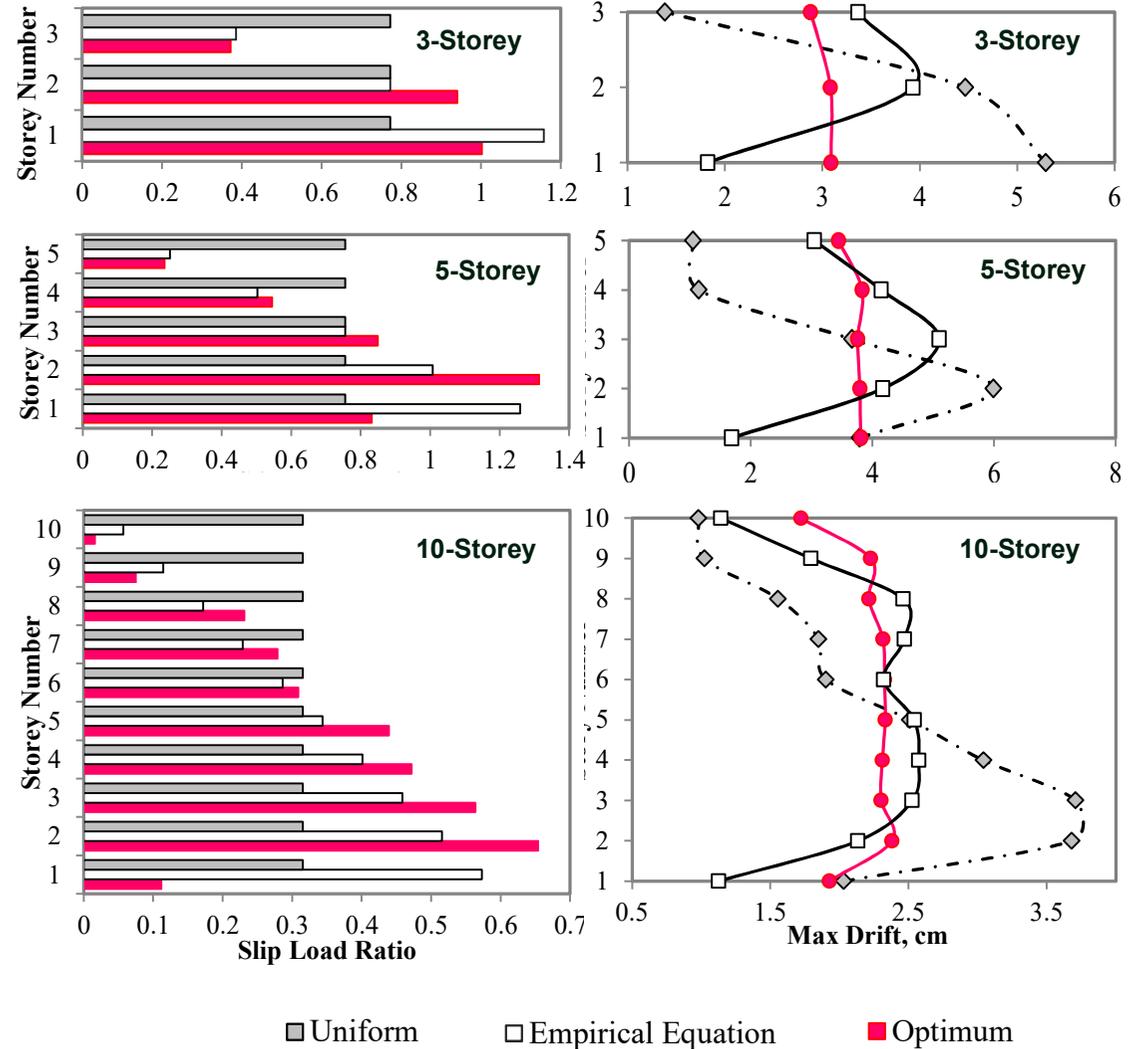
➤ Effect of initial slip load distribution:





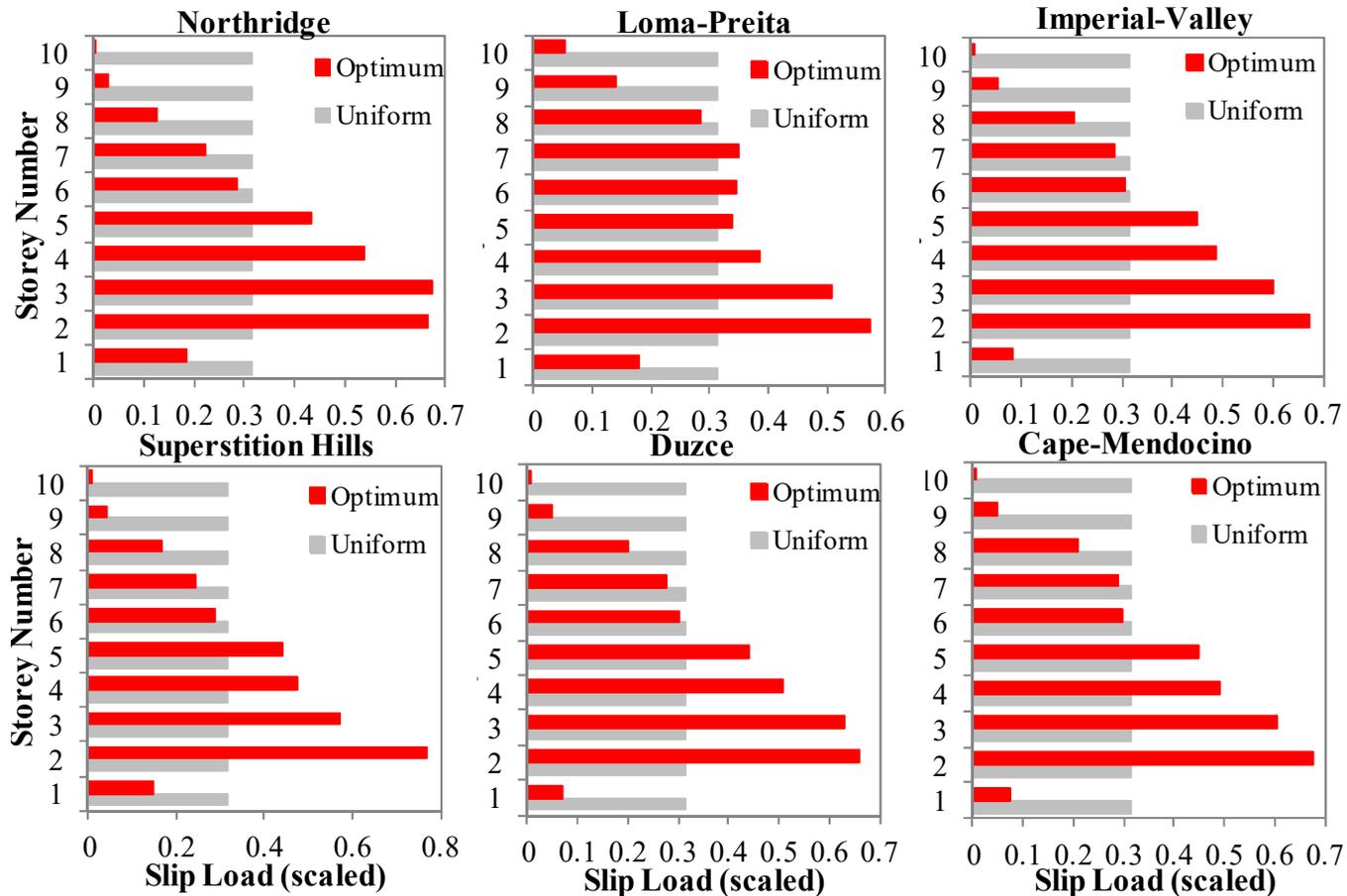
Optimum design for synthetic spectrum-compatible earthquakes:

- 1) Up to 45% less maximum drift ratio
- 2) Avoiding damage localisation and soft-storey failure
- 3) Removing unnecessary friction wall dampers





➤ Effect of design earthquake:

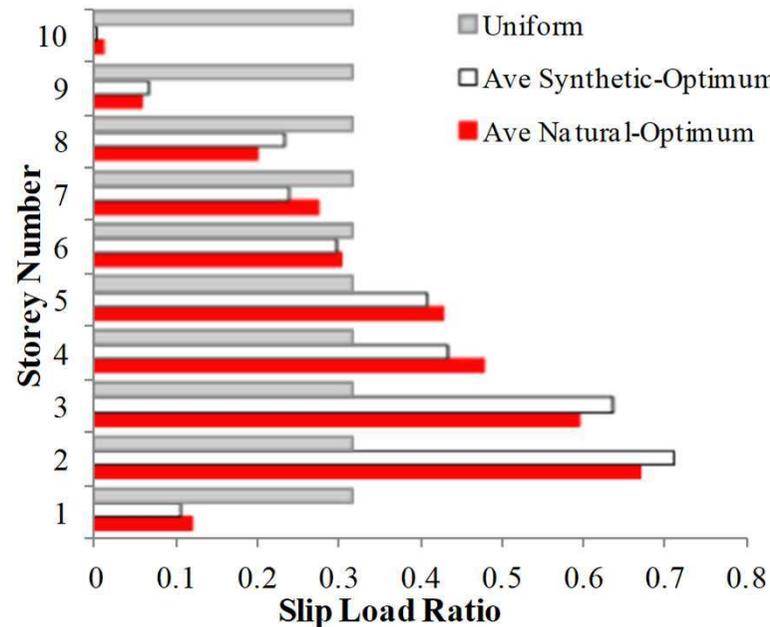


Comparison between optimum and uniform distribution of slip loads (scaled to the average storey shear strength) for 10-storey frames under six natural earthquakes



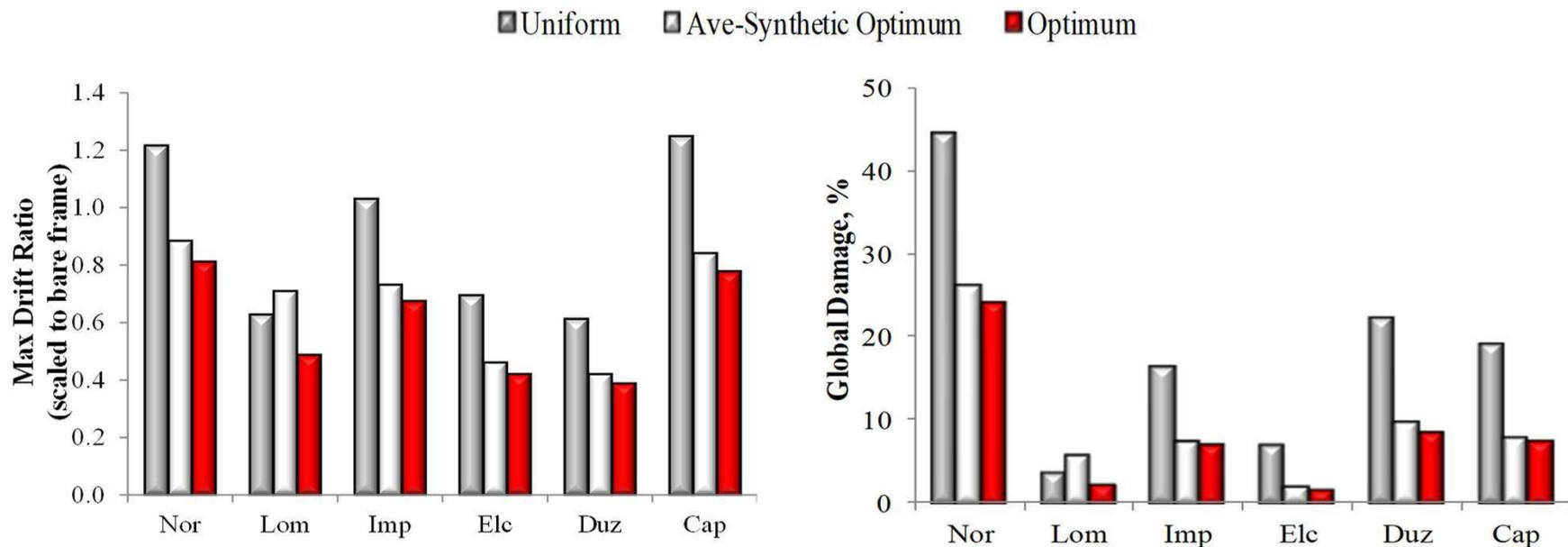
➤ Optimum Design Solution for a Code Design Spectrum

The optimum slip load distributions corresponding to the natural and synthetic earthquakes are almost identical. This implies that there is a unique optimum design solution for each frame subjected to the design spectrum.





➤ Optimum Design Solution for a Code Design Spectrum



Average of optimum slip load patterns for a set of a spectrum compatible earthquakes can be used for practical design purposes



- A practical performance-based design methodology was proposed for optimum design of RC frames using friction-based wall dampers and the computational efficiency of the method was demonstrated.
- Optimum design friction-based dampers could increase the energy dissipation capacity (by up to 61%) and decrease the maximum drift ratio (by up to 30%) compared to the conventional design solutions.
- Using the proposed practical design equation resulted in up to 50% lower global damage compared to the conventional design solutions.
- The seismic load uncertainty can be efficiently managed by using the average of optimum load patterns corresponding to a set of synthetic earthquakes representing the design spectrum.



Optimum Design of Friction-Walls

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