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Assessing the uniaxial compressive strength of extremely hard cryptocrystalline flint

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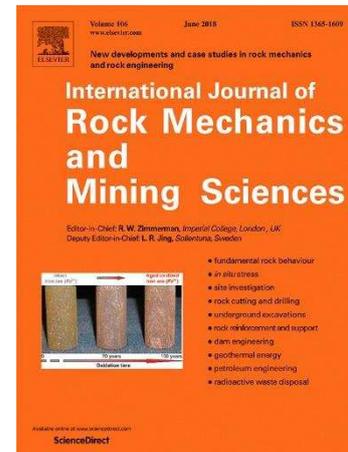
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36 **Keywords:** Flint; Uniaxial compressive strength; Empirical estimation; Drilling;
37 TBM tunneling

38 **Highlights**

- 39 • An experimental study of the mechanical and mineralogical properties
40 of cryptocrystalline flint.
- 41 • Assessment and development of UCS prediction models for the
42 extremely strong cryptocrystalline flint.
- 43 • Validity study of the proposed models, and comparison between
44 measured and estimated UCS.

45 **Abstract**

46 Cryptocrystalline flint is an extremely hard siliceous rock that is found in chalk
47 formations. The chalk is frequently used as a host for underground rock
48 caverns and tunnels in Europe and North America. A reliable estimation of the
49 uniaxial compressive strength (UCS) of the extremely strong flint, with an
50 average UCS of about 600 MPa will provide a scientific guidance for a proper
51 engineering design, where flint is encountered, thereby avoiding project
52 progress delay, litigation as well as economic consequences. Conventional
53 UCS measurement using core samples is cumbersome for flint due to the
54 extreme strength and hardness of the rock, for which the core sample
55 preparation process is often extremely difficult. In this study, the UCS
56 prediction models of flints collected from the North-West Europe were
57 developed and the validity of the developed models was investigated. A series
58 of laboratory index tests (comprising the three-point-bending, point load,
59 ultrasonic velocity, density, Shore hardness and Cerchar Abrasivity tests)
60 were performed. The index test results were correlated with the UCS values
61 previously determined in the laboratory using both cylindrical and cuboidal
62 specimens to develop the UCS prediction models. Regression analysis of the
63 UCS and the index test results was then performed to evaluate for any
64 potential correlations that can be applied to estimate UCS of the
65 cryptocrystalline flint. Intensive validity and comparison studies were
66 performed to assess the performance of the proposed UCS prediction models.
67 This study showed that UCS of the tested flint is linearly correlated with its
68 point load strength index, tensile strength and compressional velocity, and is

69 parabolically correlated with its density. The present study also demonstrated
70 that only a couple of the previously developed empirical UCS models for
71 estimating UCS are suitable for flint, which should be used with care.

72 **1 Introduction**

73 Flint is a siliceous, cryptocrystalline rock that forms in chalk formations
74 which in recent decades are often used as a host for underground
75 infrastructures like underground caverns, power houses and tunnels. Hosted
76 by chalk, flint is extensively distributed in Europe and North America.¹ Flint is
77 initially used as a manufacturing tool early days and now as one of the most
78 critical engineering threats to drilling and tunneling in chalk-bearing flint, due
79 to its extremely strong nature.

80 In the process of drilling or TBM tunneling, the existence of flint usually
81 result in the deflecting of drill bits away from flint layers,² and more worse the
82 severe wear of drill bits and TBM cutters, which can lead to the replacement
83 of drill bits and cutters,³ and in some cases the whole tunnel and TBM
84 machine had to be redesigned.^{4,5} Without a proper planning and design,
85 experiencing these challenges will delay project progress,⁶ thereby resulting
86 in litigation as well as economic consequences.

87 Uniaxial compressive strength (UCS) is generally acknowledged to be
88 often used in the current rock mass classification schemes (such as RMR and
89 Q) and practical rock engineering applications.⁷ It is generally recognized as
90 one of the key rock properties, and as an initial step for a proper engineering
91 design, to understand the UCS of flint. This parameter can be directly
92 measured in the laboratory, following the ISRM standard⁸, which relies on
93 high-quality core samples and certified testing apparatus. One challenge is
94 that the process of core sample preparation can be cumbersome, where
95 extremely strong and hard rock such as the cryptocrystalline flint are
96 encountered. As such, it is necessary to estimate and assess the UCS of flint
97 using empirical methods.

98 Assessment of UCS through empirical methods (referring to index tests
99 such as point load strength, ultrasonic and Cerchar abrasivity tests, etc.) has
100 received significant attention since 1960s. One of the pioneering studies on
101 this topic was reported by Deer and Miller⁹, where five charts were proposed
102 for estimating UCS of intact rock. The establishment of the charts was based

103 on the results of a series of index tests on a total of 257 specimens collected
104 from 27 localities in the United States.

105 Bieniawski¹⁰ also assessed the applicability of using point load test results
106 to estimate UCS and concluded that diametrical point load test was the most
107 convenient and reliable in use; and this method was later recommended by
108 the ISRM¹¹ for the measurement of point load index strength and the
109 estimation of UCS.

110 After an extensive laboratory testing and multivariate statistical analysis,
111 Ulusay et al.¹² proposed several polynomial equations for inferring UCS from
112 the petrographic characteristics (i.e., texture, grain shape and size) and index
113 properties (i.e., density, point load strength and porosity) of Litharenite
114 sandstone in Turkey. Gokceoglu and Zorlu¹³ and Kahraman et al.¹⁴ reported
115 linear relationships between UCS and the Brazilian tensile strength of various
116 rocks.

117 Ultrasonic compressional and shear velocities have also been widely used
118 in the estimation of UCS.¹⁵⁻¹⁹ Kong and Shang²⁰ tested the validity of the point
119 load and Schmidt hammer index tests in the estimation of UCS, by using a
120 range of “standard” bricks, whereby the potential effects of lithological
121 heterogeneity and grain size on results were removed. Those studies
122 concluded that homogeneous rock samples should be used to get a reliable
123 estimation results and point load tests exhibited a somewhat higher accuracy
124 in the estimation of UCS.

125 Although hundreds of empirical equations for estimating UCS are
126 available in literature, those relationships, however, are often rock-type and
127 geological formation dependent. A considerable discrepancies (sometimes
128 can be termed “error”) between estimated UCS and measured UCS can be
129 expected when empirical equations derived from different rock types and
130 formations were used (Kong and Shang²⁰). Readily available and applicable
131 UCS estimation models for characterising the extremely hard cryptocrystalline
132 flint have not yet been developed. This hypothesis motivated the authors to
133 experimentally explore prediction models for assessing the UCS of the flint,
134 which has rarely been investigated and published.

135 The cryptocrystalline flint samples used in this study were collected from
136 the North-West Europe, spanning from the United Kingdom, France to

137 Denmark. A series of index properties including point load strength index,
138 three-point-bending tensile strength, ultrasonic velocities, density, Cerchar
139 abrasivity index and Shore hardness, as well as UCS values of the collected
140 samples were measured in the laboratory. The assessment and estimation of
141 UCS of the cryptocrystalline flint using those index test results were
142 performed by regression analysis and verification study was subsequently
143 conducted. An intensive comparison study was presented by comparing the
144 measured UCS and the estimated UCS using both the currently proposed and
145 previously proposed UCS estimation models.

146 **2 Sample collection and characterisation**

147 2.1 Study sites, sample collection and characterisation

148 The flint samples used in the study were collected from the Upper
149 Cretaceous Chalk formations within the North-West Europe, ranging from the
150 Northern and Southern Provinces of the United Kingdom, the North Western
151 France to the South Eastern Denmark (Fig. 1). Table 1 shows the
152 nomenclatures and origins of the collected flint samples from the study sites.
153 A detailed geological descriptions of the sites.

154 Some representative flint blocks are shown in Fig. 2. It can be seen that
155 the samples exhibited different color (from light grey to dark brownish grey)
156 which is the result of variation in mineral (calcite and silica) composition and
157 degree of cementation as observed in Aliyu et al.²². Varying degrees of white
158 carbonate inclusion (closed by the yellow dashed lines) can be noted from the
159 appearance of the samples. Scanning Electron Microscope (SEM)
160 examination of the flint samples demonstrated that these samples comprise
161 homogenous cryptocrystalline quartz as the dominant mineral (87-99 %), with
162 occasional calcite. Fig 2b shows that the flint sample collected from North
163 Landing (BNLUK) exhibited a clear white crust (closed by the red dashed line)
164 surrounding flint. The relationship between the white crust and flint is
165 illustrated in Fig. 3, where a SEM image of the flint-crust boundary (see the
166 thin section sample in Fig. 3a) is presented. A clear textural variation can be
167 noted between the darker flint (Fig. 3b and 3c) and the more porous white
168 crust (Fig. 3b and 3d). Another feature of flint is the presence sponge spicules
169 and silicified micro-fossils.^{23,24} This feature was also observed in the collected
170 flint samples and is illustrated in Figs. 4c and 4d, where thin section

171 photomicrographs of the flint sample SDFR (France) are presented. Figs 4a
172 and 4b also reveal a void-filling phase dominated by euhedral mega quartz
173 crystals surrounded by cryptocrystalline quartz.

174 **2.2 Uniaxial compressive strength of the flint samples**

175 The uniaxial compressive strengths of the flint samples (Fig. 2) were
176 measured using both cylindrical and cuboidal specimens. In the preparation of
177 the cylindrical specimens, the Richmond SR 2 radial drill was used, with a
178 suitable speed of 1500 Revmin⁻¹, this was found to be the optimum drilling rate
179 through a trial-and-error process. It has been observed from this coring
180 process that the readily available core bits (normally used in the laboratory for
181 regular rocks) were completely worn while coring 1-2 flint specimens
182 (diameter 25 mm and length 60 mm). To resolve this issue, specially-
183 manufactured core bits were used to drill the extremely strong
184 cryptocrystalline flint.

185 Another problem encountered in the process of preparing cylindrical
186 specimens from the BNLUK block was that it proved very difficult to prepare
187 cores without breaking, which is mainly due to the presence of the white
188 carbonate inclusions and micro-fractures (as shown in Fig. 2a). As an
189 alternative, cuboidal specimens (breadth: 18-32 mm; height: 63-67 mm) were
190 prepared for the BNLUK sample in accordance with the ASTM standard ²⁵.

191 Ends of the cylindrical and cuboidal specimens were ground flat. The well-
192 prepared flint specimens were then uniaxially compressed using the Denison
193 loading machine (with a capacity of 2000 kN) at a loading rate of 0.5 MPas⁻¹.
194 The axial stress was monitored by the machine, and the axial and lateral
195 strains of the specimens during the compression were measured using 5 mm
196 strain gauges.

197 Representative stress-strain curves of the tested specimens were shown
198 in Fig. 5, from which Young's modulus and Poisson's ratio were calculated in
199 accordance to the ISRM standard⁸. The mean UCS, Young's modulus and
200 Poisson's ratio of the tested flint samples are shown in Table 2, with the
201 associated standard deviations and the number of specimens tested included.
202 As can be seen from the stress-strain curves (Fig. 5), the tested flint samples
203 exhibited a typical linear deformation and failure occurred abruptly, without
204 any evidence of a post failure record. The relatively higher standard deviation

205 of UCS observed in Table 2 (Column 10) is related to the presence of
206 carbonate inclusions in the samples (Fig. 2). The reported values of the
207 Young's modulus and Poisson's ratio show small variations, which are
208 however broadly consistent with Gercek²⁶ and Pabst and Gregorová²⁷. Fig. 6
209 shows part of the flint specimens before and after the UCS test. Visual
210 observations in the process of the UCS test revealed that axial splitting and
211 brittle failure (leading to sharp and thin slabs, and small pieces, see Fig. 6d)
212 dominated for the tested flint samples, which is often accompanied with
213 catastrophic and explosive noise. Similar observations on flint UCS test were
214 reported by Cumming⁴.

215 **3 Index tests and respective results**

216 The term "index tests" used in the study refers to those simpler tests,
217 whose results can potentially be used to correlate UCS of rock.^{9,20,28-30} In the
218 present study, several widely used index tests including three-point-bending,
219 point load, ultrasonic velocity, density, Shore hardness and Cerchar Abrasivity
220 tests were performed to explore and assess their feasibility for estimating the
221 UCS of flint. A description of the process of each index test conducted in the
222 study, and test results, are presented in this section.

223 To avoid coring and polishing (which is difficult for the strong and hard
224 flint) as shown in Figs. 7a-7c, beam of flint specimens with a length to
225 thickness ratio of more than 3 were prepared for the three-point-bending test,
226 which follows Brook³¹ and Fowell & Martin³². The test was carried out by
227 placing each specimen on two ball bearings separated at various spans
228 depending on the respective specimen dimensions. A concentrated load was
229 applied at the center of each specimen until it fail in tension. In the meanwhile,
230 the failure load was logged and used to calculate the tensile strength (indirect)
231 of the flint. Corresponding results are shown in Table 2. Fig. 7d shows
232 representative failure patterns of the beam specimens tested in the study.

233 The point load test was performed using a point load tester with a loading
234 capacity of 56 kN and an accuracy of 0.05 N. The test was conducted on
235 irregular blocks and lumps of flints (Figs. 8a, 8c and 8e), which is in
236 accordance with the ISRM standard⁸. A steady load was applied on the
237 specimens until failure, and the failure load was recorded and then used to
238 calculate the standard point load index strength (i.e., $I_{s(50)}$, see also Table 2).

239 Figs. 8b, 8d and 8f present part of the failed flint specimens, from which it can
240 be seen that several brittle fractures were always induced around the
241 concentrated loading points.

242 ultrasonic pulse velocities following the ISRM suggested method⁸,
243 comprising compressional wave velocity (V_p) and shear wave velocity (V_s) of
244 flint were measured using an Ergo Tech pulse generator (pulser 1-10). The
245 flint specimens were placed between the transmitter and the receiver under a
246 constant load of 0.2 kN. The load was then applied using the MAND uniaxial
247 compression machine. Honey and a 0.1 mm thick lead foil were used to
248 achieve an acceptable acoustic coupling between the specimens and the
249 transducers. The transit time was measured and used to estimate the
250 ultrasonic velocities (V_p and V_s). Table 2 shows the test results (Columns 3-4).

251 Cerchar abrasivity test originally introduced in Cerchar³³ has been widely
252 used in the laboratory to assess the abrasivity of rocks, thereby, estimating
253 TBM performance.³⁴⁻³⁶ In this study, Cerchar abrasivity test was carried out
254 on lumps of flint specimens, following the method used by Cerchar³³ to
255 estimate the abrasiveness of flint, which translates to the drillability and
256 cutterbility of the material. A standard Cerchar apparatus with a hard steel
257 stylus of HRC 54-56 was used, and a static load of up to 90 N was applied on
258 the stylus. Readings were taken from the worn pin under a microscope
259 following a scratch (10 mm in length) on the samples. Results of the test were
260 then interpreted as that used by Plinninger³⁷; and the mean results for each
261 sample are shown in Table 2 (Column 6).

262 Shore hardness (SH) reflects the hardness of rock, which is often used to
263 evaluate the performance of drilling tools. Following the ISRM standard⁸, the
264 SH test was conducted on flint samples using the C-2 type SH testing
265 machine. In the test, a 2.44 g diamond-tipped hammer was dropped freely
266 on the specimen, and the rebound height was noted and recorded from the
267 incorporated measuring scale. This procedure was then repeated fifty times
268 on each specimen and readings were taken, while five highest as well as
269 lowest readings were discarded in the data analysis. The average of the
270 rebound heights from the remaining readings was taken as the shore
271 hardness of the sample, which are shown in Table 2 (Column 5). The density

272 of the flint samples was determined using the caliper method⁸ and the mean
273 results of each sample are shown in Table 2 (Column 2).

274 **4 Assessing and development of UCS prediction models**

275 **4.1 Regression analysis**

276 A series of regression analysis was performed to assess the potential
277 correlations between UCS of flint and each index test result (i.e. ρ , V_p , V_s , SH,
278 CAI, σ_t , and $I_{s(50)}$). In the analysis, different fitting functions such as linear,
279 parabolic, exponential and lognormal were examined, and a R^2 value of no
280 less than 0.5 was accepted in the study. Table 3 shows correlated equations
281 for estimating UCS of the extremely strong and hard flint. It can be seen that
282 three linear correlations were established, which include UCS - $I_{s(50)}$, UCS - σ_t ,
283 and UCS - V_p ; and parabolic relation was found between UCS and density (ρ).
284 No acceptable statistical correlations can be derived from V_s , SH, CAI to
285 estimate UCS of flint, although these three index tests have been used to
286 estimate UCS of various rocks such as marble³⁸, limestone and shale³⁹, and
287 serpentinites⁴⁰.

288 **4.2 Verification, comparison and discussion**

289 To verify the capability of the proposed equations (Table 3), the estimated
290 UCS values through the equations were assessed by comparing them with
291 the measured UCS values as that used by Ng et al.⁴¹ and Kong and Shang²⁰.
292 The comparison results are shown in Fig. 10, where most of the estimated
293 data were close to the 100 % line, with an acceptable deviation of $\sim \pm 20$ %
294 (i.e., within the region bounded by the 80 % and 120 % lines).

295 Additionally, the hypothesis mentioned in the Introduction (the empirical
296 equations derived from other rocks may not be suitable for the estimation of
297 the extremely hard flint) was tested in this section. Representative empirical
298 relations (i.e. UCS - $I_{s(50)}$, UCS - σ_t , UCS - V_p and UCS - ρ) in literature were
299 assembled (see the Appendix, Tables A1-A4). Those equations were
300 respectively used to estimate UCS of the flint samples tested in the study. The
301 estimated UCS values were compared with both the measured UCS and the
302 estimated results via the equations proposed in the study. Fig 11a shows a
303 comparison between the measured UCS (black dots) and the estimated UCS
304 using the point load strength index ($I_{s(50)}$). It is noted that the scattered seven

305 data points for each group (column) is related to the seven different sample
306 sites, which corresponds to BNLUK, SESUK, BLSUK, SDFR, LMFR, TSDK
307 and TMDK, respectively (from the top to the bottom). Box charts are also
308 included in Fig 11a to graphically reflect some key values (i.e. mean, median,
309 interquartile range, and maximum and minimum values) of the data from the
310 statistics point of view. Mean value was used to assess the closeness of the
311 data between each group.

312 As shown in Fig 11a, considerable discrepancies can be seen between
313 the estimations (through $I_{s(50)}$) and the measured values, with a maximum
314 overestimation of 54.9 % and a maximum underestimation of up to 65.3 %.
315 Such huge differences can be treated as an “error” in practical rock
316 engineering when some of the equations (for example that proposed by
317 Tsiambaos and Sabatakakis⁴⁹) were used to estimate the UCS of flint. Only a
318 small part of the equations including those proposed by Singh²⁸, Ulusay et
319 al.¹², Palchik and Hatzor⁵⁰, Basu and Aydin⁵², Karaman et al.⁵⁸, Kong and
320 Shang²⁰, as well as the one proposed in the present study (UCS= 17.6
321 $I_{s(50)}+13.5$) gave an acceptable estimation of the UCS of flint. This
322 phenomenon indicates that not all of the previously proposed UCS – $I_{s(50)}$
323 equations are unsuitable for the estimation of UCS of flints. The reason
324 underlying this phenomenon is still not clear, as many geological and
325 geographic factors, as well as diagenetic process may affect the results. A
326 further study is necessary to explore the main factors controlling the
327 discrepancy, so that a unified model can be developed. The present study
328 further demonstrated that the UCS - $I_{s(50)}$ model proposed in this study (Table
329 3) and the previously derived UCS - $I_{s(50)}$ model presenting a good
330 performance (mentioned above) are suggested to be used in the UCS
331 estimation of flints.

332 Figs 11b, 11c and 11d show comparisons between the measured UCS
333 and the UCS estimated using the three-point-bending tensile strength (σ_t),
334 compressional velocity (V_p) and density (ρ), respectively. Similarly, clear and
335 unacceptable discrepancies can be observed, especially for some cases
336 where the maximum underestimations can be up to 81.6 % (Fig. 11c) and
337 87.6 % (Fig. 11d). Also without exception, the presently proposed UCS – V_p
338 and UCS – ρ equations provide reliable estimations (Figs. 11c and 11d). For

339 the estimation of UCS of flint using UCS – σ_t , the relations proposed by Din
340 and Rafiq²⁹ and Kahraman et al.¹⁴ also exhibited a good performance,
341 besides the equation proposed in this study (Table 3, Fig. 11b).

342 **5 Summary and conclusions**

343 In this study, a compressive experimental investigation was carried out to
344 explore suitable empirical models for estimating UCS of the extremely strong
345 cryptocrystalline flint, which is special and often embedded in chalk formations.
346 The UCS values of the flint samples collected from the UK, France and
347 Denmark were first measured using both cylindrical and cuboidal specimens.
348 A series of index tests including three-point-bending test, point load strength,
349 ultrasonic velocity, density, Shore hardness and Cerchar abrasivity tests were
350 performed in the laboratory. Regression analysis of the UCS and index test
351 results was performed to probe any potential correlation models that can be
352 used to estimate the UCS of flint. After that, a validity study of the proposed
353 equations was presented, followed by the presentation of a comparison and
354 discussion.

355 The uniaxial compressive strength of the cryptocrystalline flint tested in
356 this study is linearly correlated with its point load strength index ($I_{s(50)}$), indirect
357 tensile strength (σ_t) and compressional velocity (V_p), and is parabolically
358 correlated with density (ρ). However, no acceptable statistical relations can be
359 obtained between UCS and results from Shore hardness test, Cerchar
360 Abrasivity test and shear velocity test. The four proposed empirical equations
361 in this study have been proofed effective, and are therefore, suggested for
362 estimating UCS of the extremely hard flint. The present finding, thus, implies
363 that quick estimate of UCS of flints can now be made using simpler and non-
364 destructive tests, thereby saving time and by implication costs (in engineering
365 projects in chalk with flints).

366 The present study also revealed that a couple of the previously derived
367 empirical UCS models from other rocks could be used to predict the UCS of
368 flints, but with much care.

369 **References**

- 370 1. Shepherd W. Flint: its origin, properties and uses. Transatlantic Arts; 1st
371 edition; 1972: pp 256.

- 372 2. Hahn-Pedersen M. AP Møller and the Danish Oil: Denmark, J.H. Schultz
373 Grafisk. 1999; pp 357.
- 374 3. Banner S. Development of Munroe regional medical center. Star Banner
375 6A, Ocala, Florida; 2001.
- 376 4. Cumming FC. Machine tunnelling in chalk with flint with particular
377 reference to the mechanical properties of flint. Unpublished PhD thesis,
378 University of Brighton, UK. 1999.
- 379 5. Mortimore RN. Chalk: a stratigraphy for all reasons. The Scott Simpson
380 Lecture: Proceedings of the Ussher Society, Geosciences in South West
381 England. 2001; 10(2):105-122.
- 382 6. Peterson MNA. Deep Sea Drilling Project Technical Report: Core bits
383 Contract NSF C-482, University of California, San Diego. 1974: Retrieved
384 from http://deepseadrilling.org/t_reports.htm.
- 385 7. Hu J, Shang J, Lei T. Rock mass quality evaluation of underground
386 engineering based on RS-TOPSIS method. J Cent South Univ. 2012;
387 43(11): 4412-4419.
- 388 8. ISRM. The complete ISRM suggested methods for rock characterization,
389 testing and monitoring: 1974-2006: In Ulusay, R., & Hudson, J. A. (eds)
390 Suggested methods prepared by the commission on testing methods,
391 International Society for Rock Mechanics, Commission on Testing
392 Methods (ISRM Turkish National Group), 2007.
- 393 9. Deere DU, Miller RP. Engineering classification and index properties for
394 intact rock. Technical Report. AFWL-TR-65-116. A. F. Weapons
395 Laboratory, Kirtland. 1966.
- 396 10. Bieniawski ZT. The point-load test in geotechnical practice. Eng Geol.
397 1975; 9(1): 1-11.
- 398 11. ISRM. Suggested method for determining point load strength. Int J Rock
399 Mech Min Sci Geomech Abstr. 1985; 22(2): 51-60.
- 400 12. Ulusay R, Türeli K, Ider MH. Prediction of engineering properties of a
401 selected litharenite sandstone from its petrographic characteristics using
402 correlation and multivariate statistical techniques. Eng Geol. 1994; 38(1-2):
403 135-157.

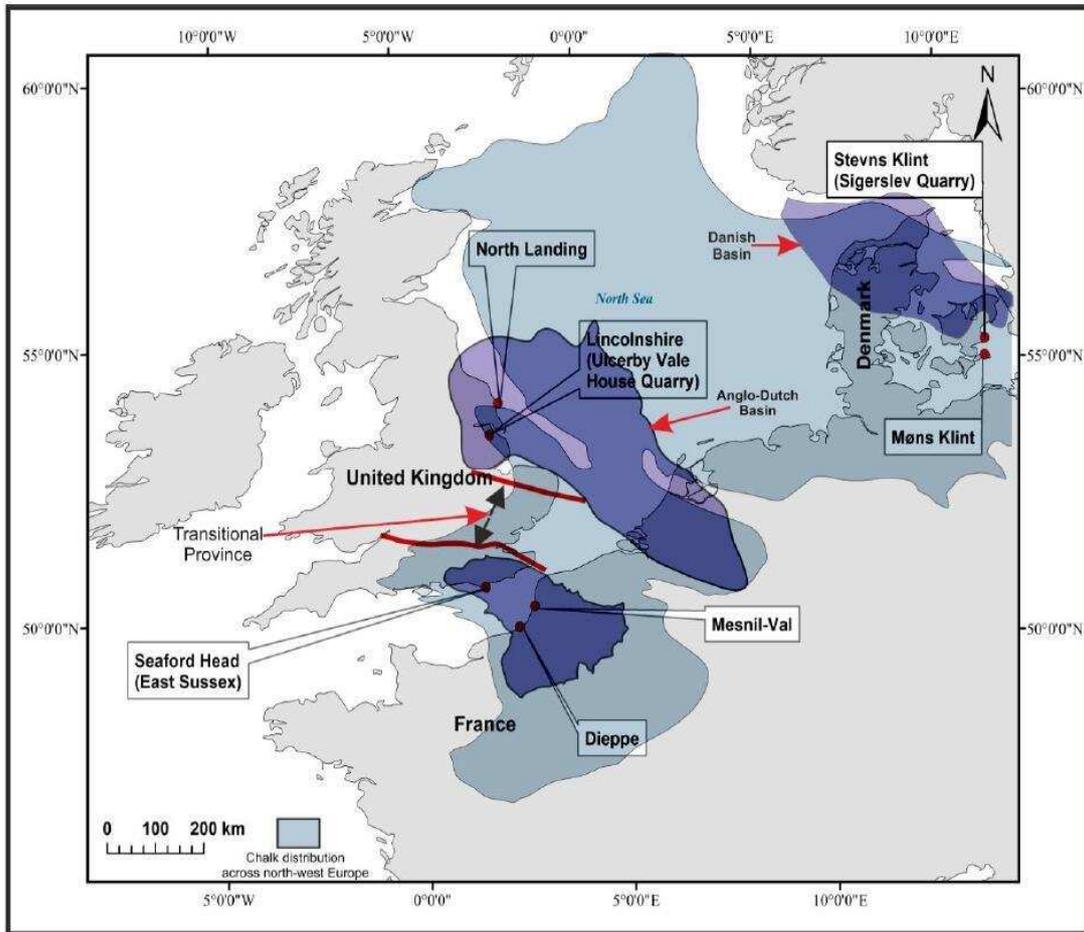
- 404 13. Gokceoglu C, Zorlu K. A fuzzy model to predict the unconfined
405 compressive strength and modulus of elasticity of a problematic rock. *Eng*
406 *Appl Artif Intell.* 2004; 17:61-72.
- 407 14. Kahraman S, Fener M, Kozman E. Predicting the Compressive and
408 Tensile Strength of Rocks from Indentation Hardness Index. *J South Afr*
409 *Inst Min Metall.* 2012; 112(5): 331-339.
- 410 15. Tuğrul A, Zarif I H. Correlation of mineralogical and textural characteristics
411 with engineering properties of selected granitic rocks from Turkey. *Eng*
412 *Geol.* 1999; 51(4): 303-317.
- 413 16. Kahraman S. Evaluation of simple methods for assessing the uniaxial
414 compressive strength of rock. *Int J Rock Mech Min Sci.* 2001; 38(7): 981-
415 994.
- 416 17. Yaşar E, Erdoğan Y. Correlating sound velocity with the density,
417 compressive strength and young's modulus of carbonate rocks. *Int J Rock*
418 *Mech Min Sci.* 2004; 41(5): 871-875.
- 419 18. Yagiz S. P-wave velocity test for assessment of geotechnical properties of
420 some rock materials. *Bull Mater Sci.* 2011; 34(4): 947-953.
- 421 19. Najibi A R, Ghafoori M, Lashkaripour G R., Asef M R. Empirical relations
422 between strength and static and dynamic elastic properties of Asmari and
423 Sarvak limestones, two main oil reservoirs in Iran. *J Petrol Sci Eng.* 2015;
424 126: 78-82.
- 425 20. Kong F, Shang J. A validation study for the estimation of uniaxial
426 compressive strength based on index tests. *Rock Mech Rock Eng.* 2018;
427 51(7): 2289-2297.
- 428 21. Aliyu MM. The origin and properties of flint in the Upper Cretaceous Chalk.
429 PhD thesis, the University of Leeds, United Kingdom. 2016; pp 283.
- 430 22. Aliyu MM, Murphy W, Lawrence JA, Collier R. Engineering geological
431 characterization of flints. *Q J Eng Geol Hydrogeo.* 2017; 50(2): 133-147.
- 432 23. Bromley R G, Ekdale A A. Trace fossil preservation in flint in the European
433 chalk. *J Paleontol.* 1984; 58: 298-311.
- 434 24. Donovan SK, Fearnhead FE. Exceptional fidelity of preservation in a
435 reworked fossil, Chalk drift, South London, England. *Geol J.* 2015; 50(1):
436 104-106.

- 437 25. ASTM. Standard test method for laboratory determination of pulse
438 velocities and ultrasonic elastic constants of rock (D2845-00). West
439 Conshohocken, American Society for Testing Materials, ASTM. 2000.
- 440 26. Gercek H. Poisson's ratio values for rocks. *Int J Rock Mech Min Sci.* 2007;
441 44(1): 1-13.
- 442 27. Pabst W, Gregorová E. Elastic properties of silica polymorphs—a review.
443 *Ceramics – Silikáty.* 2013; 57(3): 167-184.
- 444 28. Singh DP. Determination of some engineering properties of weak rocks. In:
445 *Proceedings of the international symposium on weak rock, Tokyo.* 1981;
446 pp. 21-24.
- 447 29. Din F, Rafiq M. Correlation between compressive strength and tensile
448 strength/index strength of some rocks of North-West Frontier Province
449 (limestone and granite). *Geol Bull.* 1997; 30:183-193.
- 450 30. Sousa LMO, Suárez del Río L M, Calleja L, Ruiz de Argandoña VG, Rey
451 AR. Influence of microfractures and porosity on the physio-meehanieal
452 properties and weathering of ornamental granites. *Eng Geol.* 2005;
453 77:153-168.
- 454 31. Brook N. The measurement and estimation of basic rock strength.
455 *Comprehensive rock engineering*, Pergamon Press. 1993; 3: 41-66.
- 456 32. Fowell RJ, Martin JA. Cutterbility Assessment of Paramoudra Flints. A
457 report for AMEC Civil Engineering Limited/Southern Water Limited.
458 Department of Mining and Mineral Engineering, University of Leeds, 1997.
- 459 33. Cerchar. Centre d'Études et des Recherches des Charbonages de France.
460 The Cerchar abrasiveness index. Verneuil. 1986.
- 461 34. Suana M, Peters T. The Cerchar abrasivity index and its relation to rock
462 mineralogy and petrography. *Rock mechanics.* 1982; 15(1): 1-8.
- 463 35. Fowell R J, Abu Bakar MZ. A review of the Cerchar and LCPC rock
464 abrasivity measurement methods. In: *11th Congress of the International
465 Society for Rock Mechanics, Second half century for rock mechanics,
466 Lisbon.* 2007; 1: 155-160.
- 467 36. Käsling H, Thuro K. Determining abrasivity of rock and soil in the
468 laboratory. In: *Williams AL, Pinches GM, Chin CY, Mcmorran TJ, Massey
469 Cl. Geologically Active: Proceedings of the 11th IAEG Congress.* CRC
470 Press, London. 2010; 1973-1980.

- 471 37. Plinninger RJ. Abrasiveness assessment for hard rock drilling. *Geomech*
472 *Tunnelling*. 2008; 1(1): 38-46.
- 473 38. Deliormanlı AH. Cerchar abrasivity index (CAI) and its relation to strength
474 and abrasion test methods for marble stones. *Constr Build Mater*. 2012;
475 30:16-21.
- 476 39. Shalabi FI, Cording EJ, Al-Hattamleh OH. Estimation of rock engineering
477 properties using hardness tests. *Eng Geol*. 2007; 90(3-4): 138–147.
- 478 40. Diamantis K, Gartzos E, Migiros G. Study on uniaxial compressive
479 strength, point load strength index, dynamic and physical properties of
480 serpentinites from Central Greece: test results and empirical relations. *Eng*
481 *Geol*. 2009; 108:199-207.
- 482 41. Ng IT, Yuen KV, Lau CH. Predictive model for uniaxial compressive
483 strength for grade III granitic rocks from Macao. *Eng Geol*. 2015; 199: 28-
484 37.
- 485 42. D'Andrea DV, Fisher RL, Fogelson DE. Prediction of compression strength
486 from other rock properties. *Colo Sch Min Q*. 1964; 59(4b): 623-640.
- 487 43. Broch E. Estimation of strength anisotropy using the point-load test. *Int J*
488 *Rock Mech Min Sci*. 1983; 20:181-187.
- 489 44. Bieniawski ZT. The point-load test in geotechnical practice. *Eng Geol*.
490 1975; 9(1): 1-11.
- 491 45. Hoek E. Rock mechanics laboratory testing in the context of a consulting
492 engineering organization. *Int J Rock Mech Min Sci*. 1977; 14: 93-101.
- 493 46. Cargill JS, Shakoor A. Evaluation of empirical methods for measuring the
494 uniaxial strength of rock. *Int J Rock Mech Min Sci Geomech Abstr*. 1990;
495 27(6): 495-503.
- 496 47. Smith HJ. The point load test for weak rock in dredging applications. *Int J*
497 *Rock Mech Min Sci*. 1997; 34: 295.e1–295.e13.
- 498 48. Hawkins AB. Aspects of rock strength. *Bull Eng Geol Environ*. 1998;
499 57:17-30.
- 500 49. Tsiambaos G, Sabatakakis N. Considerations on strength of intact
501 sedimentary rocks. *Eng Geol*. 2004; 72(3-4): 261-273.
- 502 50. Palchik V, Hatzor YH. The influence of porosity on tensile and
503 compressive strength of porous chalks. *Rock Mech Rock Eng*. 2004; 37(4):
504 331-341.

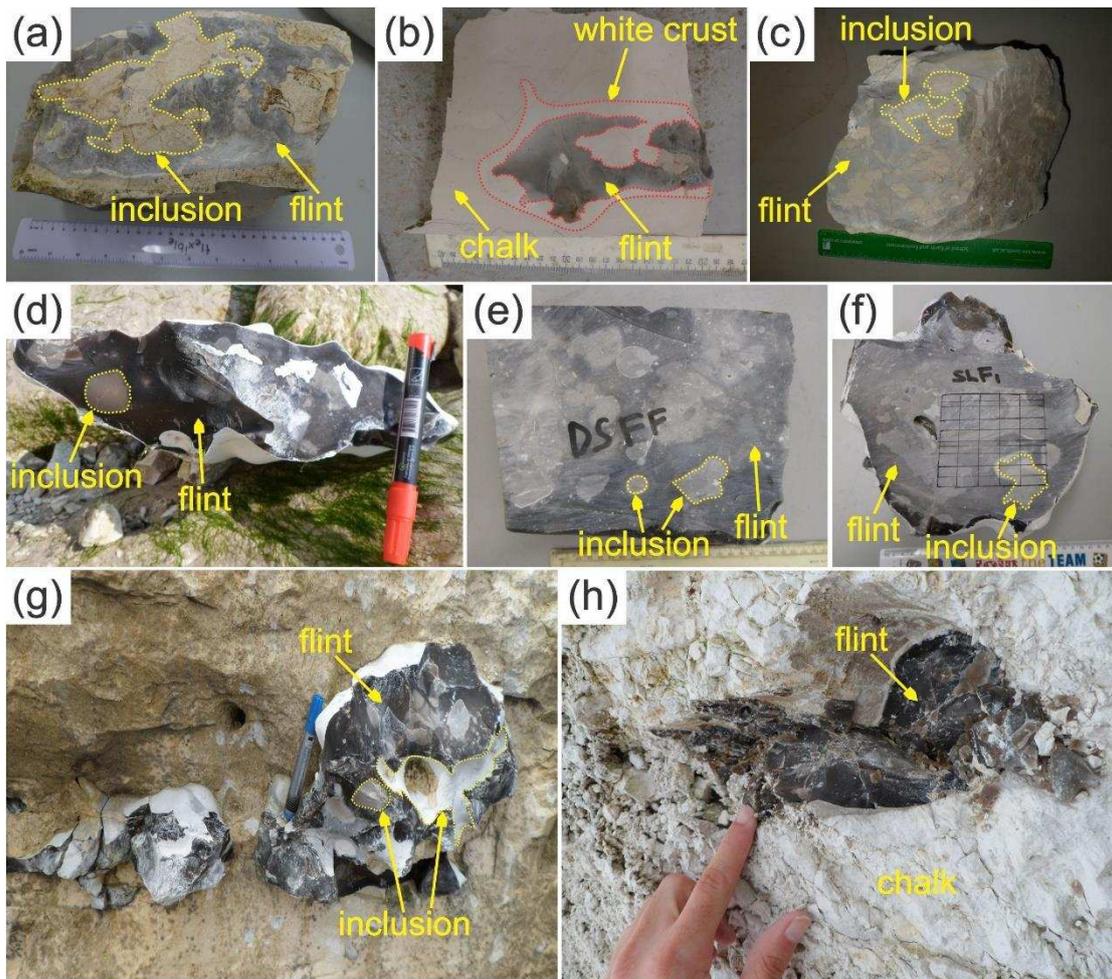
- 505 51. Fener M, Kahraman S, Bilgil A, Gunaydin O. A comparative evaluation of
506 indirect methods to estimate the compressive strength of rocks. *Rock*
507 *Mech Rock Eng.* 2005; 38(4): 329-343.
- 508 52. Basu A, Aydin A. Predicting uniaxial compressive strength by point load
509 test: significance of cone penetration. *Rock Mech Rock Eng.* 2006; 39(5):
510 483-490.
- 511 53. Yilmaz I, Yuksek G. Prediction of the strength and elasticity modulus of
512 gypsum using multiple regression, ANN, and ANFIS models. *Int J Rock*
513 *Mech Min Sci.* 2009; 46: 803-810.
- 514 54. Basu A, Kamran M. Point load test on schistose rocks and its applicability
515 in predicting uniaxial compressive strength. *Int J Rock Mech Min Sci.* 2010;
516 47(5):823-828.
- 517 55. Heidari M, Khanlari G R, Mehdi Torabi Kaveh, Kargarian S. Predicting the
518 uniaxial compressive and tensile strengths of gypsum rock by point load
519 testing. *Rock Mech Rock Eng.* 2012; 45: 265-273.
- 520 56. Kohno M, Maeda H. Relationship between point load strength index and
521 uniaxial compressive strength of hydrothermally altered soft rocks. *Int J*
522 *Rock Mech Min Sci.* 2012; 50(2):147-157.
- 523 57. Singh TN, Kainthola A, Venkatesh A. Correlation between point load index
524 and uniaxial compressive strength for different rock types. *Rock Mech*
525 *Rock Eng.* 2012; 45(2): 259-264.
- 526 58. Karaman K, Kesimal A, Ersoy H. A comparative assessment of indirect
527 methods for estimating the uniaxial compressive and tensile strength of
528 rocks. *Arab J Geosci.* 2015; 8:2393-2403.
- 529 59. Altindag R, Guney A. Predicting the relationships between brittleness and
530 mechanical properties (UCS, TS and SH) of rocks. *Sci Res Essays.* 2010;
531 5(16): 2107-2118.
- 532 60. Nazir R, Momeni E, Armaghani DJ, Amin MFM. Correlation between
533 unconfined compressive strength and indirect tensile strength of limestone
534 rock samples. *Electron J Geotech Eng.* 2013; 18:1737-1746.
- 535 61. Mohamad ET, Armaghani DJ, Momeni E. Prediction of the unconfined
536 compressive strength of soft rocks: a PSO-based ANN approach. *Bull Eng*
537 *Geol Environ.* 2015; 74(3): 745-757.

- 538 62.Çobanğlu İ, Çelik SB. Estimation of uniaxial compressive strength from
539 point load strength, Schmidt hardness and P-wave velocity. Bull Eng Geol
540 Environ. 2008; 67(4):491-498.
- 541 63.Sharma PK, Singh TN. A correlation between P-wave velocity, impact
542 strength index, slake durability index and uniaxial compressive strength.
543 Bull Eng Geol Environ. 2008; 67:17-22.
- 544 64.Khandelwal M, Singh T N. Correlating static properties of coal measures
545 rocks with p-wave velocity. Int J Coal Geol. 2009; 79:55-60.
- 546 65.Moradian Z A, Behnia M. Predicting the uniaxial compressive strength and
547 static young's modulus of intact sedimentary rocks using the ultrasonic
548 test. Int J Geomech; 2009; 9(1):14-19.
- 549 66.Khandelwal M. Correlating P-wave velocity with the physico-mechanical
550 properties of different rocks. Pure Appl Geophys. 2013; 170(4): 507-514.
- 551 67.Yesiloglu-Gultekin N, Gokceoglu C, Sezer EA. Prediction of uniaxial
552 compressive strength of granitic rocks by various nonlinear tools and
553 comparison of their performances. Int J Rock Mech Min Sci. 2013; 62(9):
554 113-122.
- 555 68.Azimian A, Ajalloeian R, Fatehi L. An empirical correlation of Uniaxial
556 Compressive Strength with P-wave velocity and point load strength index
557 on Marly rocks using statistical method. Geotech Geol Eng.
558 2014; 32(1):205-214
- 559 69.Tiryaki B. Predicting intact rock strength for mechanical excavation using
560 multivariate statistics, artificial neural networks, and regression trees. Eng
561 Geol. 2008; 99(1): 55-60.
- 562 70.Gupta V. Non-destructive testing of some Higher Himalayan rocks in the
563 Satluj Valley. Bull Eng Geol Environ. 2009; 68(3): 409-416.
- 564
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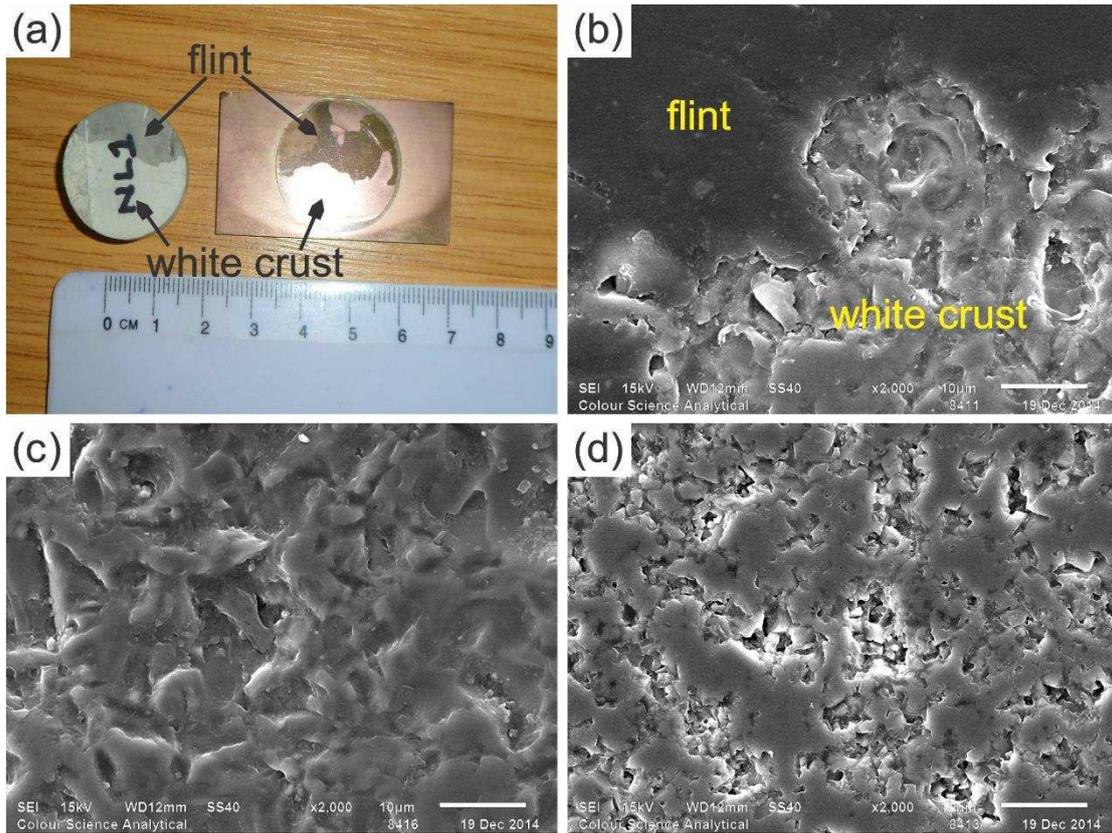
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574 **Fig. 1** Study sites indicated by the red dots. Adapted from Aliyu et al.²²



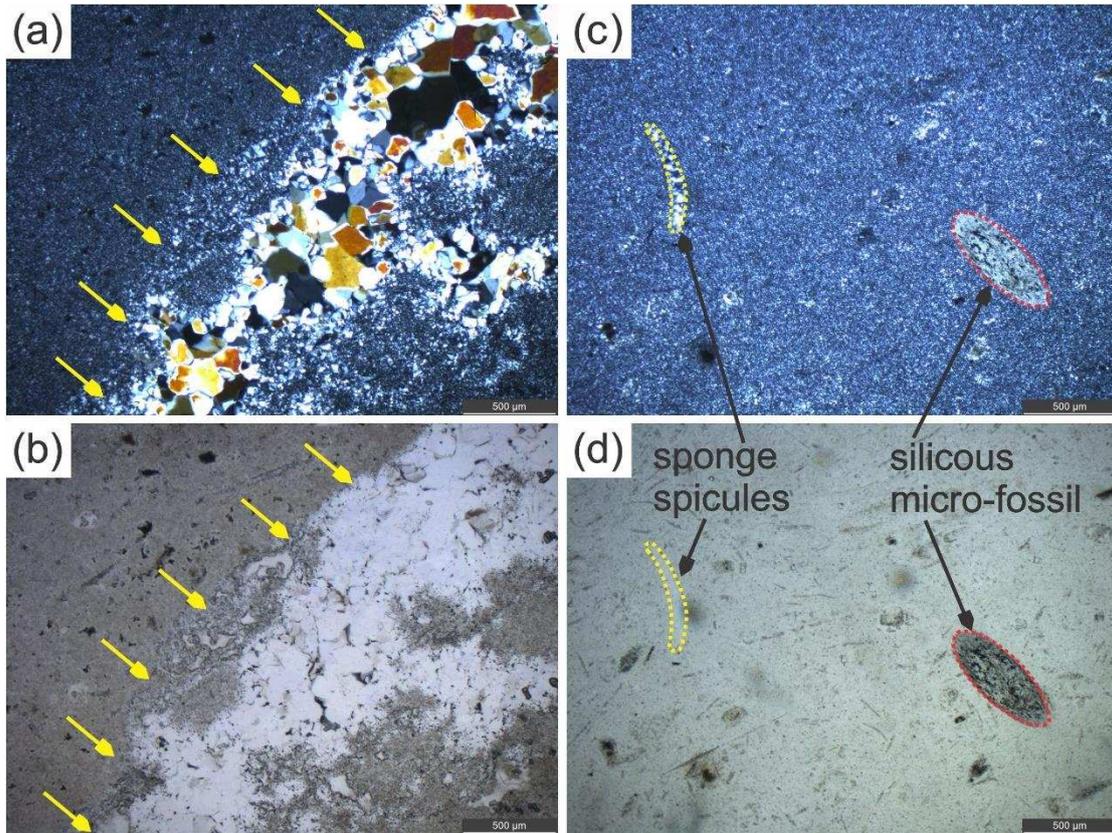
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576 **Fig. 2** Representative flint samples from the North-Western Europe. (a) and (b)
 577 BNLUK; (c) BLSUK; (d) SESUK; (e) SDFR; (f) LMFR; (g) TSDK and (h) TMDK.
 578 The carbonate inclusions and white crust (b) were closed by yellow and red
 579 dashed lines, respectively. See Table 1 for the nomenclature of the flint
 580 samples.



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582 **Fig. 3** (a) Samples used for the SEM analysis of the flint-crust boundary
 583 observed in Fig. 2b; (b) SEM of the flint-crust boundary from the North
 584 Landing flint (UK); (c) SEM of only the flint segment of the samples and (d)
 585 SEM of the crust segment of the sample. A clear textural variation can be
 586 observed between the darker flint and the more porous white crust.



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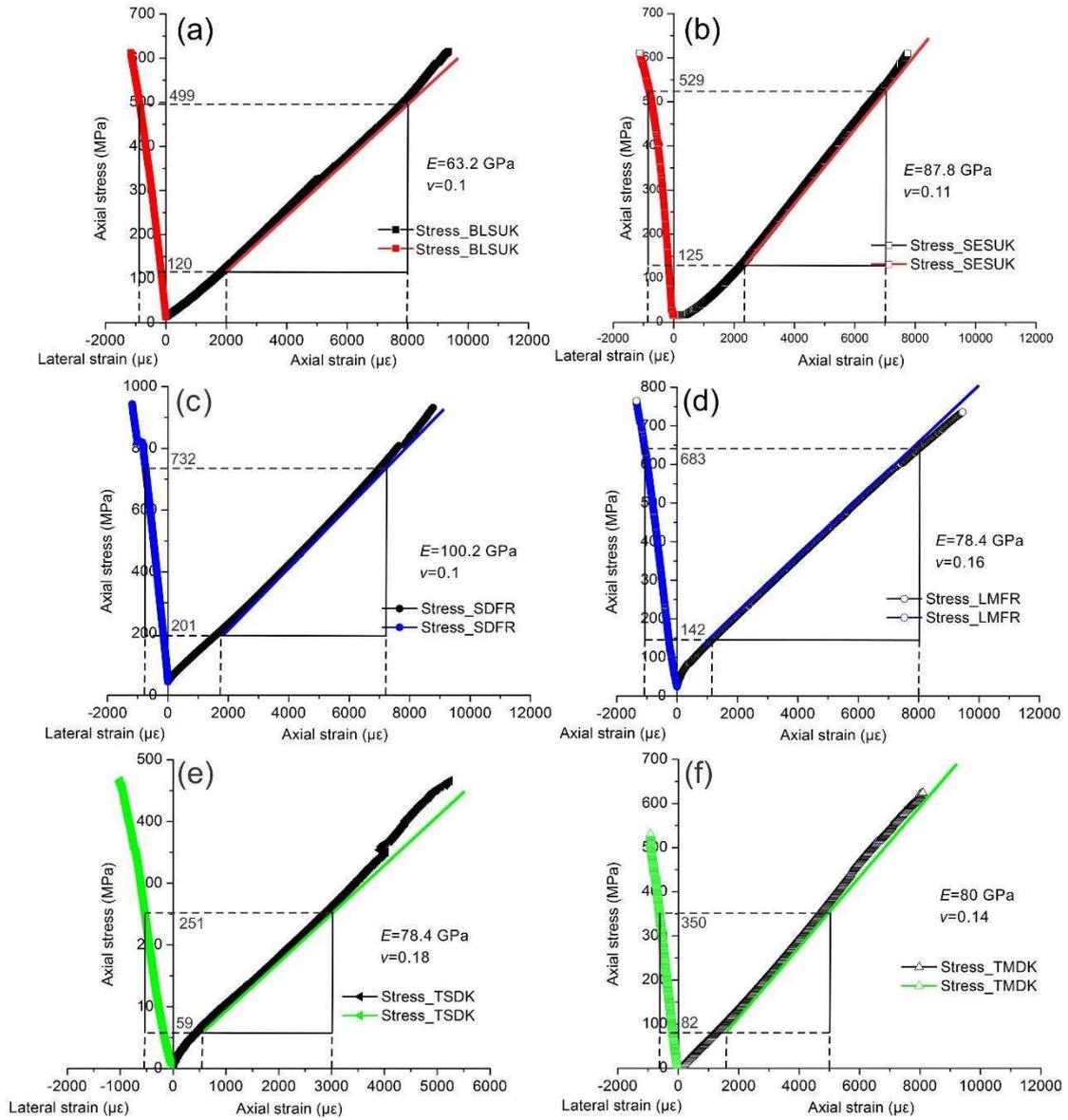
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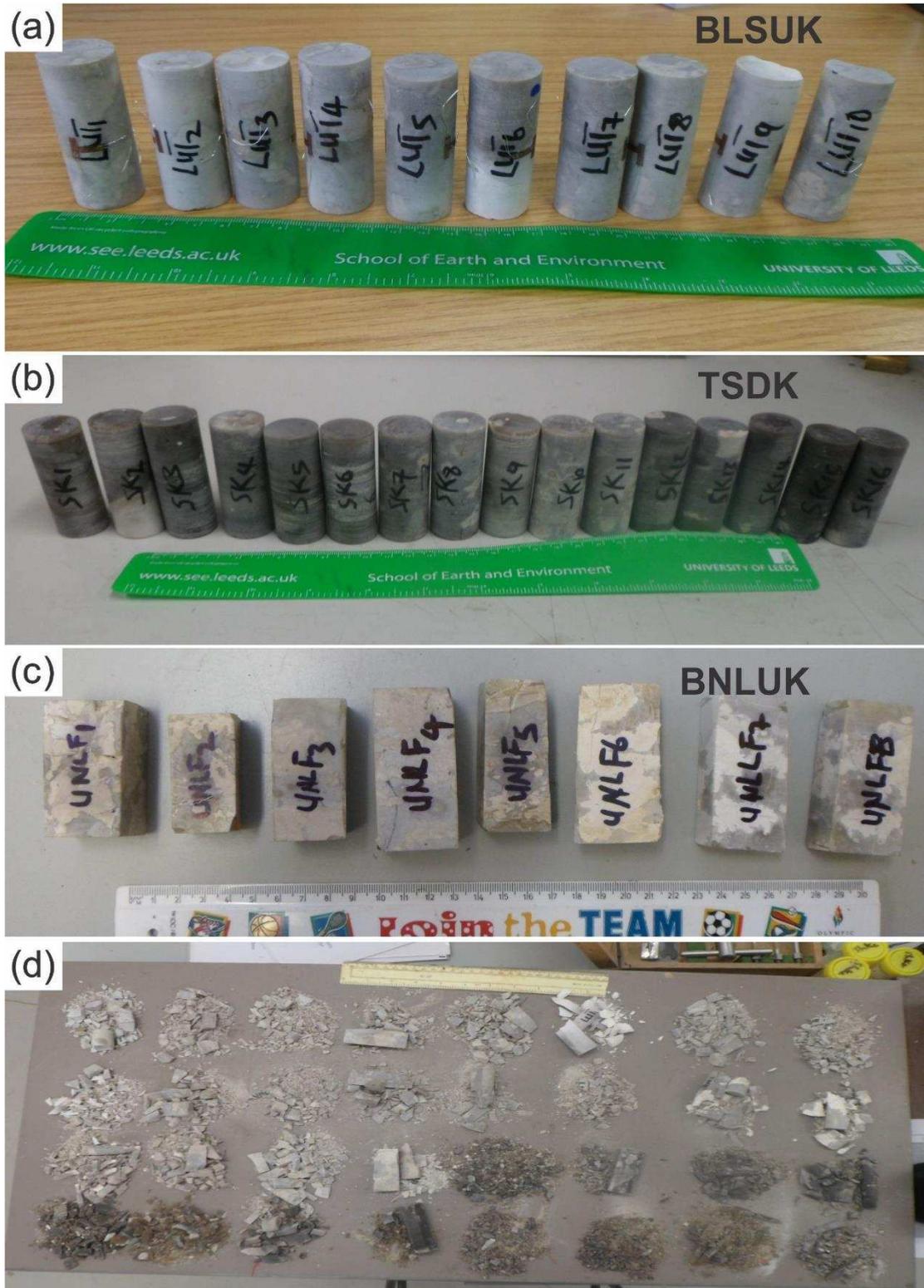
Fig. 4 Thin section photomicrographs of flint from the Seaford Chalk at Dieppe, France (SDFR, also see Fig. 2e). (a) and (c) Graphs observed under cross-polarized light; (b) and (d) are (a), and (c) presented under plane-polarized light. Note that Euhedral mega quartz crystals surrounded by cryptocrystalline quartz are shown by the yellow arrows ((a) and (b)). A sponge spicule and a siliceous micro-fossil were observed and closed by yellow and red dashed lines, respectively ((c) and (d)).



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596 **Fig. 5** Typical stress-strain curves for UCS tests on the tested flint samples. (a)

597 BLSUK; (b) SESUK; (c) SDFR; (d) LMFR; (e) TSDK and (f) TMDK.

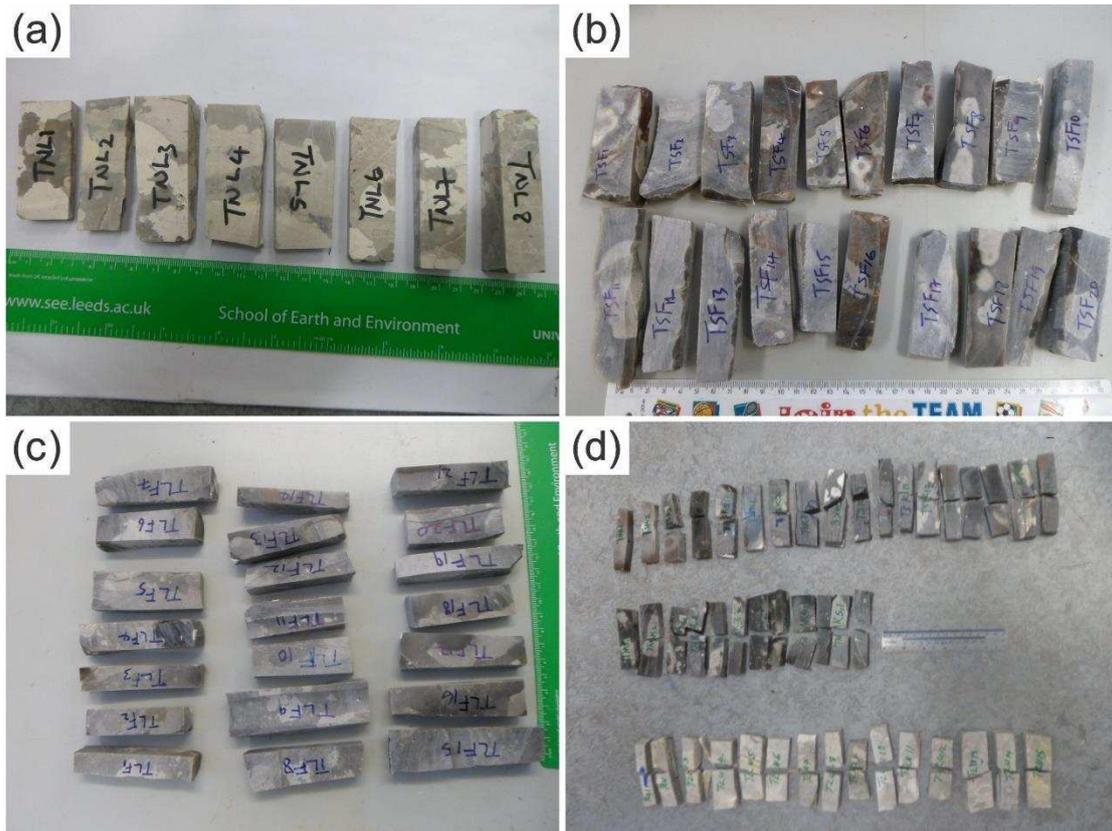


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599 **Fig. 6** Part of specimens before and after UCS test. Cylindrical specimens of

600 BLSUK (a) and TSDK (b); (c) Cuboidal specimens of BNLUK and (d) failure

601 patterns.



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Fig. 7 Part of specimens before and after three-point-bending test. Beam specimens of BNLUK (a), SESUK (b) and LMFR (c); (d) Failure patterns.

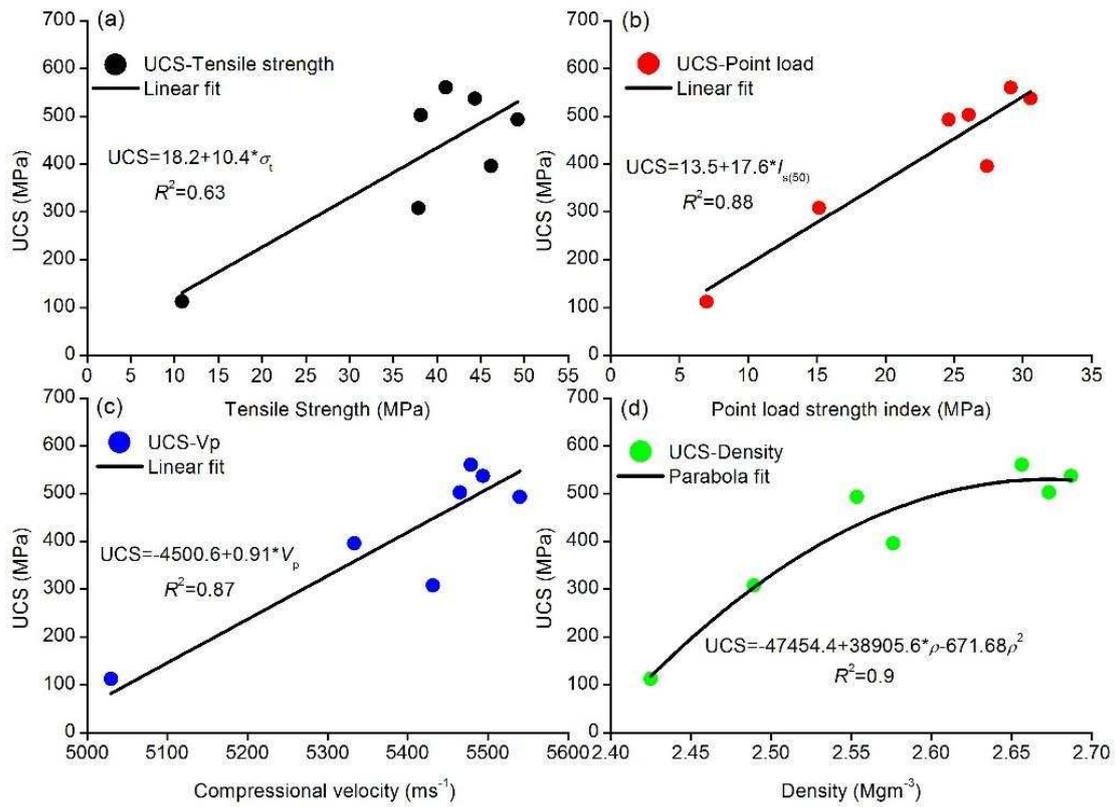


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Fig. 8 Part of specimens before and after point load test. (a) and (b) SDFR; (c) and (d) TMDK, and (e) and (f) TSDK.



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609 **Fig. 9** Relationship between UCS of flint and index test results. (a) UCS vs. σ_t ;

610 (b) UCS vs. $I_{s(50)}$; (c) UCS vs. V_p and (d) UCS vs. ρ .

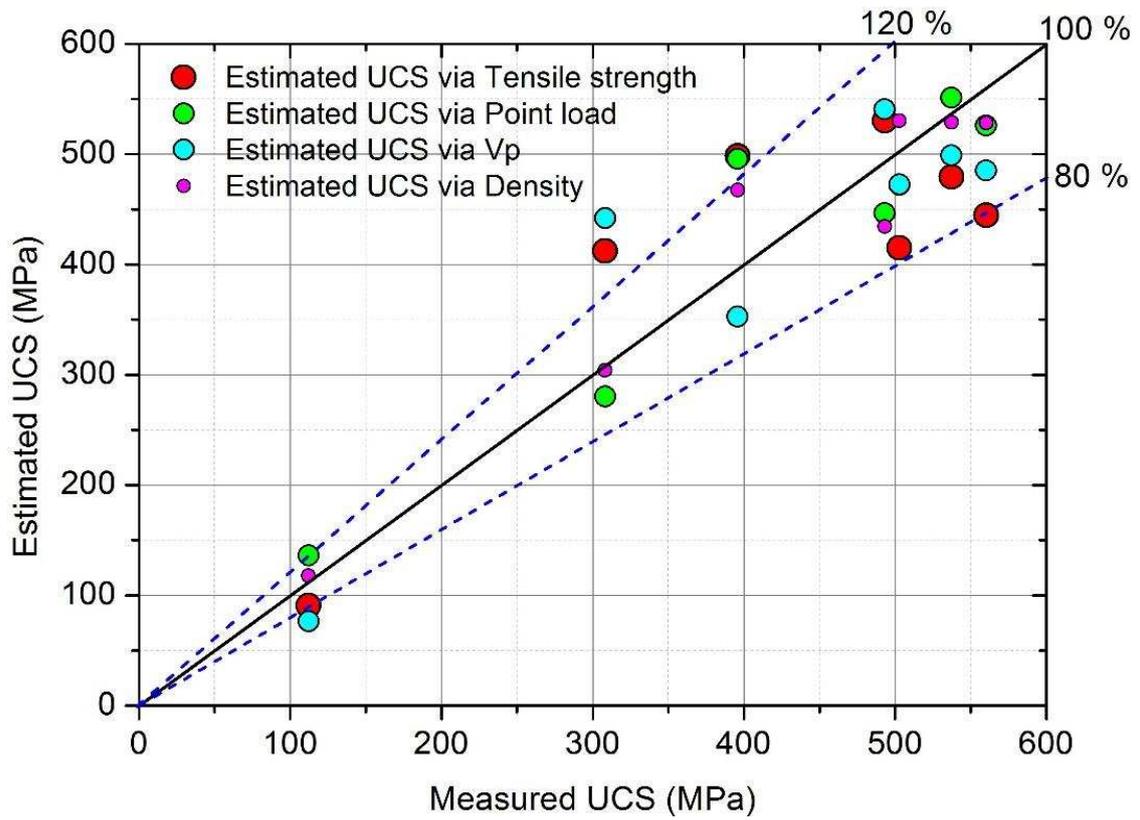
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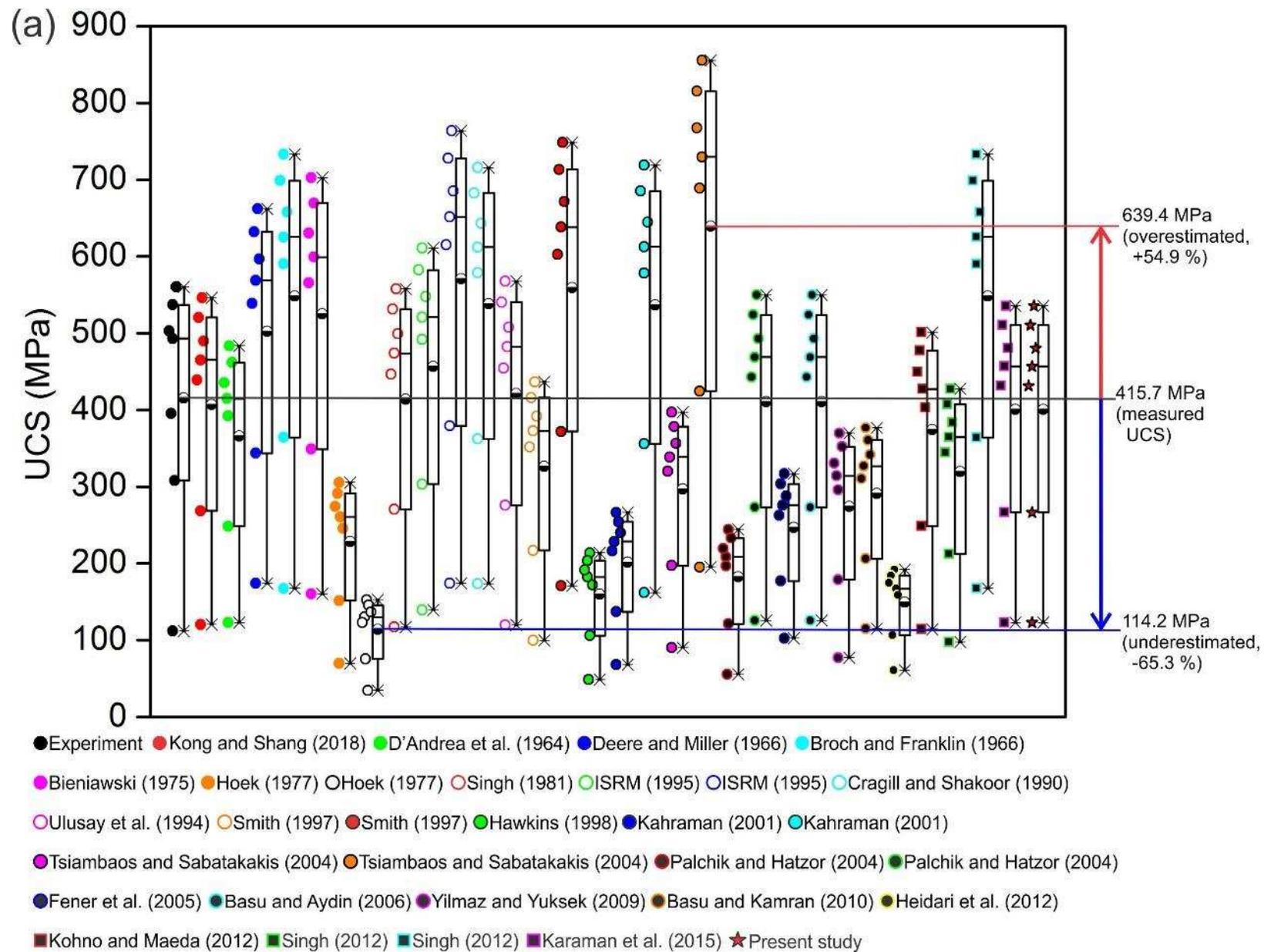
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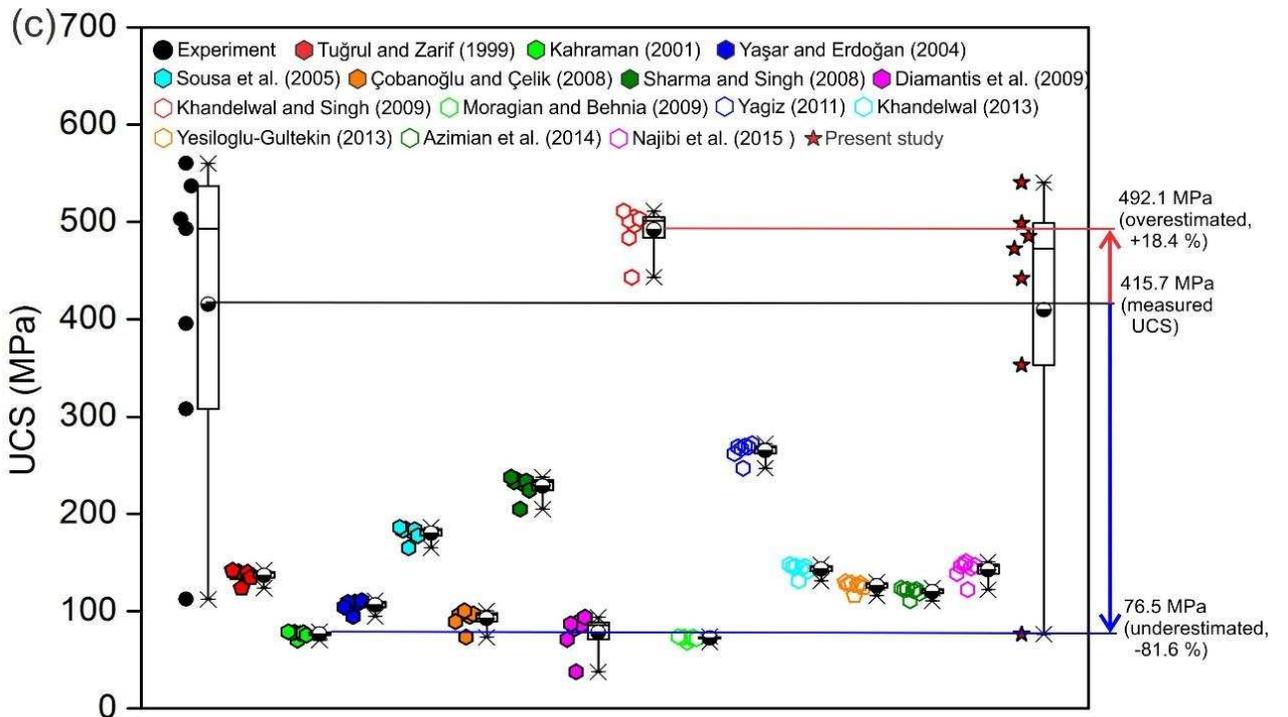
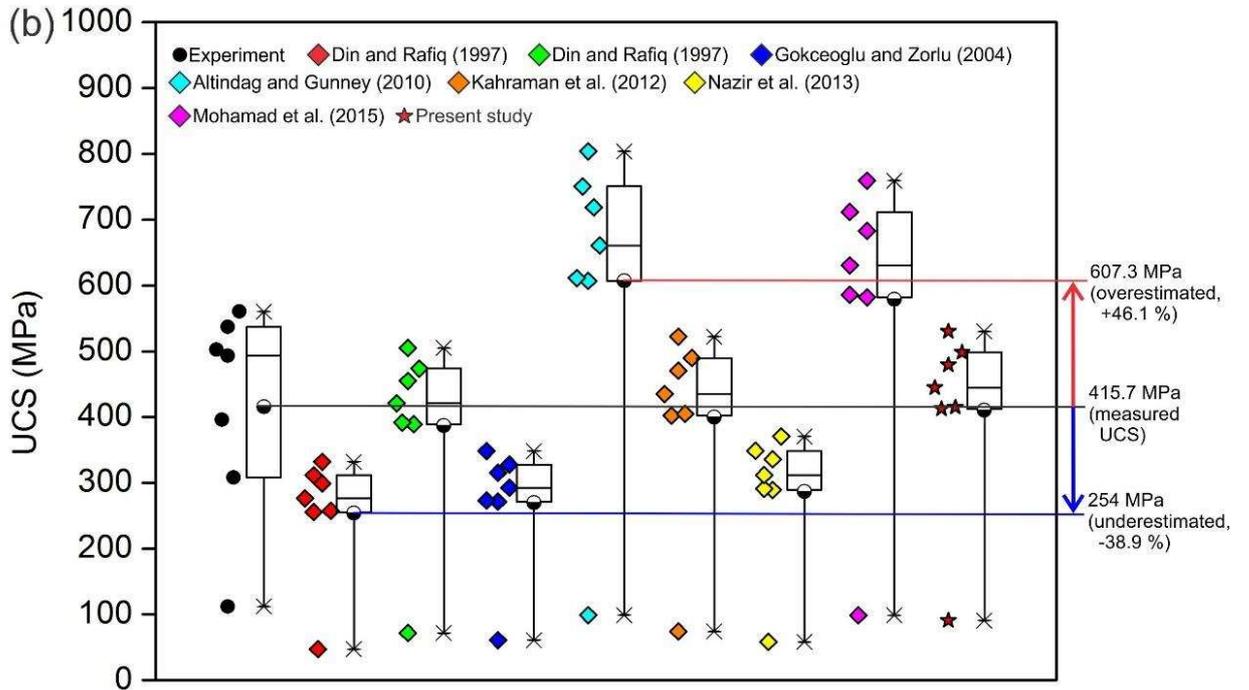


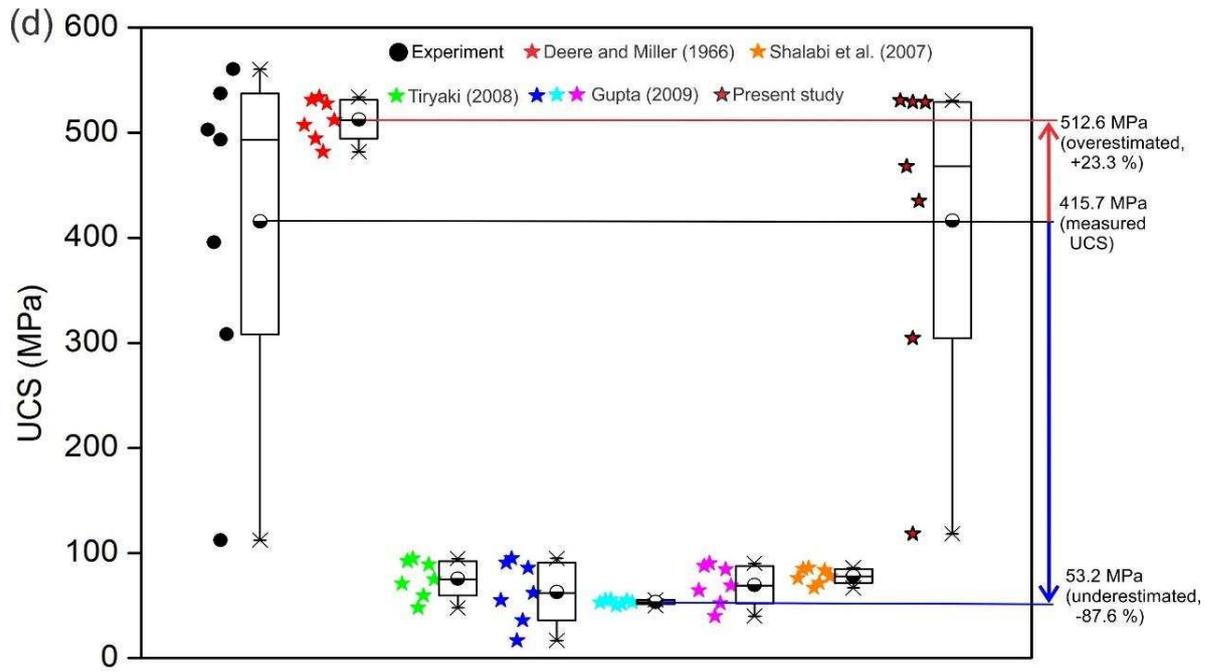
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617 **Fig. 10** Performance of the proposed equations (Table 3) in the UCS
 618 estimations. The 100 % line and the region bonded by the 80 % and 120 %
 619 lines are included for quantitative assessment.

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628 **Fig. 11** Comparison between measured UCS and estimated UCS using
 629 previously proposed equations and presently proposed equations. (a) UCS vs.
 630 $I_{s(50)}$; (b) UCS vs. σ_t ; (c) UCS vs. V_p and (d) UCS vs. ρ . Box charts are also
 631 included for assessing some key values of the data. See text for details.

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633 **Tables**634 **Table 1** Nomenclature and origin of flint samples

Nomenclature of samples	Geological formation	Geographic location	Country
BNLUK	Burnham Chalk Formation	North Landing, Yorkshire	United Kingdom
SESUK	Seaford Chalk Formation	East Sussex	United Kingdom
BLSUK	Burnham Chalk Formation	Lincolnshire	United Kingdom
SDFR	Seaford Chalk Formation	Dieppe	France
LMFR	Lewes Chalk Formation	Mesnil-Val Plage	France
TSDK	Tor Chalk Formation	Stevns Klint	Denmark
TMDK	Tor Chalk Formation	Møns Klint	Denmark

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648 **Table 2** Properties and experimental results of flint samples.

Sample	Density, ρ (Mgm ⁻³)	Compressional velocity, V_p (ms ⁻¹)	Shear velocity, V_s (ms ⁻¹)	Shore hardness	Cerchar Abrasivity index, CAI
BNLUK	2.43±0.12 (8)	5029.76±483.88 (8)	3530.77±307.30 (9)	109.48±5.80 (120)	3.39±0.53 (45)
SESUK	2.69±0.10 (20)	5493.96±95.93 (20)	3490.54±91.43 (16)	111.56±2.90 (320)	3.56±0.56 (52)
BLSUK	2.49±0.05 (20)	5431.47±306.81 (20)	3471.48±164.21 (40)	106.63±2.59 (86)	3.48±0.46 (50)
SDFR	2.67±0.13 (20)	5465.17±286.72 (20)	3571.27±166.95 (10)	108.45±2.32 (280)	3.66±0.47 (40)
LMFR	2.66±0.12 (20)	5479.06±223.43 (20)	3538.61±122.32 (10)	105.45±3.07 (80)	3.90±0.55 (40)
TSDK	2.55±0.01 (16)	5539.90±501.71 (16)	3609.96±229.23 (8)	111.76±2.22 (280)	3.59±0.35 (50)
TMDK	2.58±0.01 (5)	5333.51±210.55 (5)	3476.06±210.55 (5)	--	3.32±0.32 (50)
Sample	Tensile strength, σ_t (MPa)	Point load, $I_{s(50)}$ (MPa)	Uniaxial compressive strength, σ_c (MPa)	Young's modulus, E (GPa)	Poisson's ratio, ν
BNLUK	6.97±2.63 (8)	6.97±3.85 (52)	112.19±71.04 (10)	--	--
SESUK	44.35±20.61 (49)	30.55±11.87 (82)	537.23±176.41 (20)	80.49±13.34 (20)	0.12±0.04 (20)
BLSUK	37.90±10.09 (12)	15.17±4.86 (17)	308.20±169.32 (16)	69.14±10.54 (10)	0.13±0.03 (10)
SDFR	38.15±13.65 (20)	26.06±8.93 (20)	502.88±150.35 (20)	85.13±16.12 (20)	0.12±0.03 (20)
LMFR	41.01±12.49 (20)	29.12±6.50 (20)	560.31±178.41 (20)	85.44±13.28 (20)	0.11±0.04 (20)
TSDK	49.24±5.67 (12)	24.60±9.17 (14)	493.18±222.13 (13)	74.01±25.01 (10)	0.14±0.05 (10)
TMDK	46.19±11.02 (6)	27.40±5.76 (7)	395.76±173.07 (5)	84.95±19.01 (6)	0.13±0.04 (6)

649 Note: The figure in the brackets represents the number of specimens / repetitions in each test.

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654 **Table 3** Proposed equations for estimating the uniaxial compressive strength of extremely hard flint.

Parameters	Equations	R ²
UCS, $I_{s(50)}$	$UCS=17.6 I_{s(50)}+13.5$	0.88
UCS, σ_t	$UCS=10.4\sigma_t +18.2$	0.63
UCS, ρ	$UCS=-47454.4+35905.6\rho-6716.8\rho^2$	0.90
UCS, V_p	$UCS=0.91V_p-4500.6$	0.80

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670 **Appendix**671 **Table A1** Representative correlations between UCS and point load strength index ($I_{s(50)}$).

Equations	Lithology	Number of samples (specimens) tested	References
$UCS=15.3I_{s(50)}+16.3$	-	-	D'Andrea et al. (1964) ⁴²
$UCS=20.7I_{s(50)}+29.6$	Basalt, dolomite, sandstone, limestone, marble (US)	28 samples (257 specimens)	Deere and Miller (1966) ⁹
$UCS=24I_{s(50)}$	-	-	Broch and Franklin (1972) ⁴³
$UCS=23I_{s(50)}$	-	-	Bieniawski (1975) ⁴⁴
$UCS=10I_{s(50)}$	Brittle rocks	-	Hoek (1977) ⁴⁵
$UCS=5I_{s(50)}$	Soft rocks	-	
$UCS=18.7I_{s(50)}-13.2$	Sandstone, sandy shale (India)	-	Singh (1981) ²⁸
$UCS=(20 - 25)I_{s(50)}$	-	-	ISRM (1985) ¹¹
$UCS=23I_{s(50)}+13$	Limestone, sandstone, marble (US)	14 samples (140 specimens)	Cargill and Shakoor (1990) ⁴⁶
$UCS=19I_{s(50)}-12.7$	Kozlu-Zonguldak sandstone (Turkey)	15 specimens	Ulusay et al. (1994) ¹²
$UCS=14.3I_{s(50)}$	Biohermal lime rocks (US)	3 samples (57 specimens)	Smith (1997) ⁴⁷

$UCS=24.5I_{s(50)}$	Sandstone, limestone (US)	3 samples (75 specimens)	
$UCS=(7 - 68)I_{s(50)}$	Limestone, chalk, sandstone (UK)	-	Hawkins (1998) ⁴⁸
$UCS=8.41I_{s(50)}+9.51$	Limestone, sandstone, etc. (Turkey)	11 specimens	Kahraman (2001) ¹⁶
$UCS=23.62I_{s(50)}-2.69$	Coal measure rocks-marl etc. (Turkey)	26 specimens	
$UCS=(13 - 28)I_{s(50)}$	Limestone, marly-limestone, sandstone, marlstone (Greece)	5 samples (20-93 specimens)	Tsiambaos and Sabatakakis (2004) ⁴⁹
$UCS=(8-18)I_{s(50)}$	Porous chalks	12-18 specimens	Palchik and Hatzor (2004) ⁵⁰
$UCS=9.08I_{s(50)}+39.32$	Basalt, granite, limestone, travertine, quartzite, marble, etc. (Turkey)	11 samples	Fener et al. (2005) ⁵¹
$UCS=18I_{s(50)}$	Granitic rocks (Hong Kong, China)	40 specimens	Basu and Aydin (2006) ⁵²
$UCS=12.4I_{s(50)}-9.08$	Hafik Formation gypsum (Turkey)	121 specimens	Yilmaz and Yuksek (2009) ⁵³
$UCS=11.1I_{s(50)}+37.659$	Jaduguda uranium schist (India)	19 specimens	Basu and Kamran (2010) ⁵⁴
$UCS=5.575I_{s(50)}+21.92$	Gachsaran Formation gypsum (Iran)	15 specimens	Heidari et al. (2012) ⁵⁵
$UCS=16.4I_{s(50)}$	Hydrothermally altered volcanoclastic rocks (Japan)	44 specimens	Kohno and Maeda (2012) ⁵⁶
$UCS=(14 - 24)I_{s(50)}$	Gabbro, sandstone, limestone, shale, quartzite etc. (India)	11 samples (106 specimens)	Singh et al. (2012) ⁵⁷

$$\text{UCS}=17.5I_{s(50)}+1$$

Hamurkesen Formation basalt

37 specimens

Karaman et al. (2015) ⁵⁸

Berdiga Formation limestone
(Turkey)

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687 **Table A2** Representative correlations between UCS and tensile strength (σ_t)

Equations	Lithology	Number of samples (specimens) tested	Methodology	References
$UCS=(6.74 - 10.26)\sigma_t$	Granite and limestone	-	Brazilian test	Din and Rafiq (1997) ²⁹
$UCS=6.8\sigma_t + 13.5$	Andesite, agglomerate, greywacke, limestone, spilite, schist (Ankara basin, Turkey)	82 samples	Brazilian test	Gokceoglu and Zorlu (2004) ¹³
$UCS=12.308\sigma_t^{1.0725}$	-	-	Brazilian test	Altindag and Guney (2010) ⁵⁹
$UCS=10.61\sigma_t$	Granite, basalt, sandstone, limestone, marble (Turkey)	46 samples	Brazilian test	Kahraman et al. (2012) ¹⁴
$UCS=9.25 \sigma_t^{0.947}$	Limestone	20 specimens	Brazilian test	Nazir et al. (2013) ⁶⁰
$UCS=15.361\sigma_t - 10.303$	Shale, old alluvium, iron pan (Nusajaya, Malaysia)	40 samples (160 specimens)	Brazilian test	Mohamad et al. (2015) ⁶¹

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693 **Table A3** Representative correlations between UCS and P-wave velocity (V_p)

Equations	Lithology	Number of samples (specimens) tested	References
$UCS=0.03554V_p-55$	Granite, granodiorite (Turkey)	19 samples	Tuğrul and Zarif (1999) ¹⁵
$UCS=9.95(10^{-3}V_p)^{1.21}$	Limestone, sandstone, coal measure rocks (Turkey)	37 specimens	Kahraman (2001) ¹⁶
$UCS=0.0315V_p-63.7$	Limestone, dolomite, marble (Turkey)	13 specimens	Yaşar and Erdoğan (2004b) ¹⁷
$UCS=0.004V_p^{1.247}$	Granite (Portugal)	9 samples	Sousa et al. (2005) ³⁰
$UCS=0.05293V_p-192.93$	Sandstone, limestone, cement motar (Antalya, Turkey)	150 specimens	Çobanğlu and Çelik (2008) ⁶²
$UCS=0.0642V_p-117.99$	Basalt, sandstone, phyllite, schist, coal, shaly rock	9 samples (48 specimens)	Sharma and Singh (2008) ⁶³
$UCS=0.11V_p-515.56$	Serpentinities (Greek)	32 samples	Diamantis et al. (2009) ⁴⁰
$UCS=0.1333V_p-227.19$	Sandstone, shale, coal (India)	12 samples	Khandelwal and Singh (2009) ⁶⁴
$UCS=165.058e^{(-4451/V_p)}$	Limestone, sandstone, marlstone (Iran)	64 samples	Moradian and Behnia (2009) ⁶⁵

$UCS=0.0494V_p-1.67$	Travertine, limestone, schist (Turkey)	9 samples (90 specimens)	Yagiz (2011) ¹⁸
$UCS=0.033V_p-34.83$	Granite, sandstone, limestone, dolomite, marble (India)	13 samples	Khandelwal (2013) ⁶⁶
$UCS=0.027V_p-19.759$	Granite, granodiorite (Turkey)	6 samples (75 specimens)	Yesiloglu-Gultekin (2013) ⁶⁷
$UCS=0.026V_p-20.207$	Marly Formation rocks (Shiraz, Iran)	40 samples	Azimian et al. (2014) ⁸
$UCS=3.67*(0.001V_p)^{2.14}$	Sarvak and Asmari limestone (Iran)	45 specimens	Najibi et al. (2015) ¹⁹

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704 **Table A4** Representative correlations between UCS and density (ρ)

Equations	Lithology	Number of samples (specimens) tested	References
$UCS=(28812.5\rho-52.586)*0.0069$	Basalt, dolomite, sandstone, limestone, marble (US)	28 samples (257 specimens)	Deere and Miller (1966) ⁹
$UCS=73\rho-110.32$	Dolomite (Chicago, US)	58 specimens	Shalabi et al. (2007) ³⁹
$UCS=178.33\rho-384.65$	-	-	Tiryaki (2008) ⁶⁹
$UCS=298\rho - 706$	Granite, gneiss, quartzite, (India)	29 samples	Gupta (2009) ⁷⁰
$UCS=21\rho-1$			
$UCS=192\rho-425.8$			

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