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1 **An improved method to represent DEM uncertainty in glacial lake**
2 **outburst flood propagation using stochastic simulations**

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6

7 **Abstract**

8 Modelling glacial lake outburst floods (GLOFs) or ‘jökulhlaups’, necessarily involves the
9 propagation of large and often stochastic uncertainties throughout the source to impact
10 process chain. Since flood routing is primarily a function of underlying topography,
11 communication of digital elevation model (DEM) uncertainty should accompany such
12 modelling efforts. Here, a new stochastic first-pass assessment technique was evaluated
13 against an existing GIS-based model and an existing 1D hydrodynamic model, using three
14 DEMs with different spatial resolution. The analysis revealed the effect of DEM uncertainty
15 and model choice on several flood parameters and on the prediction of socio-economic
16 impacts. Our new model, which we call MC-LCP (Monte Carlo Least Cost Path) and which
17 is distributed in the supplementary information, demonstrated enhanced ‘stability’ when
18 compared to the two existing methods, and this ‘stability’ was independent of DEM choice.
19 The MC-LCP model outputs an uncertainty continuum within its extent, from which relative
20 socio-economic risk can be evaluated. In a comparison of all DEM and model combinations,
21 the Shuttle Radar Topography Mission (SRTM) DEM exhibited fewer artefacts compared to
22 those with the Advanced Spaceborne Thermal Emission and Reflection Radiometer Global
23 Digital Elevation Model (ASTER GDEM), and were comparable to those with a finer
24 resolution Advanced Land Observing Satellite Panchromatic Remote-sensing Instrument for

1 Stereo Mapping (ALOS PRISM) derived DEM. Overall, we contend that the variability we
2 find between flood routing model results suggests that consideration of DEM uncertainty and
3 pre-processing methods is important when assessing flow routing and when evaluating
4 potential socio-economic implications of a GLOF event. Incorporation of a stochastic
5 variable provides an illustration of uncertainty that is important when modelling and
6 communicating assessments of an inherently complex process.

7 **Keywords:** GLOF, Flow path, Monte Carlo, Uncertainty, Digital Elevation Model, Bhutan

8 **1. Introduction**

9 Deglaciation is giving rise to a globally distributed increase in the number and size of glacial
10 lakes (Carrivick and Tweed, 2013). In the Himalaya the trend of lake development is spatially
11 variable in response to climate and the evolution of debris-covered glaciers (Gardelle et al.,
12 2011; Benn et al., 2012; Nie et al., 2013). Sudden outbursts of large volumes of water from
13 such lakes, termed Glacial Lake Outburst Floods (GLOFs) or ‘jökulhlaups’, can be hazardous
14 to downstream communities and infrastructure. Hazardous lake identification is essential to
15 direct timely remedial works, further investigations, or implement early warning strategies
16 (Worni et al., 2012). The time to peak flow is usually short and lacks warning, meaning
17 assessments of likely flood inundation become a primary tool for disaster preparedness
18 (UNDMT, 2005; Koike and Takenaka, 2012; Takenaka et al., 2012). Application of a flood
19 model can provide an indication of downstream exposure to a GLOF event, since flood
20 propagation is primarily a function of the underlying topography and the GLOF hydrograph.

21 The shape and magnitude of the lake breach hydrograph determines the distribution and
22 timing of the flood event, and is therefore a key component of a hazard assessment (Westoby
23 et al., 2014). Complete GLOF hazard assessments follow a sequential source to impact
24 methodology that usually comprises an investigation of lake dynamics, lake surroundings,

1 breach scenarios, and creating downstream risk assessments highlighting potential inundation
2 zones (Worni et al., 2014). The current state of knowledge of the GLOF process chain was
3 reviewed by Worni et al. (2014) and Westoby et al. (2014), who both highlighted that studies
4 evaluating the propagation of uncertainty throughout the chain are lacking. Sources of
5 uncertainty are discussed by Westoby et al. (2014) and Westoby et al. (2015). Briefly, they
6 concern the initiating trigger mechanism, parameterisation of the dam-breach and initial dam
7 conditions, and the hydrodynamic modelling itself. The hydrodynamic modelling has
8 uncertainties arising from the topographic resolution used; channel roughness coefficients;
9 model dimensionality; and model coupling between the trigger, breach, and flood. A
10 probabilistic unified GLOF modelling workflow implemented by Westoby et al. (2015)
11 addresses several sources of this cascading uncertainty, but has high data requirements. Such
12 methods of addressing stochastic elements and compounding uncertainty should be
13 implemented and communicated, concurrent with use of integrated workflows for assessing
14 GLOF hazard (e.g. Huggel et al., 2002; Bolch et al., 2011; Mergili and Schneider, 2011;
15 Worni et al., 2012; Mergili et al., 2013).

16 Where field data are insufficient to implement a physically based numerical flood model, or
17 to guide their application, first-pass assessments are commonly implemented (e.g. Huggel et
18 al., 2003; Mergili and Schneider, 2011; Mergili et al., 2013). These generally utilise medium
19 resolution DEM products such as the ASTER GDEM (herein GDEM) and SRTM DEM
20 (herein SRTM) which carry greater vertical uncertainty. Such coarser datasets can still be
21 valuable for flood inundation modelling (e.g. Sanders, 2007). However, GIS-based
22 algorithms using flow direction are highly sensitive to vertical errors in such DEMs (Veregin,
23 1997; Endreny and Wood, 2001). Additionally, a river channel is often poorly defined in
24 coarse terrain data and may be offset compared to the ground truth channel and hence socio-
25 economic infrastructure. This DEM error can be compounded by sink filling, which is a DEM

1 processing routine that can remove local elevation minima representing the channel (e.g.
2 Czubski et al., 2013). Therefore communication of DEM uncertainty should accompany such
3 first-pass modelling efforts since this uncertainty is intrinsically important for understanding
4 flow propagation.

5 This paper therefore presents an inter-model comparison of two GIS-based first-pass flood
6 assessment techniques and a 1D flood model. The Modified Single-Flow-direction (MSF)
7 model developed by Huggel et al. (2003) was compared to the new MC-LCP developed
8 herein, and also to a hydrodynamic model created using HEC-RAS. The aim of this study
9 was to quantify the differences between methods when using the same underlying terrain
10 data, and when using three different DEM products.

11 The first-pass MC-LCP GLOF assessment technique developed in this study incorporates the
12 evaluation and communication of DEM uncertainty in the modelled output. It avoids the
13 requirement of a flow direction grid and hence a 'filled' DEM. This increases its utility and
14 minimises potential artefacts in low relief populated areas, for which a reliable inundation
15 output is most desired. The method is proposed as an alternative first-pass GLOF
16 vulnerability assessment for data poor regions and features a transferable, easily
17 implemented, and adaptable methodology. It is applied to a case study in Bhutan to evaluate
18 applicability in a high relief catchment using only remotely sensed data. Since this case study
19 is purely hypothetical, the MC-LCP is also validated using geomorphic evidence of the 1985
20 Dig Tsho GLOF in Nepal.

21 **2. Background**

22 2.1 Previous assessment strategies

23 GLOF hazard assessments require consideration of the probability of an event occurring and
24 the vulnerability of downstream communities and infrastructure, in order to make informed

1 decisions on the risk magnitude and hence suitable remediation or adaptation strategies.
2 Studies may adopt a qualitative (e.g. Huggel et al., 2004a), semi-quantitative (e.g. Bolch et
3 al., 2011), or quantitative approach (e.g. McKillop and Clague, 2007; Mergili and Schneider,
4 2011) using factor combinations of lake and dam characteristics, the surrounding lake
5 topography, and adjacent glacier dynamics (Emmer and Vilímek, 2013). However, their
6 combination and weighting in a hazard assessment is not standardised. Increasingly there is a
7 transition towards modelling the source-to-impact GLOF process chain, using higher-order
8 physically based models (Worni et al., 2014). However, constructing physically-based flood
9 models is limited by the uncertainty and availability of remotely sensed parameters such as
10 dam geometry (e.g. Worni et al., 2012), and the time investment in creating and applying
11 such models. They nevertheless provide an enhanced understanding of GLOF flow
12 characteristics where the paucity of high resolution terrain data limits most modelling efforts
13 to using flow routing algorithms or 1D models, which cannot fully represent flow dynamics
14 (Westoby et al., 2014).

15 Many previous studies have made use of medium-resolution DEM products such as ASTER-
16 derived DEMs (e.g. Byers et al., 2013), and the GDEM and SRTM DEMs (e.g. Wang et al.,
17 2012). These DEMs permit catchment-scale coverage where a similar extent of finer
18 resolution products such as photogrammetry or airborne laser scanning would be otherwise
19 limited logistically or prohibitively expensive. Additionally, the low data processing and
20 storage requirements of the GDEM and SRTM permit their rapid exploitation and
21 interrogation for simple flow models. However, these products contain inherent uncertainty
22 in grid cell elevations of the same order in magnitude to a GLOF flow depth. This is most
23 prevalent in mountainous terrain (Hayakawa et al., 2008; ASTER GDEM Validation Team,
24 2011; Kolečka and Kozak, 2014) since pixel resolution is less representative of terrain
25 characteristics (Fisher and Tate, 2006). For the SRTM, elevation uncertainty contributes to a

1 systematic negative elevation bias with increasing altitude (Paul, 2008), identified in the
2 French Alps (Berthier et al., 2006), and confirmed for the Himalaya (Berthier et al., 2007),
3 though its precise origin is not apparent. For the GDEM, Rexer and Hirt (2014) demonstrated
4 improved vertical accuracy compared to the SRTM over mountainous terrain in Australia.
5 However, forested land cover contributed to a positive elevation bias for both the GDEM
6 (ASTER GDEM Validation Team, 2011; Rexer and Hirt, 2014) and SRTM (Sun et al., 2003;
7 Shortridge and Messina, 2011; Koleccka and Kozak, 2014).

8 Quantification of the potential implications of spatially variable elevations biases for GLOF
9 modelling requires ground truth data, but nevertheless should be considered on a catchment
10 by catchment basis. For example, GLOFs originate in high-altitude vegetation sparse
11 environments and will generally flow through increasingly vegetated reaches, such that a
12 general trend of increasingly positive elevation bias could exist with distance downstream in
13 forested Himalayan reaches. Consideration of the uncertainty and spatial autocorrelation in
14 medium resolution topographic products should therefore accompany their usage.

15 GIS-based assessments of GLOF routing and inundation provide a first-pass assessment tool
16 to identify vulnerable catchments for further analysis (Huggel et al., 2004b), but may also be
17 utilised to identify likely inundation characteristics at finer scales and hence an initial
18 assessment of relative risk (Nussbaumer et al., 2014). The ArcGIS-based MSF model
19 developed by Huggel et al. (2003) has seen usage for modelling GLOFs and debris flows
20 (e.g. Huggel, 2004; Huggel et al., 2004b; Schneider et al., 2008; Frey et al., 2010; Iribarren
21 Anacona et al., 2014; Nussbaumer et al., 2014). Similar procedures are available in GRASS
22 GIS, weighting the flow propagation using local slope and flow direction (e.g. Mergili and
23 Schneider, 2011; Gruber and Mergili, 2013). Hydrodynamic modelling is also feasible on
24 global DEMs, though uncertainty is increased as a consequence of poorly resolved channel
25 networks (e.g. Wang et al., 2012; Czubski et al., 2013).

1 Models dependent on the maintenance of flow direction require a hydrologically correct
2 DEM with spurious sinks 'filled' to avoid the algorithm terminating. This filling raises pitted
3 areas of the DEM surface and can lead to parallel flow artefacts where lengths of the channel
4 are filled to similar elevations (Melles et al., 2011). Uncertainty introduced by filling is
5 compounded by underlying DEM uncertainty; commonly reported as a root mean square
6 error (RMSE). Therefore communication of the uncertainty in modelled flood extents would
7 both increase the confidence in and utility of such first-pass assessments. An evaluation of
8 DEM uncertainty can be gained by using stochastic simulation techniques such as the Monte
9 Carlo method (Lindsay and Evans, 2008). In this study, a Monte Carlo approach allows
10 modelled inundated areas to be evaluated based on their inundation frequency following an
11 iterative process of flow routing over sequential terrain realisations, hence making predictions
12 of socio-economic impacts more robust.

13 2.2 Bhutan case study

14 The Chamkhar Chu catchment in Bhutan was selected to exemplify a data-scarce site; hence
15 requiring the use of globally available DEMs and a more simplistic inundation modelling
16 approach. Here, the paucity of fine-resolution data sets, documented past events, or field
17 surveys, limits the possible application of physically-based flood models. Simple GIS-based
18 first-pass flood assessments such as the MSF or the MC-LCP proposed, therefore offer a fast
19 and transferable alternative for delineating likely GLOF flow paths whilst also considering
20 lateral extent.

21 Addressing GLOF risk became a priority in Bhutan following the 1994 GLOF event from
22 Lugge Tsho (Watanbe and Rothacher, 1996; Ghimire, 2005). Remediation and early warning
23 strategies were implemented through two internationally funded efforts involving the United
24 Nations Development Program (UNDP) (Meenawat and Sovacool, 2011). Substinence

1 communities using mountain streams for agricultural irrigation are often located on elevated
2 terraces away from the valley bottoms and hence flood risk (Wangdi and Kusters, 2012) but
3 the long runout distance of GLOF peaks increases the vulnerability of downstream
4 infrastructure and settlements located closer to the river channel (Takenaka et al., 2012).
5 Safeguarding hydropower generation is of particular importance in Bhutan because it
6 represents 99 % of electricity generation (Jamtsho, 2012), accounts for an estimated 22 % of
7 Bhutan's GDP, and has secured investments in future run-of-the-river plants that are
8 susceptible to runoff variability (NEC, 2009).

9 A study by Mool et al. (2001) identified the existence of 24 potentially dangerous glacial
10 lakes in Bhutan (Figure 1); although a more recent GIS assessment considering dam slope
11 suggests fewer exist (Fujita et al., 2013). Nevertheless, glacial lake expansion in Bhutan
12 (Komori, 2008) and the expected enhanced glacial lake development on stagnant, debris-
13 covered glaciers (Kääb, 2005), suggests the potential impact of future GLOFs should be
14 continually evaluated.

15 [Figure 1 approximate location]

16 **3. Data sources and pre-processing**

17 3.1 DEMs

18 The ASTER GDEM V2. has ca. 30 m horizontal resolution and a reported vertical RMSE of
19 ± 15.1 m over mountainous terrain (ASTER GDEM Validation Team, 2011). The SRTM
20 DEM 4.1 has 90 m horizontal resolution and a vertical RMSE of ca. ± 16 m (Rexer and Hirt,
21 2014). Both are surface models, such that stated elevations reflect the tops of dense
22 vegetation and built up areas. However, the SRTM sampling coincided with leaf-off
23 conditions for northern hemisphere deciduous forests, suggesting that elevation data over
24 forested regions may represent a mix signal between tree height and ground level elevations

1 (ASTER GDEM Validation Team, 2011). In the absence of finer resolution DEMs, both
2 global datasets are used for hydrodynamic modelling (e.g. Gichamo et al., 2012; Wang et al.,
3 2012). In this study, a third DEM was created at 15 m horizontal resolution using stereo
4 ALOS PRISM scenes of 2.5 m resolution, and ground control points (GCPs) derived from
5 Google Earth. The accuracy and resolution of the Google Earth elevation data is unknown,
6 although good association has been demonstrated with ASTER and SRTM elevation data
7 (e.g. Rusli et al., 2014). In this study, we assumed that the GCPs were of similar accuracy to
8 the SRTM data and used a vertical RMSE of ± 16 m for the ALOS DEM. Summary statistics
9 showing the relative accuracy of each DEM are presented in Table 1, highlighting that our
10 ALOS DEM is most comparable to the SRTM. The GDEM and SRTM were resampled to 15
11 m resolution using bilinear interpolation to provide a common pixel size for analysis in this
12 study.

13 The spatial resolution of the GDEM and SRTM means that they cannot accurately represent a
14 river valley with steep banks. Specifically, these DEMs contain spurious peaks and
15 depressions that require pre-processing to improve flow routing (Fisher and Tate, 2006;
16 Pitman et al., 2013). Since a DEM is modified by this pre-processing, several correction
17 methods were considered in this study using the optimized pit removal tool (Soille, 2004;
18 CRWR, 2013). All three DEMs were processed to minimise net change for the HEC-RAS
19 and MC-LCP models. This was carried out automatically with the optimized pit removal tool
20 which uses cut and fill operations to minimise DEM sinks. In this study, these are referred to
21 as optimized pit removal net (OPRN) DEMs (Figure 2a). In contrast, we filled all three
22 DEMs for use with the MSF model, which is a requirement for it to run and caused
23 comparatively higher cell modification (Table 2). The mean difference between OPRN and
24 'filled' DEM profiles is smallest for the SRTM (5.6 m), followed by the ALOS (7.5 m) and
25 GDEM (23.4 m) (Table 1), suggesting a higher incidence of channel artefacts in the GDEM.

1 [Figure 2 approximate location]

2 3.2 Socio-economic

3 Socio-economic impact was evaluated using a 2010 land cover dataset produced by Gilani et
4 al., (2014), supplemented with a buildings and road network layer digitised from Google
5 Earth imagery.

6 **4. Methods**

7 The MC-LCP model was evaluated against the MSF GIS-based model and a 1D
8 hydrodynamic model using three DEM products of different initial resolutions, following the
9 general workflow outlined in Figure 3.

10 [Figure 3 approximate location]

11 4.2 Monte Carlo Least Cost Path model (MC-LCP)

12 The MC-LCP model (Figure 4) was developed as part of this study and incorporates an
13 iterative cost path analysis and Monte Carlo loop of modelled DEM uncertainty, and was
14 implemented in ArcGIS 10.2. Similar to the MSF model, the MC-LCP has no physical basis
15 and is proposed as a first-pass assessment technique. Incorporating stochastic DEM
16 uncertainty within the model facilitates lateral spread and produces relative inundation
17 probabilities for each DEM cell. Spatial autocorrelation of DEM error is a recognised quality
18 issue, though is difficult to assess without a reference dataset and only a RMSE is commonly
19 provided for global datasets (Carlisle, 2005; Wechsler, 2007). In this study, a spatial
20 autocorrelation of RMSE uncertainty was introduced in each model iteration as six 15 m
21 pixels for the GDEM, six 15 m pixels for the SRTM which is degraded from 30 m to 90 m
22 horizontal resolution, and three 15 m pixels for the ALOS DEM, using a focal neighbourhood
23 filter following Hebel and Purves (2009) and Zandbergen (2011).

1 The MC-LCP model workflow is detailed in Figure 4, which describes the processes involved
2 in the three aspects of the model: modelling DEM uncertainty, creating the cost layers, and
3 modelling least cost path iterations. For each iteration, a normally distributed raster layer was
4 created with a standard deviation matching the RMSE of each DEM and a mean of zero
5 (Gatzliolis and Fried, 2004; Zandbergen, 2011). Spatially autocorrelated uncertainty was then
6 introduced with a neighbourhood filter. Each iteration of the error model was then added to
7 the initial DEM, thereby creating a new terrain realisation (Lindsay and Evans, 2008). A cost
8 path analysis was conducted between a start and end location defining the study reach, using
9 equally weighted cost layers of vertical elevation difference from the river channel and local
10 slope. This produced a pixel-wide downstream least cost path. Subsequent iterations
11 producing new least cost paths were sequentially added, hence the final output represented
12 the number of times each cell was considered a least cost path, and therefore inundated. In
13 this study, outputs were initially evaluated at 50, 100, 500, 1000, and 1500 iterations for each
14 DEM. Stable inundation extents were apparent at 500 model iterations (Figure 5), hence all
15 subsequent least cost path results were derived using 500 iterations. The top one percent of
16 inundated cells were excluded in this study to remove spurious paths. This exclusion is an
17 arbitrary decision based on an inspection of the output where spurious paths diverge notably
18 from the main distribution.

19 [Figure 4 approximate location]

20 [Figure 5 approximate location]

21 The MC-LCP model design produced a distribution of least cost paths down the study reach
22 for which it is conceivable that a flood could inundate. The likelihood of inundation is
23 represented by the inundation frequency output. In this study the lateral spread of least cost
24 paths represents our first-pass assessment of GLOF extent. This is similar to a ‘random walk’
25 procedure implemented in GRASS GIS by Mergili and Schneider (2011) and Gruber and

1 Mergili (2013), which produced a lateral inundation extent weighted for local slope, but did
2 not incorporate DEM uncertainty.

3 Applying a variable threshold to the inundation frequency output could be used to delineate a
4 river network, since a high inundation frequency would be indicative of the valley bottom.
5 Similar thresholding is used on the output of the flow accumulation function in ArcGIS to
6 delineate a stream and river network, however this requires a 'filled' DEM (Melles et al.,
7 2011). Where a ground truth river is unavailable, the model can be used to generate a least
8 cost path down the study reach using the original DEM, i.e. no error model is applied. This
9 would represent the input river channel (Figure 4) and can be derived from an OPRN
10 processed DEM.

11 4.2.1 Dig Tsho validation

12 Although the MC-LCP output is not directly applicable to any particular magnitude of GLOF
13 event, validation of the MC-LCP output was undertaken using geomorphic evidence of the
14 1985 Dig Tsho GLOF extent, which featured an estimated peak discharge of ca. $2000 \text{ m}^3 \text{ s}^{-1}$
15 (Vuichard and Zimmermann, 1987). The six locations of GLOF geomorphic evidence
16 mapped by Cenderelli and Wohl (2001) was used as the flood extent, which was compared to
17 the MC-LCP modelled extent for the GDEM (Figure 6, Table 3). This validation used a least
18 cost path derived river, therefore representing the MC-LCP in its simplest state without a
19 ground truth river network.

20 Application of the MC-LCP to the 1985 Dig Tsho GLOF event revealed good spatial
21 association with geomorphic evidence of the known GLOF extent (Figure 6, Table 3). Slight
22 discrepancies between the 'flow path' of the MC-LCP and the field-measured extent were
23 apparent in reach L1 and L8, which was attributed to the DEM resolution (30 m)
24 misrepresenting the high relief channel, rather than potential contemporary river channel
25 migration reflected in the DEM. However, overall the MC-LCP provided a good

1 representation of the 1985 GLOF extent with a mean classification accuracy of 78 % for the
2 reaches mapped (Table 3).

3 [Figure 6 approximate location]

4 4.2.2 Additional land cover cost

5 Following the study of Nussbaumer et al. (2014), which considered multi-temporal GLOF
6 risk following land use change, we modelled the influence of including an additional land
7 cover cost factor into the MC-LCP, which we denote herein as MC-LCP LC; specifically on
8 the inundated area and whether high-cost land covers such as woodland would consequently
9 experience lower inundation. This was carried out using a simple reclassification of the land
10 cover layer using Manning's N hydraulic roughness values following Chow (1959). When
11 multiplied by 1000, these 'land cover costs' ranged from 10 – 100, which created a
12 normalised scale between the three cost layers. Appropriate weightings for including cost
13 factors are speculative; hence this study primarily focuses on the utility of the MC-LCP
14 without this additional cost factor.

15 4.3 Modified Single-Flow-direction (MSF) model

16 Modelled flow in the MSF model has no physical basis and is solely a function of underlying
17 terrain data, promoting its usage as a first-order assessment for GLOF flow path modelling
18 (Huggel, 2004). It uses ArcGIS's D8 flow routing method and Path Distance tool, allowing
19 flow to propagate downstream following the steepest descent, with up to 45° of lateral
20 diversion. A methods workflow was outlined by Gruber et al. 2009. The MSF model can be
21 stopped when a threshold run-out distance is reached based on the average channel slope
22 from the source. However, for this study the flow exceeded the end of the study reach in all
23 scenarios. The MSF model output reflects a qualitative likelihood of inundation, accounting

1 for increased flow resistance with lateral spread and distance downstream (Huggel et al.,
2 2003).

3 4.4 HEC-RAS model

4 1D flood modelling was carried out in HEC-RAS 4.1.0, which has previously been used for
5 modelling GLOF scenarios and reconstructing past events (e.g. Cenderelli and Wohl, 2003;
6 Alho et al., 2005; Bajracharya et al., 2007; ICIMOD, 2011; Osti et al., 2013; Klimes et al.,
7 2014), offering computational efficiency over long study reaches. Characteristically confined
8 and topographically steep Himalayan reaches restrict the lateral inundation extent. If
9 topographically unconfined and shallow, 2D models would better represent flow dynamics
10 (e.g. Carrivick, 2006; Stains and Carrivick, 2015). In this study, 516 cross sections were
11 added at 100-150 m intervals to capture downstream topographic and land cover changes
12 (Figure 2b), and Manning's N roughness values were allocated to respective land covers
13 following Chow (1959) and similar GLOF studies (e.g. Dussaillant et al., 2010; Jain et al.
14 2012). The HEC-GeoRAS extension in ArcGIS was used to extract cross section geometric
15 and roughness data for use in HEC-RAS. Three scenarios of unsteady flow were evaluated,
16 which represented a low, medium, and high magnitude event of $500 \text{ m}^3 \text{ s}^{-1}$, $1000 \text{ m}^3 \text{ s}^{-1}$ and
17 $2000 \text{ m}^3 \text{ s}^{-1}$ respectively. The scenarios are purely hypothetical and are derived from
18 evaluation empirical regression equations relating an estimated lake volume for potentially
19 dangerous glacial lakes in the catchment identified by the International Centre for Integrated
20 Mountain Development (ICIMOD), to a potential peak discharge. The hypothetical
21 hydrograph followed a linear rising and falling limb creating a triangular profile (e.g. Wang
22 et al., 2012). The scenarios are referred to as profiles (Pf) one, two and three. The
23 downstream boundary condition was set several kilometres below the town of Jakar (Figure
24 1) such that any errors arising from it would not affect the study reach (Brunner, 2010).

1 4.5 Flood implications

2 Finally, inundation extents and socio-economic impacts for each flood model and DEM
3 combination were compared in order to assess the implications of using the different models
4 and input DEMs. Downstream wetted width and flood depth were extracted for each cross
5 section. Since the MC-LCP and MSF models do not produce a depth output, the channel
6 width elevations of their flood extents were extracted and interpolated over the channel to
7 estimate a depth surface, and this surface was differenced from the DEM.

8 Flood depths maps produced in HEC-RAS do not consider DEM uncertainty. Hence, a Monte
9 Carlo based approach was used to communicate uncertainty within the HEC-RAS modelled
10 flood extent. Here, the same DEM error model as from the MC-LCP was applied to the HEC-
11 RAS depth map to evaluate whether each cell depth remained positive after iterative DEM
12 uncertainty realisations. Since the process does not simulate a new flow after each terrain
13 realisation (which would require a coupling between HEC-RAS and ArcGIS that was beyond
14 the scope of this study), potential inundation outside of the initial flood extent was not
15 considered,

16 **5. Results**

17 5.1 Inundated area

18 Differences in overall inundated area for each scenario represented a regional-scale model
19 comparison that provides an indication of model stability with use of each DEM, and
20 respective socio-economic and land cover inundation implications (Figure 7). The ALOS
21 DEM was the only DEM producing a similar trend across all models. With the exception of
22 the MC-LCP LC scenarios, the ALOS DEM consistently produced the smallest inundated
23 area (Figure 7).

1 At a regional-scale, model sensitivity to DEM inputs is indicated by the inter-DEM inundated
2 area range. Here, a smaller range indicates a lower dependence of the model on the terrain
3 data used. The MC-LCP produced the smallest range at 0.85 km^2 , followed by HEC-RAS
4 with a minimum range considering all profiles of 2.48 km^2 , and the MSF model at 4.86 km^2
5 (Table 4, Figure 7). Overall, the largest inundated areas were produced by the MC-LCP for
6 all DEMs. However, these areas reduced when a land cover cost factor was introduced
7 (Figure 7a). The inundated areas of the MSF model were most comparable to the HEC-RAS
8 scenarios. HEC-RAS profiles 1 - 3 displayed the largest inundated range on the GDEM at
9 0.74 km^2 , followed by the ALOS DEM and SRTM at 0.52 km^2 and 0.48 km^2 respectively
10 (Figure 7b, Table 4).

11 [Figure 7 approximate location]

12 5.2 Inundated extents

13 Reporting total inundated area does not consider flow propagation, which was examined
14 using downstream wetted width and visual inspection of flood extents between the models
15 and for each DEM (Figure 8, 9). Wetted width variability due to DEM choice originates from
16 the different initial products resolutions and hence variable river channel representation, and
17 the DEM quality following pre-processing. Here, the ALOS and SRTM DEMs produced the
18 smoothest downstream channel profiles following OPRN processing (Figure 2a). Notably the
19 'filled' SRTM and GDEM displayed a large positive elevation offset in the lower reach, which
20 reflected the prevalence of artefacts in the original DEMs in this area (Figure 2a, b).

21 [Figure 8 & 9 approximate location]

22 The MC-LCP produced a consistently higher downstream wetted width (Figure 9) which
23 reflects the overall larger inundated area (Figure 7a). Wetted width for the HEC-RAS Pf2
24 and the MSF models appeared similar, although it is evidently more variable downstream for

1 the HEC-RAS scenario (Figure 9b, c). A comparatively wider initial flood extent exists in all
2 models between 0 - 10 km downstream, which was most prevalent in the HEC-RAS
3 scenarios (Figure 9b). Similarly, all models suggested an increased inundation extent in the
4 lower 10 km of the study reach, which was of greatest magnitude for the MSF model on the
5 SRTM and GDEM (Figure 8c, 9c). This corresponded partly to where the valley bottom
6 becomes wider and to where settlements are located (Figure 2).

7 Intra-model variability in downstream flood extent was generally greatest for the GDEM,
8 whereas the ALOS and SRTM DEMs were more comparable and displayed less high
9 magnitude peaks in wetted width. The high wetted width variability for the GDEM is most
10 prominent in the HEC-RAS scenarios and for the MSF model (Figure 9b, c). In contrast to
11 the comparatively continuous flood extent output by the MC-LCP (e.g. Figure 10a, d),
12 downstream extent variability in the HEC-RAS output is highlighted by intermittent areas of
13 ponding where water backwater effects are created by the confined channel reaches (Figure
14 10b, e).

15 [Figure 10 approximate location]

16 5.3 Depth characteristics

17 Downstream maximum depth displayed greatest variability for all models when run on the
18 GDEM (Figure 9). The high incidence of low depth values for the MSF model (Figure 9c)
19 corresponded to narrow modelled flood extents, where extracting a depth value beneath an
20 interpolated surface exhibited greatest uncertainty. The physically-based HEC-RAS model
21 produces the most robust indication of downstream depth variability. For the ALOS DEM,
22 depth was generally below 20 m, which was always the case in the SRTM output (Figure 9a).
23 In contrast, the GDEM scenario depth often exceeded 30 m and was over 40 m in some cases.

1 5.4 Inundated extent artefacts

2 Parallel flow artefacts representing finer scale uncertainty in flow routing were apparent in all
3 MSF scenarios (e.g. Figure 8c, 10c, f). These artefacts were over 1 km long in some instances
4 (e.g. Figure 10c). A notable contrast was also apparent in the lower reach for the MSF ALOS
5 DEM, where a narrow flood extent contrasts with that of the SRTM and GDEM (Figure 8c,
6 9c). Artefacts of the OPRN pre-processing procedure were apparent in the HEC-RAS outputs
7 where narrowly cut channels are apparent between areas of ponding (Figure 10b, e). In
8 contrast, the MC-LCP produced a downstream continuum and represented flow 'braiding'
9 around 'higher cost' channel features (e.g. Figure 10a, d).

10 5.5 Socio-economic implications

11 Examination of inundated land cover highlighted the prevalence of forest cover adjacent to
12 the river channel and hence the importance of considering vegetation roughness in modelling
13 scenarios (Figure 2b, 7). This forest cover also identifies potential debris input and damming
14 hazard emanating from forested reaches, which was not considered in the modelling
15 framework. The susceptibility to inundation of agricultural land in the lower reach is also
16 highlighted for all scenarios (Figure 7). Comparing socio-economic vulnerabilities for the
17 HEC-RAS output revealed a general increase in building and road inundation with higher
18 magnitude flooding (Figure 7b), but inter-model comparisons revealed no clear association
19 between socio-economic cost and inundated extent, highlighting the importance of evaluating
20 local flow characteristics. For example, the MSF SRTM model predicted notably higher
21 building and road inundation despite only representing 48 % of the inundated area depicted
22 by the MC-LCP SRTM scenario (Figure 7a), since the MSF featured a large lateral extent in
23 the populated lower reach. HEC-RAS outputs displayed the smallest range of building and
24 road inundation across all scenarios, followed by the MC-LCP and the MSF model (Table 4).

1 5.6 Communication of risk

2 Each model pertains to a means of risk identification through either the number of times a
3 grid cell was inundated, the depth of inundation at a cell, or a visual indication of inundation
4 probability, for the MC-LCP, HEC-RAS, and MSF models respectively (Figure 10). Since
5 the MSF model contains a function of downstream distance, a difference in inundation
6 probability is only apparent in the vicinity of the start zone. Hence relative inundation
7 probabilities cannot be meaningfully compared at finer scales for the MSF model, in contrast
8 to the MC-LCP (Figure 10).

9 **6. Discussion**

10 The ability to conduct timely yet robust first-pass GLOF assessments is critical to direct
11 further investigations, implement mitigation efforts, and derive hazard zonation, in response
12 to climatic warming, glacial mass loss, and subsequent increased glacial lake development
13 observed in the eastern Himalaya (Gardelle et al., 2011; Benn et al., 2012). Such first-pass
14 assessments may represent the only means of GLOF flow routing where fine resolution
15 topographic data are not available for robust hydrodynamic modelling. Additionally, a
16 probabilistic GIS-based first-pass assessment is equally valuable prior to hydrodynamic
17 modelling, since flow paths and areas of interest can be rapidly derived. The inter-model
18 comparison presented here indicates that modelled flood extent, its dependence on
19 topography, and subsequent societal impacts, can vary considerably due to the combination
20 of model and DEM used.

21 The sensitivity of model output suggests that great caution should be exercised in data poor
22 regions where such first-pass assessments may guide hazard zonation strategies, or where
23 finer scale scenario implications are sought. Hence, although computationally more
24 demanding (500 iterations for the Dig Tsho validation took four hours to run on a standard

1 laptop with a 2.20 GHz processor and 8 GB of RAM), the MC-LCP stochastic simulation
2 based analysis provides a robust and informative indication of likely GLOF inundation with
3 no increased implementation time investment required of the user.

4 The proposed stochastic simulation based analysis (MC-LCP) is able to consistently represent
5 potential flood propagation without necessarily using a hydrologically correct DEM, which
6 increases its utility in high relief catchments where artefacts are more likely in global DEM
7 products (Pitman et al., 2013). Use of elevation and slope cost factors leads to standardised
8 application between catchments and the model produces an inherent communication of
9 uncertainty at the culmination of the GLOF process chain.

10 6.1 Topographic data

11 The 30 m ASTER GDEM V2 and 30 m SRTM (previously 90 m as used in this study) DEMs
12 now represent the finest resolution open access elevation data. The GDEM and 90 m SRTM
13 DEM products have seen usage in hydrodynamic modelling, with observed SRTM
14 overestimation and GDEM underestimation of channel elevation (Wang et al., 2012).
15 Although no GPS ground truth validation were available to this study, a similar observation
16 of relatively higher elevation SRTM profiles was apparent for lengths of the study reach
17 reported here (Figure 2). Studies deriving other topographic parameters have reported greater
18 SRTM reliability (Frey and Paul, 2012; Mashimbye, 2014), but also that both DEMs cannot
19 adequately represent a river channel in steep topography owing to irregular sensor sampling
20 of the valley sides (Czubski et al., 2013; Pitman et al., 2013). SRTM 30 m data, which were
21 made available for the Himalaya in early 2015, are likely to become widely used for first-pass
22 assessments. The more comparable performance between the SRTM 90 m DEM and ALOS
23 DEM shown in this study, suggests future use of the 30 m SRTM DEM will lead to greater
24 convergence with finer resolution DEM products such as the ALOS DEM used here. This

1 likely reflects the single-pass data collection of the SRTM compared to the combination of
2 scenes used in the GDEM.

3 Depression filling can be utilised to remove spurious sinks and peaks in the river channel;
4 however, this is the most impacting approach since it can remove lengths of local minima
5 which represent the river channel (Czubski et al., 2013). Therefore methods reducing terrain
6 modification such as those implemented here are preferred (Lindsay and Creed, 2005), even
7 though they bring their own demonstrated artefacts such as 'cut' river channels. The high
8 downstream variability in extracted flood variables for the GDEM (Figure 9) represents the
9 continued existence of sinks in the DEM following OPRN pre-processing (Figure 2a). The
10 prevalence of sinks in the GDEM produces a notably variable flood extent for the HEC-RAS
11 model (Figure 8b) and areas of flow ponding, which are separated by narrow channels where
12 the OPRN algorithm has 'cut' (Figure 10b, e). The narrow channels reflect high relief regions
13 of the study reach where DEM artefacts are likely to be most prevalent.

14 Overall the SRTM displays greater association with the finer resolution ALOS DEM, despite
15 the positive channel elevation offset in some areas of the reach (Figure 2a). In contrast, Wang
16 et al. (2012) found greater association between the GDEM and a finer resolution DEM when
17 comparing the suitability of the GDEM and SRTM for GLOF assessment for a reach in Tibet.
18 The greater channel gradient and high incidence of forest cover adjacent to the river channel
19 in this study may explain the increased GDEM variability. Though both DEMs are surface
20 models, the GDEM V2 represents a ten year fusion of data acquisition, whereas the SRTM
21 dataset was collecting during an 11 day mission and coincided with leaf-off conditions for
22 northern hemisphere forests (ASTER GDEM Validation Team, 2011).

1 6.2 Model comparison

2 The MSF model can be implemented quickly for several start sites, which increases its utility
3 for regional assessments. However, sensitivity to DEM resolution and quality restricts local
4 scale evaluation of flow routing (Huggel et al., 2003), as highlighted by linear artefacts and
5 variability in inundated extent between DEMs of 4.86 km². This is especially evident in the
6 lower reach of the GDEM and SRTM MSF scenarios where significant filling occurred
7 (Figure 2a), leading to a notably larger extent compared to the ALOS DEM (Figure 8c).
8 Elevation peaks in the channel profile further downstream leads to the 'fill' algorithm raising
9 the upstream DEM cells to the height of this peak to allow continued flow propagation, hence
10 creating an artificially flat and wider channel (Figure 2b). This demonstrates a notable
11 sensitivity when applying the MSF to high relief reaches where such erroneous flood extents
12 may be misinterpreted if applied on a regional scale, without finer scale flow path and DEM
13 interrogation.

14 HEC-RAS scenarios demonstrated reduced sensitivity to DEM choice compared to the MSF
15 model with a maximum inundation range between respective profiles of 2.48 km². The
16 GDEM scenario produced the least consistent flood extent, followed by the SRTM and
17 ALOS DEMs (Figure 8b, 9b). This extent variability is partly derived from issues when
18 interpolating flood extent between cross sections for the high relief study reach, but is also
19 linked to quality of the processed DEM used. Here, the GDEM displayed prominent
20 erroneous peaks in the lower reach despite the OPRN processing (Figure 2a). This amplifies
21 the ponding of water in the model where the reaches contract and expand. However, ponding
22 was observed throughout the HEC-RAS output, where narrow reaches are encountered
23 causing a natural 'backlog' of water (e.g. Carrivick et al., 2013). The sustained high-depth
24 artefact for the HEC-RAS GDEM and SRTM scenarios at the end of the study reach (Figure
25 9b) is likely owing to the high relief and heavily forested channel here. Since the DEM

1 cannot adequately resolve the channel, this promotes a reverse profile and hence a build-up of
2 water in the hydraulic model (Figure 9b). With a finer initial sampling resolution, the ALOS
3 DEM is less sensitive to such sampling artefacts.

4 Where DEM data quality is an issue and filling causes widespread degradation of the DEM,
5 as evidenced here, the MC-LCP still produces an acceptable flood extent comparable to the
6 HEC-RAS scenarios. The MC-LCP generally produced the largest flood extent and hence
7 downstream wetted width (Figure 8a, 9a). In the field of disaster mitigation, modelling the
8 worst-case scenario is perhaps more justified than underestimation (Wang et al., 2012).
9 Additionally, the MC-LCP model accounts for DEM uncertainty, hence the inundated extent
10 represents the area that was considered a path of least resistance during the iterative terrain
11 realisations.

12 Similar to the MSF, the MC-LCP model has no physical basis and does not consider flood
13 magnitude, although the output could be classified into relative probability bands using the
14 inundation frequency output (Figure 10a, d). This would mean that cells with a high
15 inundation frequency are more susceptible to smaller flood events, whereas cells with a lower
16 inundation frequency would be more susceptible to a higher magnitude flood. The MC-LCP
17 is based on a subjective cost weighting of elevation difference from the channel, local slope,
18 and allows for the inclusion of additional GIS layers such as land cover. However, that the
19 former two factors are applicable to any DEM means that the MC-LCP's outputs are
20 standardised between applications. The inclusion of a land cover cost using a reclassification
21 of the Manning's N values used in HEC-RAS produced a lower inundated area (Figure 7a),
22 owing to restricted divergence in the extensive forested reaches but increased divergence over
23 agricultural and barren land. When investigating the impacts of future land use change on
24 GLOF risk (e.g. Nussbaumer et al., 2014), the MC-LCP could therefore demonstrate how
25 evolving land cover scenarios would modify the GLOF flow path.

1 Though still only acting as a first-pass assessment, the MC-LCP demonstrates increased
2 utility at local scales where relative risks can be evaluated using the lateral inundation extent
3 and frequency. The MC-LCP model also demonstrated the greatest stability between each
4 DEM with a difference with an inundated area range of 0.85 km² (e.g. Figure 7a). Good
5 agreement between the MC-LCP and geomorphic evidence of the Dig Tsho GLOF event
6 lends further support to using this method to evaluate GLOF flow path propagation (Figure
7 6). Improved flow routing was a notable benefit in this study, where the linear artefacts
8 inherent in the MSF output were over 1 km long in places (e.g. Figure 10c).

9 6.3. Socio-economic implications

10 Extracting the relative socio-economic implications of each scenario facilitates an evaluation
11 of model choice and DEM sensitivities. Increasing flood magnitude HEC-RAS scenarios
12 correspond with greater damage potential. However, since each model represents flow using
13 a different technique, a greater flood extent does not necessarily equate to higher socio-
14 economic implications (Figure 7). The MC-LCP uses a least cost path approach, the MSF
15 requires the maintenance of flow direction, and HEC-RAS propagates open channel flow
16 using 1D St. Venant equations. Hence differing flow patterns were expected. The MSF
17 scenarios produced the greatest variation in this case, since the GDEM and SRTM scenarios
18 displayed an exaggerated extent in the lower reach in response to using a filled DEM. In
19 contrast, the MSF model appeared to under represent lateral divergence on the ALOS DEM
20 (e.g. Figure 8c).

21 6.4 Communication of risk and uncertainty

22 The GLOF workflow often contains large uncertainties at each linkage, including dam breach
23 formation and simulation (Osti and Egashira, 2009; Westoby et al., 2014); peak discharge and
24 lake volume estimation (Huggel et al., 2002; Fujita et al., 2013); and the flood propagation

1 itself (Westoby et al., 2014). In addition, compounding factors exist such as debris
2 entrainment, temporary damming, and the initiation of secondary landslides (Kuenza et al.,
3 2010). Increased vulnerability and continued habitation of hazardous zones exists where
4 communication and trust between local people, scientists, and policy makers is lacking
5 (Carey, 2005). Modelling efforts that contain an inherent communication of uncertainty can
6 therefore begin to bridge this gap at the expense only of increased computational processing
7 time. Risk maps should represent a range of scenarios and an indication of confidence in each
8 to avoid under or over representation. Although a transition towards coupling individual
9 process-based modelling efforts of the GLOF workflow is desirable (Worni et al., 2014), such
10 efforts are not feasible at regional-scales where first-pass assessments can provide an initial
11 indication of risk.

12 The MC-LCP incorporates an uncertainty assessment and displays least sensitivity to DEM
13 quality. Monte Carlo simulations implemented following a terrain realisation approach,
14 which was adopted in the MC-LCP, can be utilised to derive confidence maps using the
15 output of any hydrodynamic model and an estimate of DEM uncertainty. Alternatively, the
16 approach can also introduce a stochastic element into flood mapping scenarios where other
17 uncertainties exist in modelling the process chain. Retrospectively applying a DEM error
18 model to the output of a hydrodynamic model (e.g. Figure 8d) allows an assessment of
19 uncertainty within the modelled flood extent. However, this approach does not evaluate DEM
20 cells outside of the initial input boundary, in contrast to Figure 8e. Considering DEM
21 uncertainty when using hydrodynamic models such as HEC-RAS, requires DEM terrain
22 realisations to be input for each model iteration. Such coupling was beyond the scope of this
23 study but was undertaken manually for 10 iterations to provide a visual illustration and
24 comparison with the MC-LCP extent (Figure 8e). These ‘fuzzy’ outputs (e.g. Figure 8d, e)
25 deliver an enhanced decision support utility for subsequent inundation probability

1 interpretation, especially when using a lower quality dataset, or undertaking hazard zonation
2 mapping.

3 **7. Conclusions and further work**

4 The utility of the MSF model to model basic GLOF flow path propagation is confirmed for a
5 Himalayan study reach. However, the long reach length subdues any lateral interpretation of
6 inundation probability and the requirement to use a 'filled' DEM can create a high incidence
7 of parallel flow artefacts. These MSF artefacts were most apparent in the global DEM
8 products but they also appeared in a 15 m resolution ALOS PRISM DEM.

9 The new MC-LCP approach developed as part of this study displayed improved flow routing
10 compared to widely used MSF model, and displayed a stable flood extent, independent of the
11 DEM used. This independence of model performance to DEM product is likely to be
12 important in other confined and high relief Himalayan reaches, where the GDEM and SRTM
13 suffer with artefacts of poor channel delineation. Model scenarios using the SRTM produced
14 more consistent flood characteristics in all cases, in line with those scenarios using the finer
15 resolution ALOS DEM. This is likely to be further improved as 30 m SRTM data become
16 commonly used for first-pass assessments.

17 More widely, this study has shown that caution should be exercised in data poor regions
18 where remote sensing based first-pass assessments may guide hazard zonation strategies, or
19 where local scale scenario implications are sought, since the socio-economic implications of
20 contrasting flow models and DEMs can diverge notably. Nevertheless, we have shown that
21 the MC-LCP model is able to represent the lateral inundation of the most vulnerable terrain in
22 to a flood event, and that inundation extent is comparable to that predicted with use of a 1D
23 hydrodynamic model. The user-customisable error model in the MC-LCP facilitates
24 uncertainty assessments even if DEM error is low, since DEM noise perturbation could

1 represent other process chain uncertainties. The stochastic approach demonstrated here could
2 benefit from GIS applications of circuit theory when applied to multiple catchments
3 simultaneously. Circuit theory can similarly replicate least cost paths between two nodes over
4 a cost layer; however, owing to enhanced algorithm development, these techniques can
5 produce multiple pathway corridors with greatly reduced processing time (McRae et al.,
6 2008). An example is provided in the supplementary information (Figure S1).

7 Further development of the model could incorporate a downstream distance decay function to
8 represent flood attenuation. This decay function would be more informative than a simple
9 slope-dependant model cut-off for Himalayan reaches, where the potential flood travel
10 distance is extensive. The inherent incorporation and 'fuzzy boundary' communication of
11 uncertainty improves the utility of the MC-LCP when dealing with a hazard for which the
12 process chain contains large and propagating uncertainties. The optional inclusion of
13 additional cost layers such as land cover, offers increased analytical ability within the model
14 framework. In addition, a simple calibration between the modelled DEM cost layers and high
15 water marks of a past event could further improve the utility of this initial assessment
16 technique since the cost layer itself could give an indication of relative inundation extents,
17 without requiring model iterations.

18 In summary, we suggest that the key advantages of the MC-LCP approach are as follows:

- 19 • It produces a flood inundation extent which represents DEM uncertainty.
- 20 • Flood inundation frequency allows an assessment of relative risk at a local scale.
- 21 • It is least sensitive to DEM choice.
- 22 • Additional cost factors such as land cover can be incorporated.
- 23 • It has low data requirements and a quick setup time.

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1 **Tables:**

2 Table 1. Relative elevation differences between downstream river channel profiles from each
3 DEM.

DEM absolute mean difference and (standard deviation) (m)	GDEM OPRN	SRTM OPRN	ALOS FILLED	GDEM FILLED	SRTM FILLED
ALOS OPRN	17.4 (21.8)	13.1 (12.9)	7.5 (18.8)	24.7 (23.0)	18.4 (17.6)
GDEM OPRN	-	18.7 (18.1)	20.2 (25.0)	23.4 (20.5)	22.7 (28.7)
SRTM OPRN	-	-	12.4 (19.7)	18.3 (20.3)	5.6 (10.4)

4

5 Table 2. Comparison of terrain modification resulting from ‘fill’ and ‘OPRN’ DEM
6 processing.

DEM	Fill			OPRN		
	Modified cells (%)	Mean fill (m)	Max fill (m)	Modified cells (%)	Mean fill/ cut (m)	Max fill/ cut (m)
GDEM	4.9	21.2	119.0	1.34	3.7/ -9.4	41.5/ -81.6
SRTM	2.4	16.0	52.0	0.49	2.3/ -5.6	14.3/ -45.0
ALOS*	7.8	53.2	1386.8	2.55	17.4/ - 21.32	953.0/ -598.5

Reported statistics highlight the difference between DEM pre-processing algorithms for each study reach DEM, not exclusively the river channel environment. *ALOS statistics therefore include large artefacts of the DEM generation procedure where areas of cloud were present on adjacent valley slopes

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1 Table 3. Validation of the MC-LCP model against geomorphic evidence of the 1985 Dig
2 Tsho GLOF

Reach ID	GLOF reach area derived from geomorphic evidence (m ²)*	MC-LCP intersection with geomorphic evidence (m ²)**	GLOF extent accounted for by the MC-LCP (%)
L1	181942	132615	73
L2	115889	99265	86
L4	137546	105172	76
L5	23652	19667	83
L7	16872	14805	88
L8	49967	31162	62

* Mapped by Cenderelli and Wohl (2001)

**Shown in Figure 6

3

4 Table 4. Inundated extent and socio-economic implications for each model scenario.

Scenario	Inundated extent (km ²)	Inter-DEM extent range (km ²)	Building inundation	Road inundation (km)	Socio-economic range (buildings/road (km))
MC-LCP GDEM 500	9.68		107	6.59	
MC-LCP SRTM 500	10.46	0.85	118	9.03	34/ 2.45
MC-LCP ALOS 500	9.61		84	6.85	
MC-LCP GDEM LC 500*	6.97		138	5.94	
MC-LCP SRTM LC 500*	8.03	1.06	144	6.54	16/ 2.49
MC-LCP ALOS LC 500*	7.88		128	8.43	
MSF GDEM	7.81		246	16.91	
MSF SRTM	5.46	4.86	145	12.14	241/ 16.03
MSF ALOS	2.94		5	0.88	
HEC-RAS GDEM Pf1	6.35		53	4.05	
HEC-RAS GDEM Pf2	6.86	2.48**	56	4.46	31/ 0.72**
HEC-RAS GDEM Pf3	7.09		56	4.37	

HEC-RAS SRTM Pf1	5.29	36	4.25	1
HEC-RAS SRTM Pf2	5.65	37	4.35	2
HEC-RAS SRTM Pf3	5.77	37	4.65	3
HEC-RAS ALOS Pf1	3.86	11	3.20	4
HEC-RAS ALOS Pf2	4.18	20	3.56	5
HEC-RAS ALOS Pf3	4.38	25	3.93	6
* MC-LCP scenarios incorporating a land cover cost factor.				7
**Minimum inter-DEM range considering all HEC-RAS scenario profiles. Individual profile ranges are given below:				8
Pf1 range: 2.48 km ² , 42 buildings, & 1.06 km of road.				
Pf2 range: 2.68 km ² , 36 buildings, & 0.90 km of road.				9
Pf3 range: 2.71 km ² , 31 buildings, & 0.72 km of road.				
				10

Figure 1. Location of the Chamkar Chu basin study reach within Bhutan and the potentially hazardous lakes identified by Mool et al. (2001).

Figure 2. Study reach overview. (a) Channel elevation profiles for each Optimised Pit Removal Net (OPRN) processed and filled DEM. (b) Example cross sections from the HEC-RAS model demonstrating the difference between the OPRN and filled DEMs.

Figure 3. Study workflow outlining the model evaluation procedure for the hypothetical inundation scenarios.

Figure 4. Components and process of the MC-LCP model.

Figure 5. Evaluation of the MC-LCP inundated area stability with increasing number of model iterations

Figure 6. Validation of the MC-LCP model against the field-measured flood extent of the 1985 Dig Tsho GLOF using the ASTER GDEM v2. The six reaches are those numbered and reported by Cenderelli and Wohl (2001).

Figure 7. Inundated area graphs for (a) GIS-based models, and (b) HEC-RAS scenarios, where profiles 1-3 represent increasing magnitude flood scenarios.

Figure 8. (a-c) Example flood extent maps for the application of each model to respective DEMs in the lower reach. The medium magnitude scenario (Pf2) extents are shown for the HEC-RAS model. (d) Inundation confidence map within one HEC-RAS output produced by classifying positive flood depth during 500 stochastic terrain realisations on the GDEM. Note that this method does not consider inundation uncertainty outside of the initial input boundary. (e) Inundation frequency derived by manually inputting ten terrain realisations into HEC-RAS simulations run on the GDEM. This method allows DEM uncertainty to be considered during each HEC-RAS simulation. The MC-LCP extent for the GDEM is shown for comparison.

Figure 9. Wetted width and maximum depth for the MC-LCP (a), HEC-RAS Profile 2 (b), and MSF (c) models. Maximum MSF depth (c) is plotted on a different scale. (d) The inter-profile difference for each HEC-RAS DEM scenario.

Figure 10. Model output comparison for an upper reach (a-c) and lower reach featuring the town of Jakar (d-f).

Figure S1. Comparison of the MC-LCP and CIRCUITSCAPE outputs using the SRTM. The fuzzy outputs are classified to single values representing the maximum modelled flood extent. 20 m contour intervals are shown with 100 m index contours.

Figure 1

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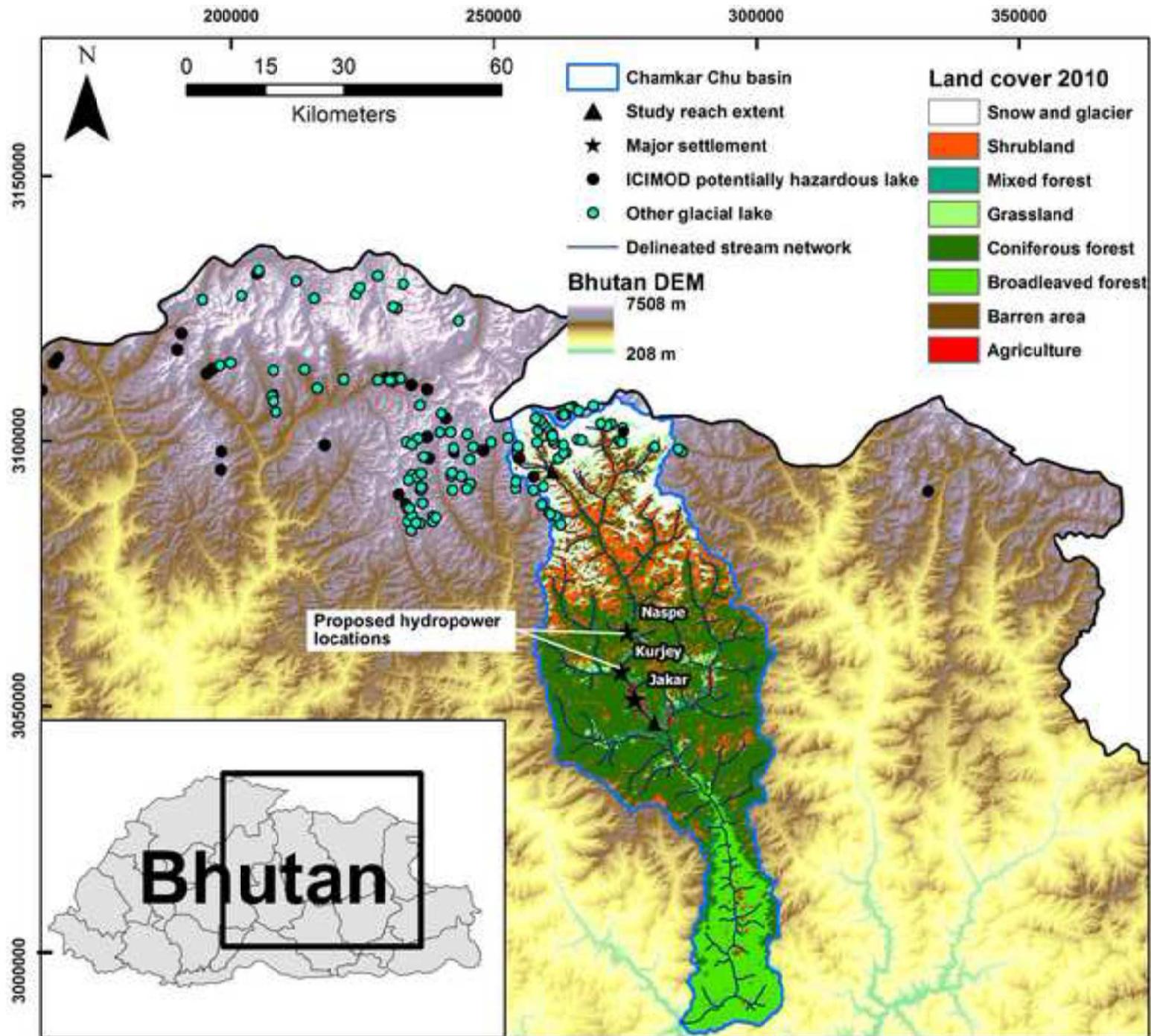


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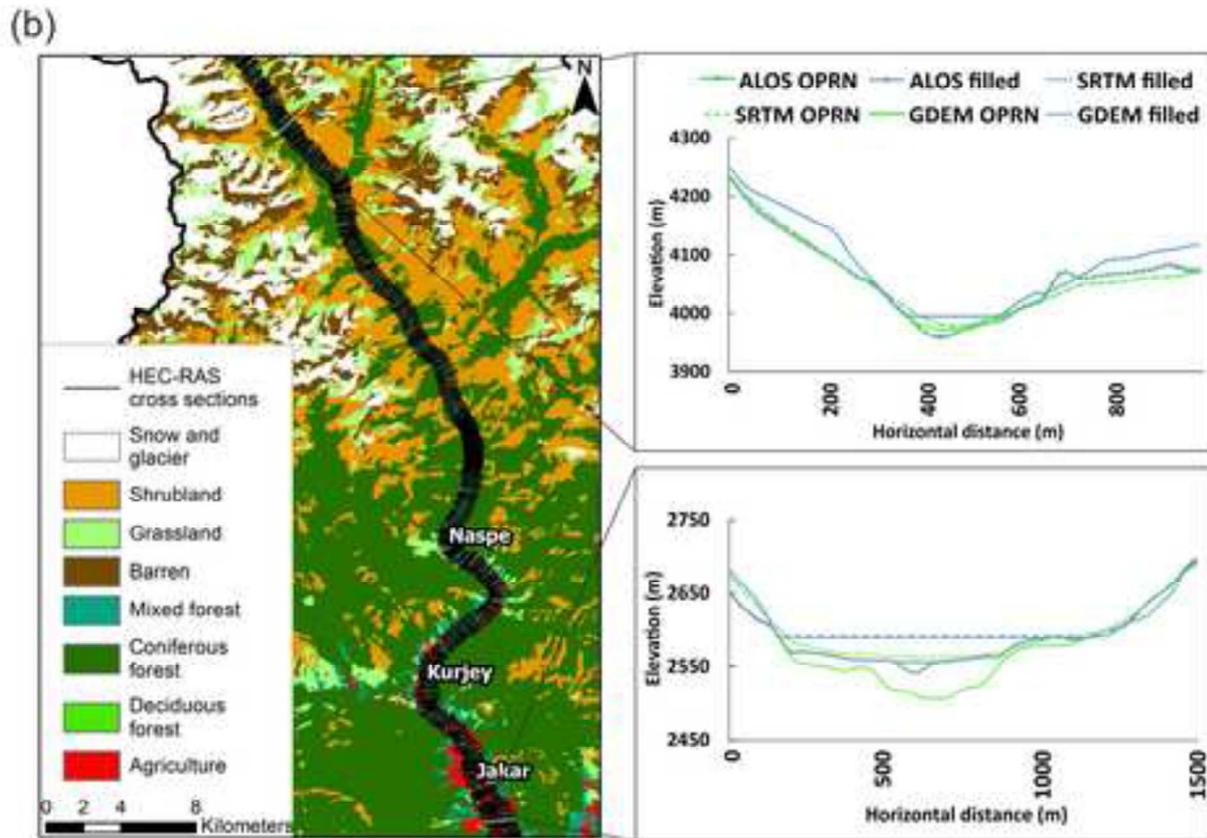
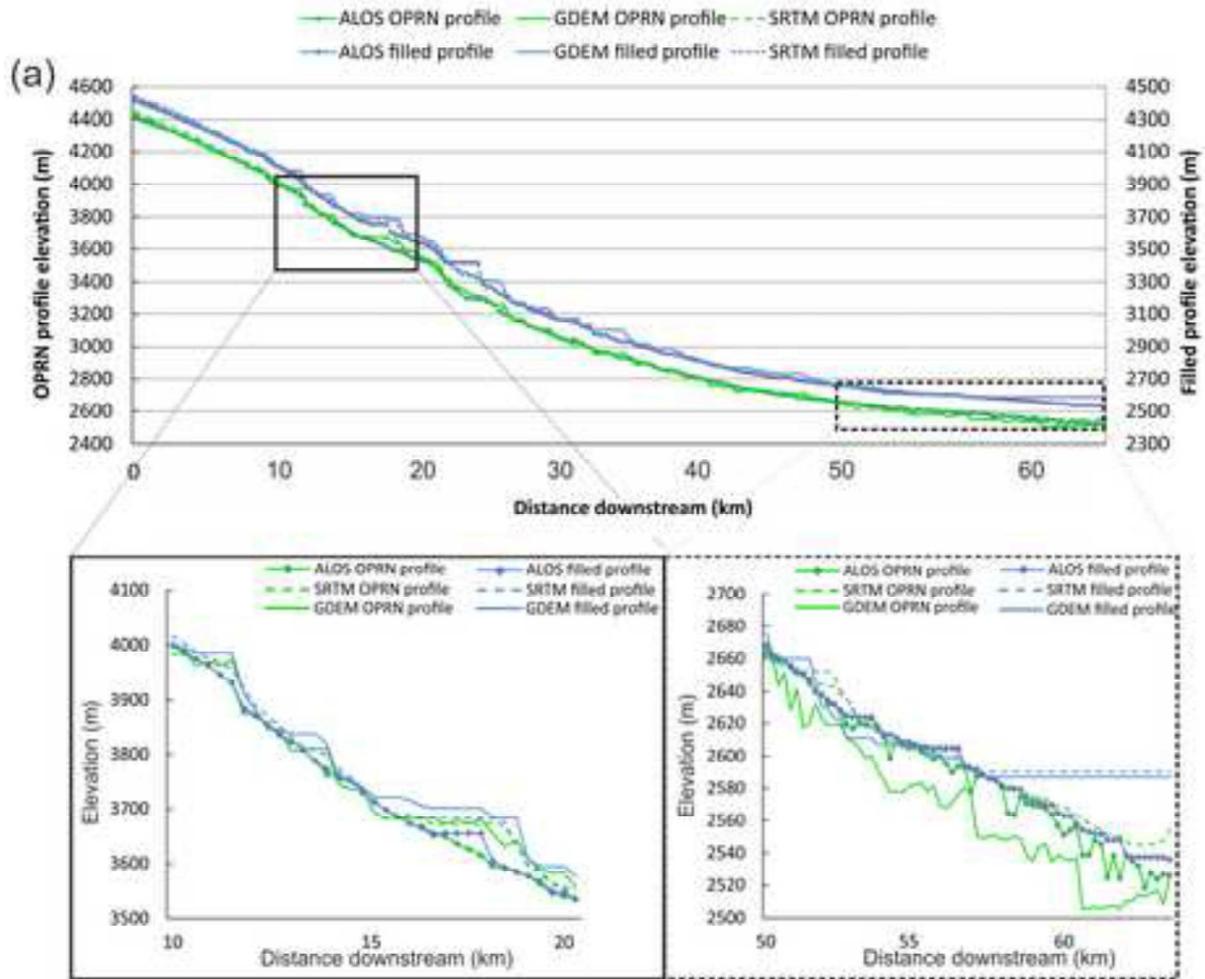


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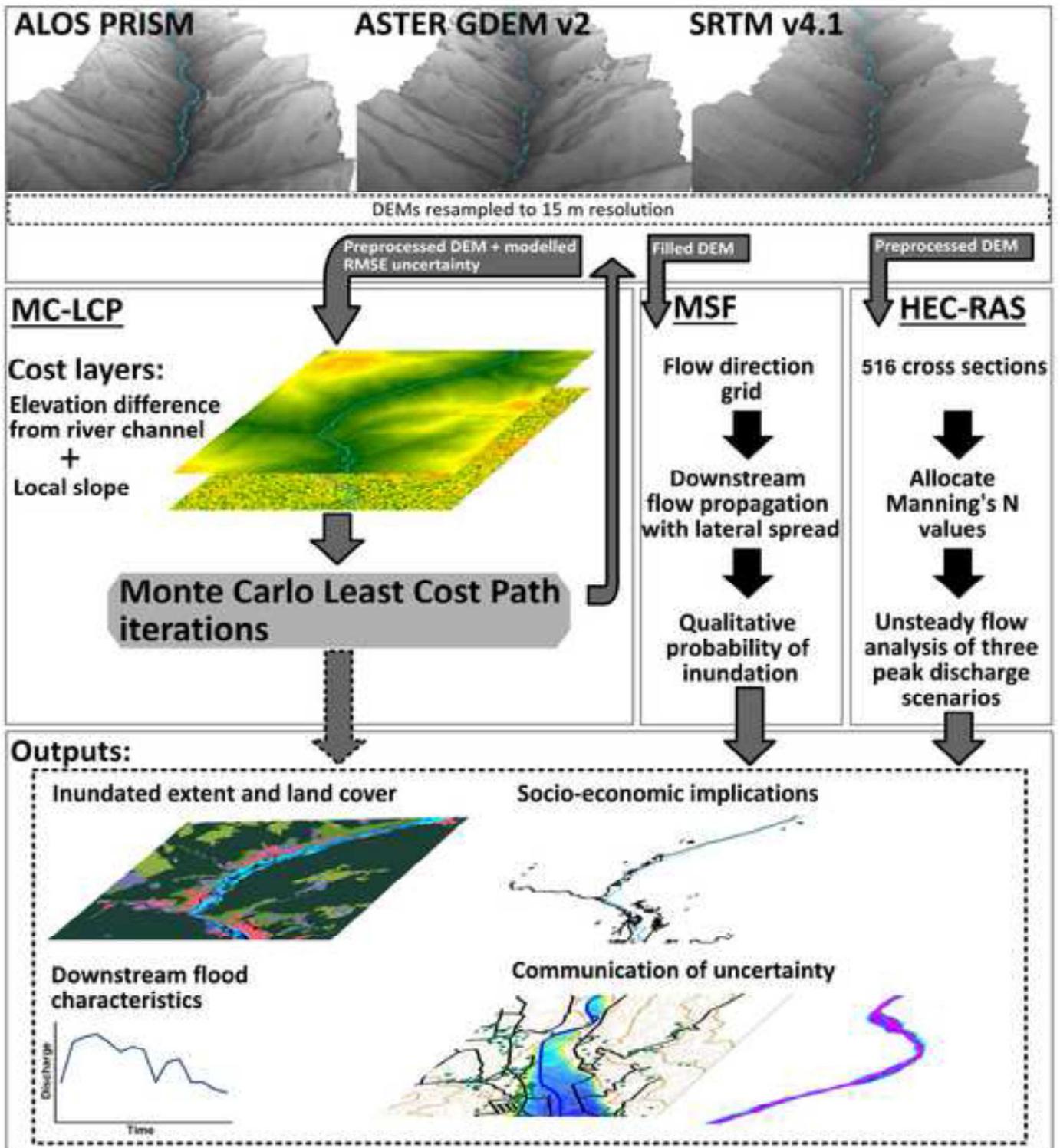


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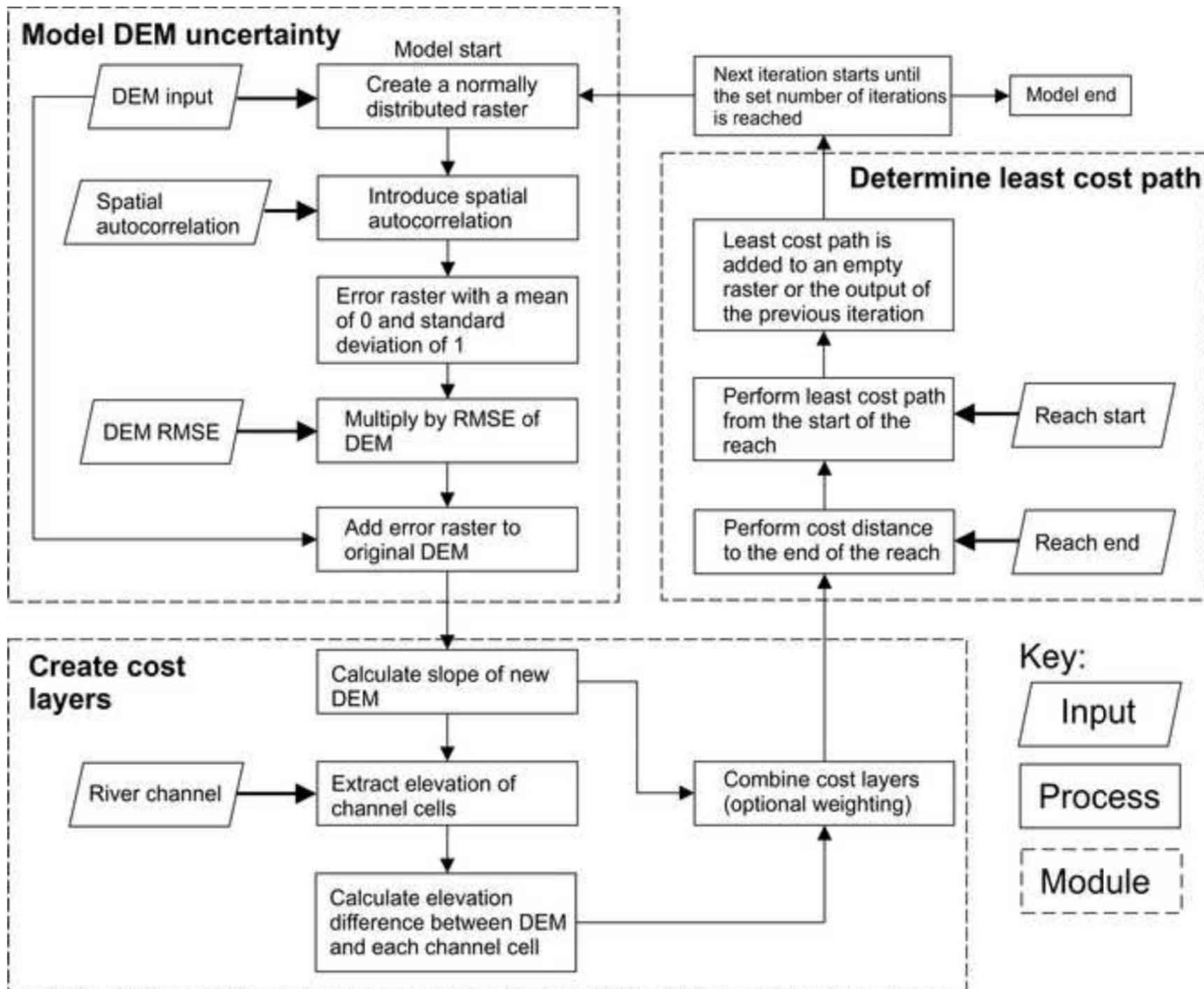


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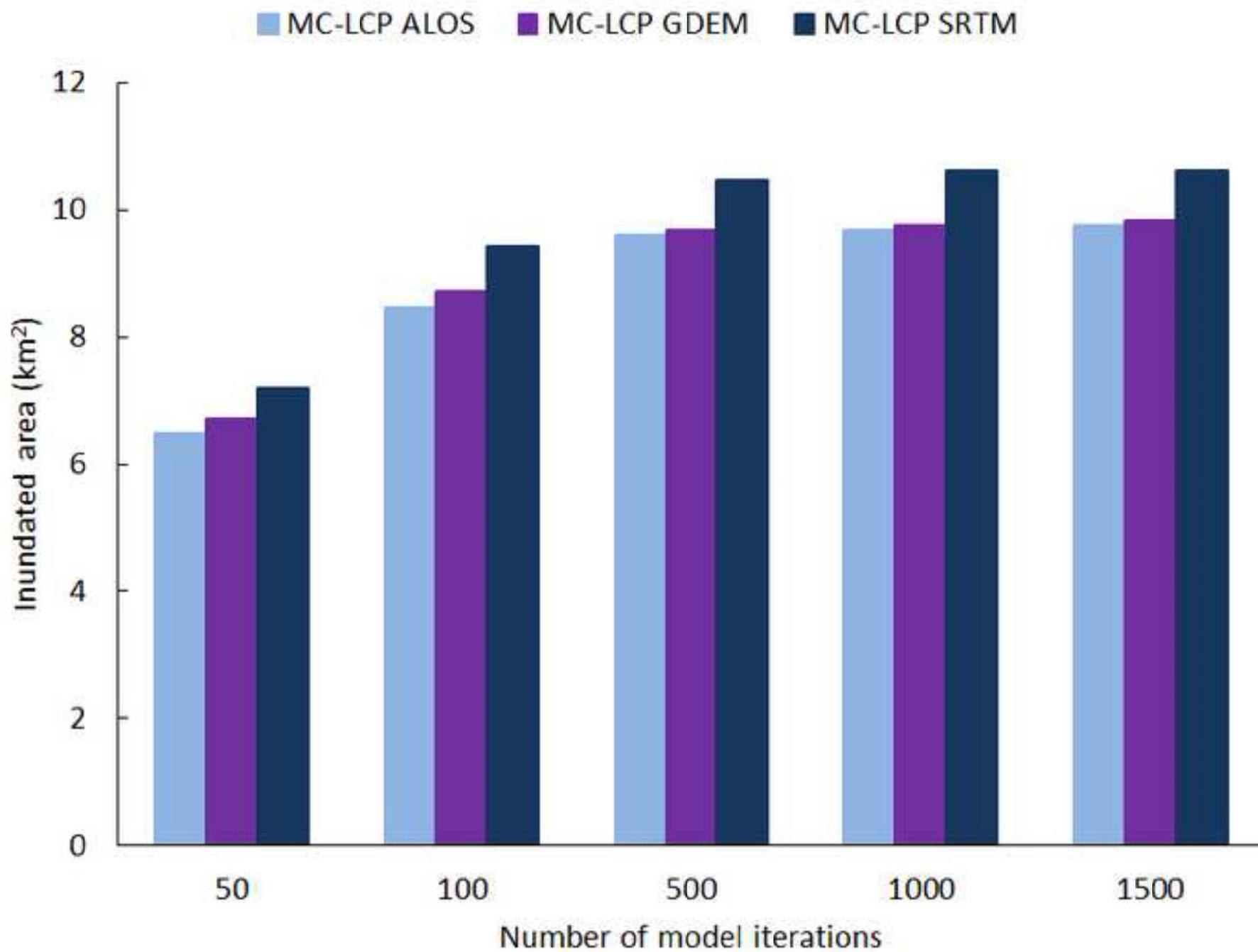


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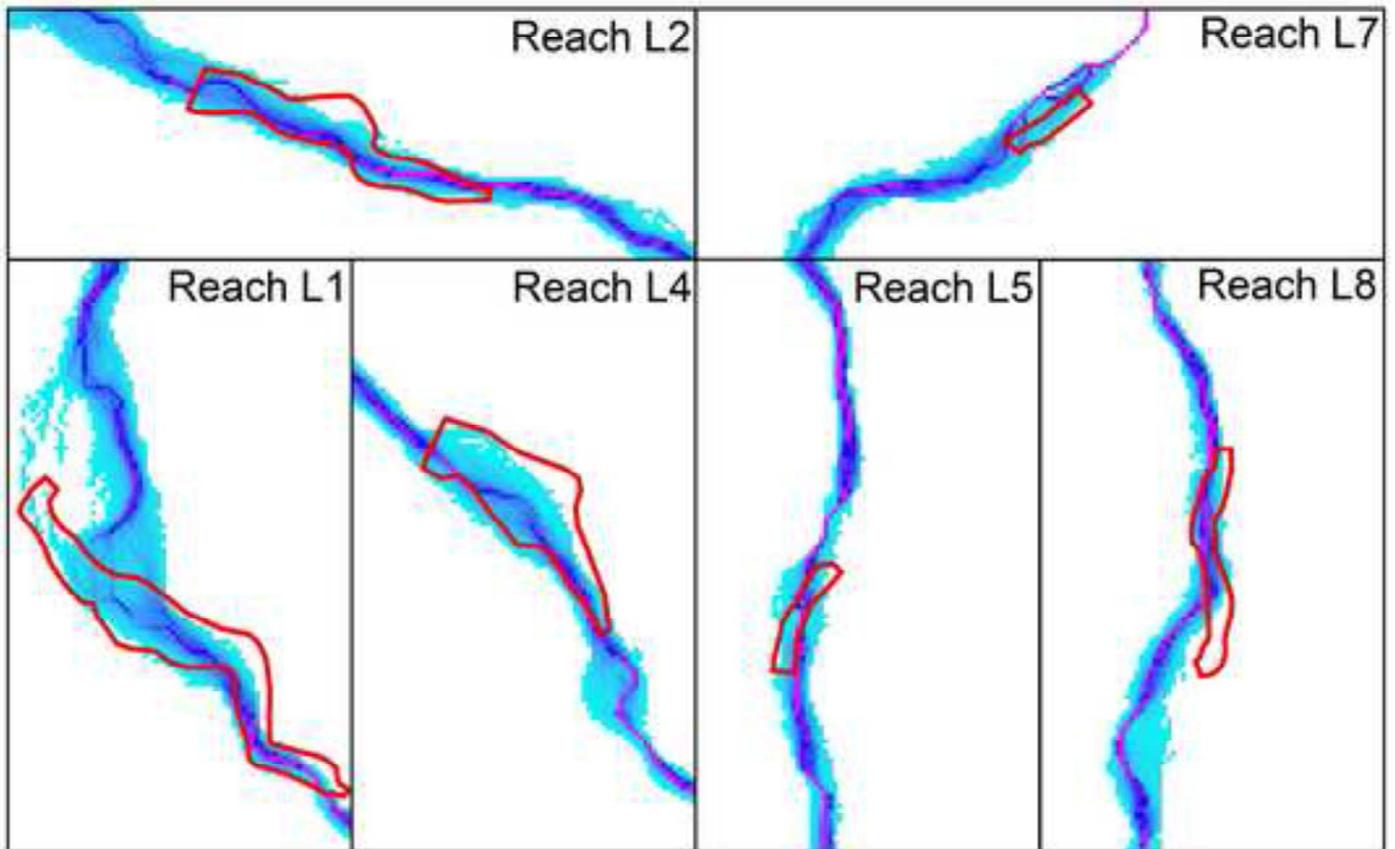
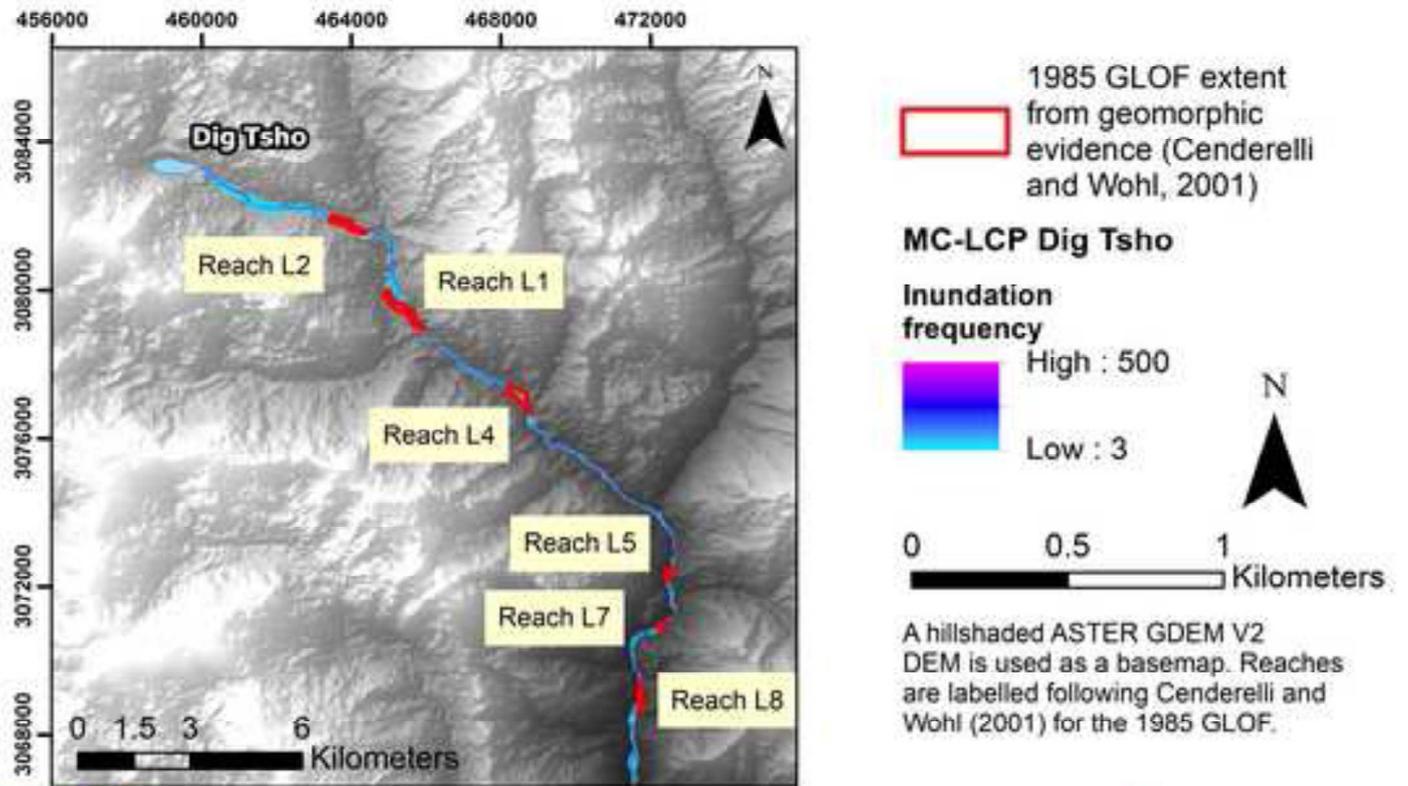


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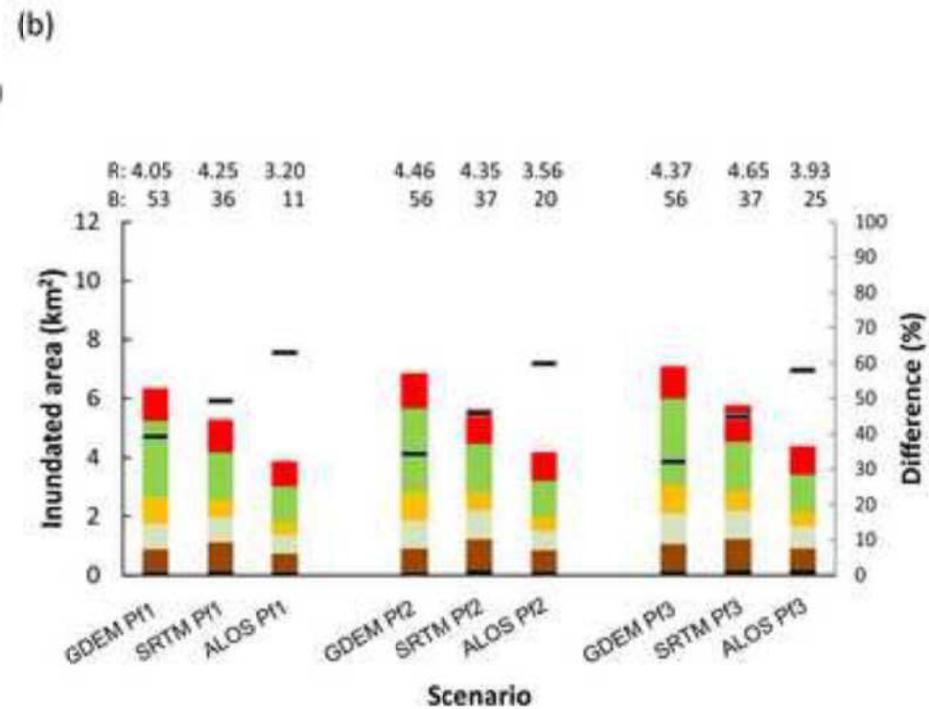
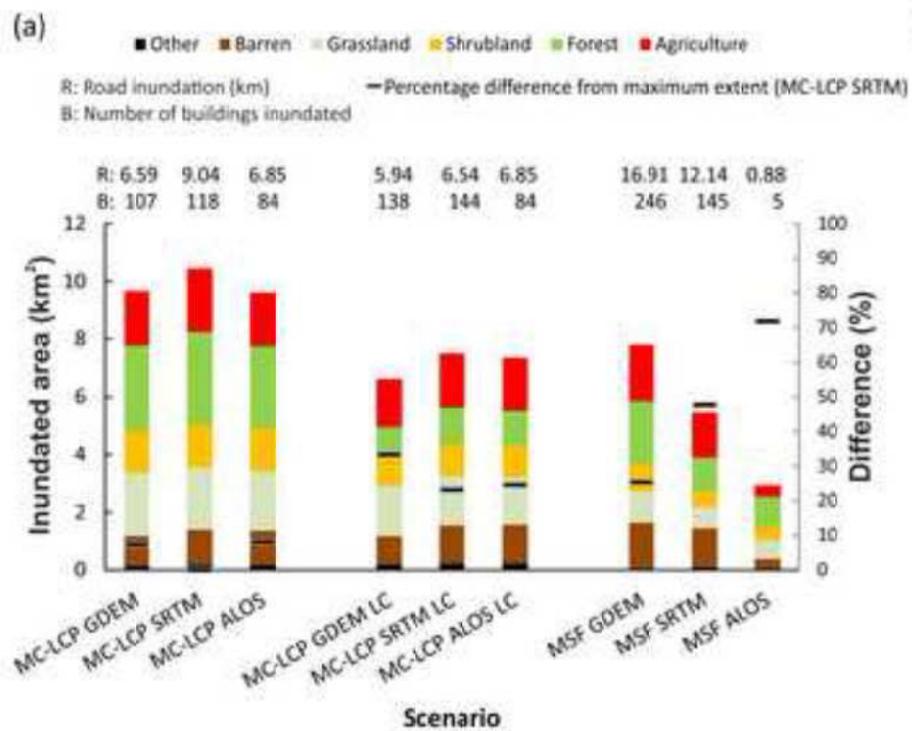


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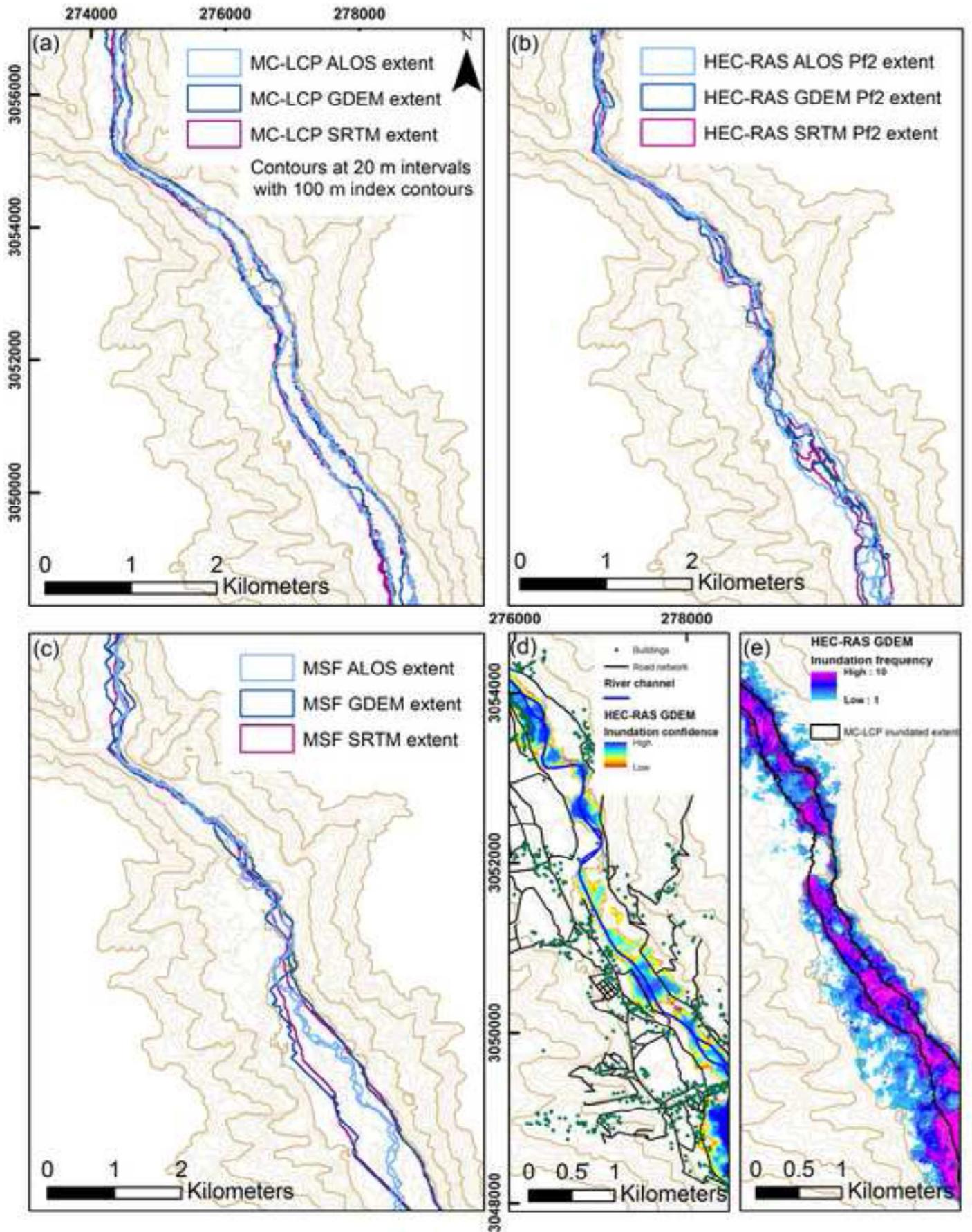


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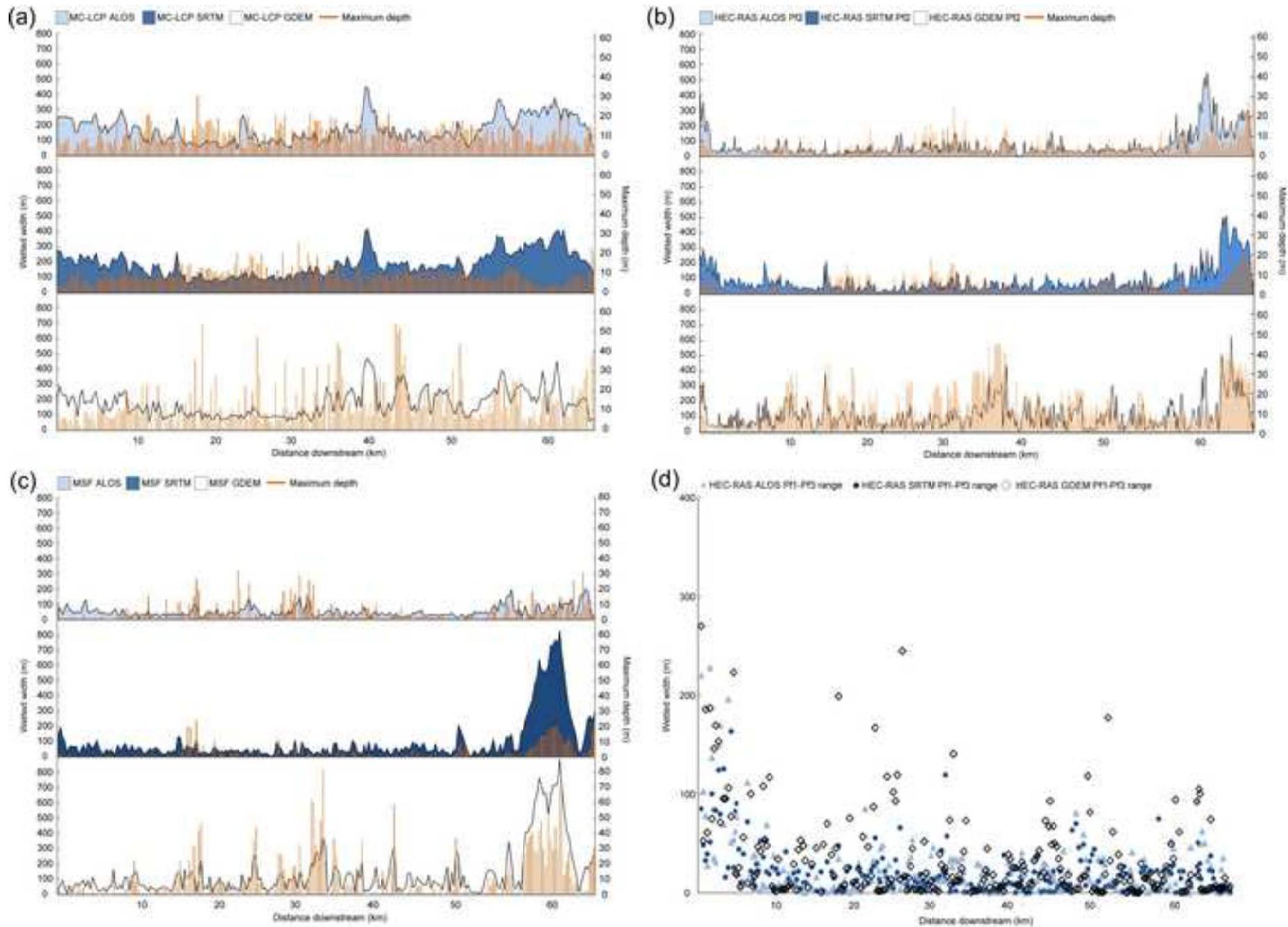
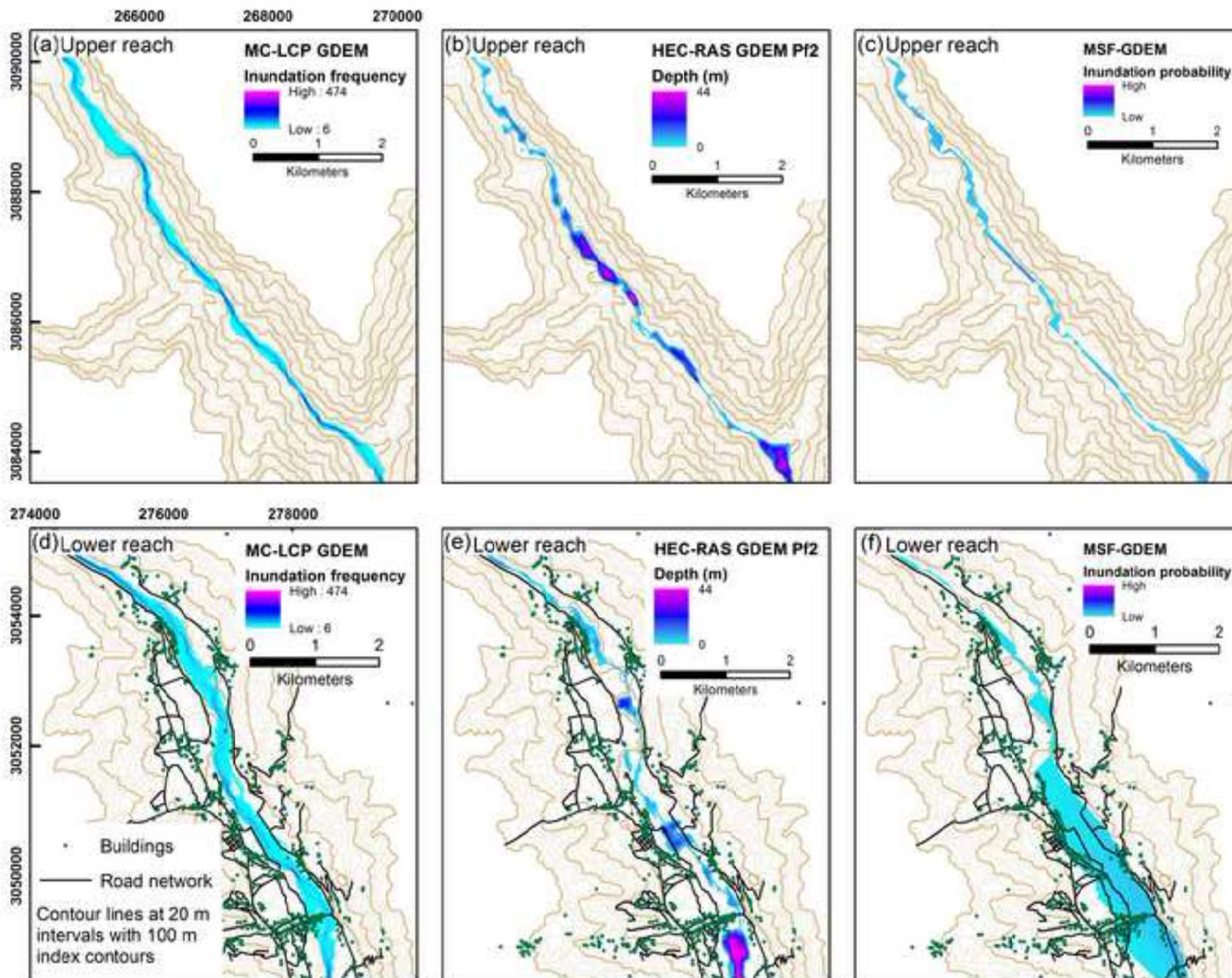


Figure 10

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Sup Figure 1

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