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Modelling real-time dynamic substructuring using partial delay differential equations

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Real-time dynamic substructuring is a new component testing method for simulating the dynamics of complex engineering systems. The physical component is tested in a computer generated 'virtual' environment using real time control techniques. Delays in communication which occur between the component and the virtual environment can potentially destabilise the simulation. In this paper the mechanism for this instability is studied using a beam-oscillator system as a case study. We will show how the stability and the amplitude response of the system change with the time delay. The numerical simulations of the reduced system as well as a full delayed beam equation are performed. A series of experimental tests is carried out on a beam-oscillator system. The comparison between the theoretical, numerical and experimental results is presented and these agree remarkably well.

Keywords: beam-oscillator system; real-time dynamic substructuring; delay equations; hybrid testing

1. Introduction

The main methods of testing the response of the structures under external loads and excitations (e.g. earthquake testing techniques) are shaking tables, pseudo-dynamic testing and real-time dynamic substructuring, see Williams & Blakeborough (2001) and Blakeborough *et al.* (2001). Every method has its advantages and limitations. In particular, shaking tables are usually expensive to build and operate, and the structures have to be scaled down. This reduces the understanding of the structural response as reducing the structures leads to a non-commensurate change of material properties of the original system. Pseudo-dynamic testing is not done in real time and this leads to a lengthy and costly experiments.

On the other hand, recent advances in analytical and numerical methods have led to further development of the real-time dynamic substructuring. The structure to be tested, or emulated, is first divided into two parts. One part is placed in the laboratory, and another one is modelled numerically. The parts are usually connected by electric or hydraulic actuators, which introduce the interface forces between computational and experimental parts. The actuators act as a transfer

system and are designed to follow appropriate output displacements calculated by the numerical model (Wallace *et al.* 2005; Kyrychko *et al.* 2006).

The main advantage of this technique is that experiments are not very expensive to build, run in real-time, and are repeatable. This technique also allows parametric variation of numerical model parameters and can be applied not only to test the structural response under an earthquake, but also to test parts of machinery, cars etc (Williams & Blakeborough 2001). Also, in this way the numerical, analytical and experimental techniques are tied together, and this helps to better understand the behaviour of the model under consideration. Potentially, real-time dynamic substructuring can be performed online in different places, even different countries.

The main challenge is to ensure that the substructured system behaves in the same way as the emulated system. The transfer systems typically introduce a time lag or delay into the system, and this effect must be accounted for while performing a real-time dynamic substructuring experiment. Recently it was suggested that the way to analytically model a dynamic substructuring experiment is to use delay differential equations (DDEs) (see, for instance, Wallace *et al.* 2005; Kyrychko *et al.* 2006). Thus the system is modelled more realistically, and therefore, shall lead to more reliable results. Unfortunately, time delay can lead to a complete destabilization of the system but artificial variation of the delay time may help one to change from unstable to stable regime. Because of the infinite-dimensionality of the DDEs, inclusion of time delays into the equation of motion makes analysis more difficult and challenging. The theory of the equations with time-delays is an active topic of research and some recent developments can be found in, for example, Laurent *et al.* (2006); Adimy *et al.* (2006); Hu & Wang (2002).

In this paper we introduce a system which consists of a clamped-free cantilever beam, with a mass-spring-damper attached to the free end of the beam. There are many applications of such system, including vibrations of elastic arms and their suppression, robotics, vibrations of vehicle on a compressed rail etc. An analytical and experimental investigation of beams carrying elastically mounted masses was performed by Ercoli and Laura (1987). Rossi *et al.* (1993) have found the exact solution of the free vibrations of Timoshenko beams carrying elastically mounted masses. Gürgöze has derived the frequency equation for the cantilever beam with attached tip mass and a spring-mass (Gürgöze 1996) and studied the sensitivity of the eigenvalues of a viscously damped cantilever beam carrying a tip mass (Gürgöze 1998). The question of controlling hybrid experiments in which only some of the state variables are accessible for measurements, has been investigated by Sieber and Krauskopf (2006). Our stability findings will provide a background for implementing this control technique in a real experiment.

We propose to implement real-time dynamic substructuring in this system by modelling a mass-spring-damper connected via a transfer system to the beam placed in the laboratory, with the force being generated by an electrically driven actuator. Mathematically, the system will be modelled using partial delay differential equations (PDDEs). We believe that this is the first ever attempt to use PDDEs to represent a real-time dynamic substructuring experiment. It should be noted that the theory of PDDEs with applications to engineering systems is at its early stage in general, and neutral PDDEs in particular. The qualitative theory of partial functional differential equations with emphasis on reaction-diffusion equations with delay can be found in Wu (1996).

After reducing the system to the finite mode truncation of the beam, we analyse the dynamics of the model using the method of multiple scales, and derive the amplitude equation for the resonant case. The numerical simulations of the finite mode truncation of the delayed beam equation are performed and subsequently compared to the experimental results. We present the experimental response of the beam to the excitation with different time delays, which show periodic and quasi-periodic behaviour. Also, experimental and theoretical amplitude response diagrams are compared.

Section 2 introduces the equation of motion of the beam-mass-spring-damper system. In Section 3 the multiple scales method is used to derive the amplitude response relation. Section 4 is devoted to the stability analysis of the neutral delay differential equation and stability regions in the parameter plane are identified. The numerical simulations of the delayed system are presented in Section 5. In Section 6 the experimental results are given and compared to analytical and numerical findings. The paper concludes with the summary in Section 7.

2. Analytical formulation of the problem

The system under investigation is a cantilever steel beam clamped at one end and free at the other with a mass-spring-damper (MSD) attached to the free end of the beam. The equations of motion of the coupled system can be written as

$$\begin{aligned} & \left(M \frac{\partial^2 u}{\partial t^2}(x, t) + C \frac{\partial u}{\partial t}(x, t) + K u(x, t) \right) \delta(x - L) \\ & + m \frac{\partial^2 u}{\partial t^2}(x, t) + EI \frac{\partial^4 u}{\partial x^4}(x, t) = A \sin(\omega t) \delta(x - L), \end{aligned} \quad (2.1)$$

where x is the coordinate along the beam and t is the time, L denotes the undeformed length of the beam and m is the mass. Furthermore, δ is the Dirac delta function, EI is constant, M , C , K are mass, damping and stiffness coefficients of the oscillator. A mass-spring-damper is attached to the free end of the beam. An external force is acting on the MSD with the amplitude A and frequency ω which in turn excites the beam. The emulated, or real, system is divided and the MSD is taken to be the numerical model while the beam is constructed in the laboratory. The two parts are connected via an electrical actuator. This means that there is a delay between the time a signal is sent and the moment the displacement of the beam is received back. The schematic description of the model is presented in figure 1. Therefore, the modified equation of motion has the form:

$$\begin{aligned} & \left(M \frac{\partial^2 u}{\partial t^2}(x, t) + C \frac{\partial u}{\partial t}(x, t) + K u(x, t) \right) \delta(x - L) \\ & + m \frac{\partial^2 u}{\partial t^2}(x, t - \tau) + EI \frac{\partial^4 u}{\partial x^4}(x, t - \tau) = A \sin(\omega t) \delta(x - L), \end{aligned} \quad (2.2)$$

where τ is the time delay (assumed to be constant).

To simplify the equation (2.2), one can introduce the following non-dimensional quantities:

$$\tilde{t} = \omega_0 t, \quad \tilde{x} = \frac{x}{L}, \quad \tilde{\tau} = \omega_0 \tau, \quad \omega_0^2 = \frac{EI}{mL^4}.$$

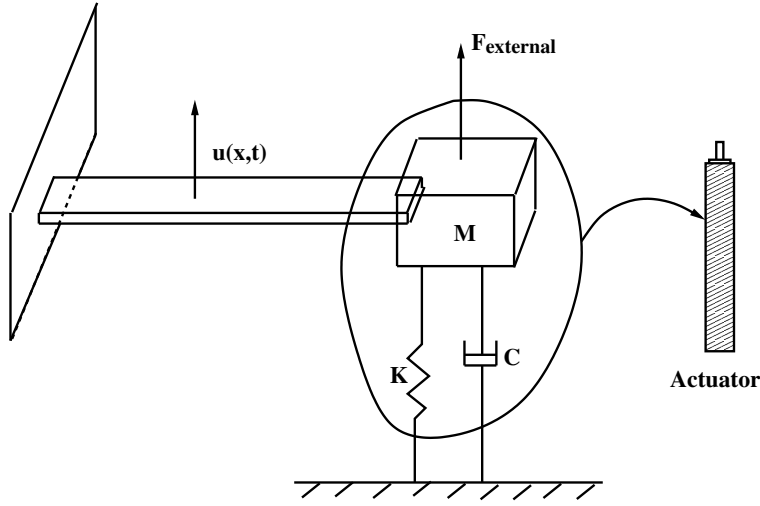


Figure 1. Schematic description of the coupled beam-MSD system.

Under this rescaling and omitting tildes equation (2.2) can be rewritten in the form

$$\begin{aligned} & \left(\frac{\partial^2 u}{\partial t^2}(x, t) + 2\zeta \frac{\partial u}{\partial t}(x, t) + \kappa^2 u(x, t) \right) \delta(x - 1) \\ & + \mu \frac{\partial^2 u}{\partial t^2}(x, t - \tau) + \mu \frac{\partial^4 u}{\partial x^4}(x, t - \tau) = \alpha \sin(\omega t) \delta(x - 1), \end{aligned} \quad (2.3)$$

where

$$\zeta = \frac{C}{2M\omega_0}, \quad \kappa = \sqrt{\frac{K}{M\omega_0^2}}, \quad \mu = \frac{m}{M} \quad \text{and} \quad \alpha = \frac{A}{M\omega_0^2}.$$

An approximate series solution of equation (2.3) can be taken to be in the form (Gürgöze 1998):

$$u(x, t) = \sum_{m=1}^n u_m(x) q_m(t),$$

where $u_m(x)$ are the orthogonal eigenfunctions of the clamped-free beam without an MSD, normalised with respect to the mass density. Functions $q_m(t)$ represent the unknown time dependent generalised coordinates. Upon substituting the expansion of the solution $u(x, t)$ into the equation (2.3), multiplying both sides by the k -th eigenfunction $u_k(x)$ and integrating over the rescaled beam length, we obtain the set of delay differential equations

$$\begin{aligned} & u_k(1) \sum_{m=1}^n u_m(1) \ddot{q}_m(t) + 2\zeta u_k(1) \sum_{m=1}^n u_m(1) \dot{q}_m(t) + \kappa^2 u_k(1) \sum_{m=1}^n u_m(1) q_m(t) \\ & + \mu \dot{q}_k(t - \tau) + \mu \beta_k^4 q_k(t - \tau) = \alpha u_k(1) \sin \omega t, \quad k = 1, \dots, n, \end{aligned} \quad (2.4)$$

where the eigenvalues β_i are solutions of the transcendental equation

$$\cosh \beta_i \cos \beta_i + 1 = 0, \quad i = 1, 2, \dots,$$

with $\beta_1 \approx 1.8751$.

Assuming that the dynamics of the beam is well represented by that of its first mode, we can reduce the system (2.4) to one second order neutral delay differential equation (as it involves time-delayed highest derivative):

$$u_1^2(1)\ddot{q}_1(t) + 2\zeta u_1^2(1)\dot{q}_1(t) + \kappa^2 u_1^2(1)q_1(t) + \mu\ddot{q}_1(t-\tau) + \mu\beta_1^4 q_1(t-\tau) = \alpha u_k(1) \sin \omega t, \quad (2.5)$$

where

$$u_1(x) = \cosh \beta_1 x - \cos \beta_1 x - \gamma_1 (\sinh \beta_1 x - \sin \beta_1 x), \quad \gamma_1 = \frac{\cosh \beta_1 + \cos \beta_1}{\sinh \beta_1 + \sin \beta_1}.$$

We can further recast the equation (2.5) as

$$\ddot{q}_1(t) + 2\zeta\dot{q}_1(t) + \kappa^2 q_1(t) + \mu^* \ddot{q}_1(t-\tau) + v q_1(t-\tau) = \alpha^* \sin \omega t, \quad (2.6)$$

where

$$\mu^* = \frac{\mu}{u_1^2(1)}, \quad v = \mu^* \beta_1^4, \quad \text{and} \quad \alpha^* = \frac{\alpha}{u_1(1)}.$$

Equation (2.6) is the equation whose properties will be investigated in this paper.

3. Perturbation analysis

In order to analyse the primary resonance of the neutral delay equation (2.6) using the method of multiple scales (Nayfeh & Pai 2004), we confine the study to the case of small damping, weak feedback, and soft excitation. This means that we assume the following:

$$\begin{aligned} \zeta &= \mathcal{O}(\varepsilon), \quad \mu^* = \mathcal{O}(\varepsilon), \quad \alpha^* = \mathcal{O}(\varepsilon), \\ v &= \mathcal{O}(\varepsilon), \quad \omega - \kappa = \varepsilon\sigma \quad \text{and} \quad \sigma = \mathcal{O}(1), \end{aligned}$$

where ε is small and σ is a detuning parameter. We look for a solution as a two scale expansion:

$$q(t) = \nu_0(T_0, T_1) + \varepsilon \nu_1(T_0, T_1) + \mathcal{O}(\varepsilon^2), \quad (3.1)$$

where $T_0 = t$ and $T_1 = \varepsilon t$. We use the following differential operators:

$$\frac{d}{dt} = \frac{\partial}{\partial T_0} + \varepsilon \frac{\partial}{\partial T_1} \equiv D_0 + \varepsilon D_1 + \mathcal{O}(\varepsilon^2), \quad (3.2)$$

$$\frac{d^2}{dt^2} = D_0^2 + 2\varepsilon D_0 D_1 + \mathcal{O}(\varepsilon^2).$$

Substituting expansion (3.1) into (2.6) and using the differential operators (3.2), gives (by equating powers of ε) a set of linear partial delay differential equations. The zeroth and first order approximations are

$$D_0^2 \nu_0(T_0, T_1) + \kappa^2 \nu_0(T_0, T_1) = 0, \quad (3.3)$$

and

$$\begin{aligned} D_0^2 \nu_1(T_0, T_1) + \kappa^2 \nu_1(T_0, T_1) &= -2D_0 D_1 \nu_0(T_0, T_1) - 2\zeta D_0 \nu_0(T_0, T_1) \\ -\mu^* D_0^2 \nu_0(T_0 - \tau, T_1) - v \nu_0(T_0 - \tau, T_1) &+ \alpha^* \sin(\kappa T_0 + \sigma T_1). \end{aligned} \quad (3.4)$$

Solving equation (3.3), we have

$$\nu_0(T_0, T_1) = A(T_1)e^{i\kappa T_0} + \text{c.c.}, \quad (3.5)$$

where c.c. stands for complex conjugate, and

$$A(T_1) = \frac{1}{2}\alpha(T_1)e^{i\kappa\beta(T_1)}. \quad (3.6)$$

Using ν_0 from (3.5) in (3.4) gives

$$\begin{aligned} D_0^2\nu_1(T_0, T_1) + \kappa^2\nu_1(T_0, T_1) &= -2i\kappa D_1 A e^{i\kappa T_0} - 2\zeta\kappa i A e^{i\kappa T_0} - v A e^{i\kappa T_0} e^{-i\tau\kappa} \\ &+ \kappa^2 \mu^* A e^{-i\tau\kappa} e^{i\kappa T_0} + \frac{\alpha^*}{2} e^{i\sigma T_1} e^{i\kappa T_0} + \text{c.c.} \end{aligned} \quad (3.7)$$

To eliminate the secular terms in the equation (3.7), we set

$$-2i\kappa(D_1 + \zeta)A + \kappa^2 \mu^* A(T_1)e^{-i\tau\kappa} - v A e^{-i\tau\kappa} + \frac{\alpha^*}{2} e^{i\sigma T_1} = 0.$$

Substituting (3.6) into the last expression and separating into the real and imaginary parts gives

$$\begin{aligned} D_1\alpha &= -\frac{1}{2\kappa} [\mu^* \kappa^2 \alpha \sin \kappa\tau - \alpha^* \sin(\sigma T_1 - \beta\kappa) - v\alpha \sin \kappa\tau + 2\zeta\kappa\alpha], \\ \alpha D_1\beta &= -\frac{1}{2\kappa^2} [\mu^* \alpha \kappa^2 \cos \kappa\tau - v\alpha \cos \kappa\tau + \alpha^* \cos(\sigma T_1 - \beta\kappa)]. \end{aligned}$$

Moreover, if we introduce a phase φ such that $\varphi = \sigma T_1 - \beta(T_1)\kappa$, then the equations governing the amplitude $\alpha(T_1)$ and the phase $\varphi(T_1)$ take the form:

$$\begin{aligned} D_1\alpha &= \frac{1}{2\kappa} (v \sin \kappa\tau - \mu^* \kappa^2 \sin \kappa\tau - 2\zeta\kappa) \alpha + \frac{\alpha^*}{2\kappa} \sin \varphi, \\ \alpha D_1\varphi &= \frac{1}{2\kappa^2} (\sigma\kappa^2 + v \cos \kappa\tau - \mu^* \kappa^2 \cos \kappa\tau) \alpha - \frac{\alpha^*}{2\kappa^2} \cos \varphi. \end{aligned} \quad (3.8)$$

Obviously, the presence of time delay τ in the original equation modifies the averaged equations by adding additional terms.

Steady state solutions of equation (2.6) for the primary resonance correspond to the fixed points of equations (3.8), and these can be obtained by setting $D_1\alpha = 0$ and $D_1\varphi = 0$. This leads to a set of algebraic equations (assuming $\alpha \neq 0$):

$$(v \sin \kappa\tau - \mu^* \kappa^2 \sin \kappa\tau - 2\zeta\kappa) \alpha + \alpha^* \sin \varphi = 0$$

$$(\sigma\kappa^2 + v \cos \kappa\tau - \mu^* \kappa^2 \cos \kappa\tau) \alpha - \alpha^* \cos \varphi = 0.$$

From the last system of equations we can derive the frequency response relations between the amplitude α and σ and the phase φ and σ :

$$\begin{aligned} \alpha &= \frac{\alpha^*}{\left[(v \sin \kappa\tau - \mu^* \kappa^2 \sin \kappa\tau - 2\zeta\kappa)^2 + (\sigma\kappa^2 + v \cos \kappa\tau - \mu^* \kappa^2 \cos \kappa\tau)^2 \right]^{1/2}} \\ \tan \varphi &= -\frac{(v \sin \kappa\tau - \mu^* \kappa^2 \sin \kappa\tau - 2\zeta\kappa)}{(\sigma\kappa^2 + v \cos \kappa\tau - \mu^* \kappa^2 \cos \kappa\tau)}. \end{aligned} \quad (3.9)$$

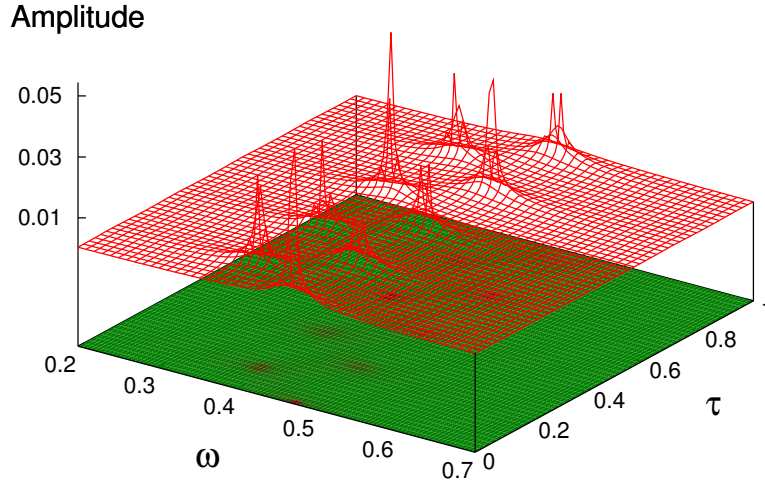


Figure 2. a) Amplitude response as a function of time delay and frequency of the perturbation.

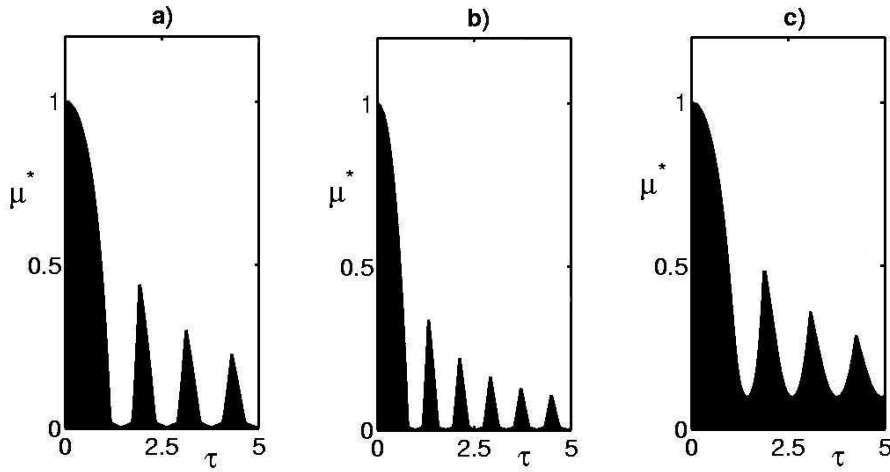


Figure 3. Stability boundary: a) $\kappa = 5.4, \zeta = 0.003$; b) $\kappa = 8, \zeta = 0.003$; c) $\kappa = 5.4, \zeta = 0.153$. Coloured region is a stable region, area above it is an unstable region.

These expressions will be used later when comparing analytical and experimental amplitude responses. The extreme values of the amplitude are

$$\alpha_{\text{extr}} = \frac{\alpha^*}{v - \mu^* \kappa^2 \pm \kappa \sqrt{\sigma^2 \kappa^2 + 4\zeta^2}}, \quad (3.10)$$

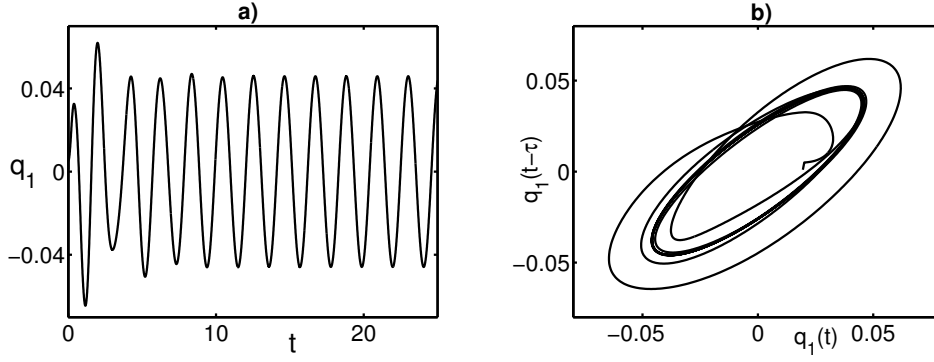


Figure 4. Solution $q_1(t)$ of equation (2.6) with delay time $\tau = 0.2$. a) Temporal dynamics. b) Phase portrait in the plane $(q_1(t), q_1(t - \tau))$.

and they are attained at

$$\tau_m^{\text{extr}} = \arctan\left(-\frac{2\zeta}{\sigma\kappa}\right) + m\pi, \quad m = 1, 2, \dots$$

Figure 2 illustrates amplitude response as given by the expression (3.9) for different delay times. One can observe that there are two nearby perturbation frequencies which exert the largest response of the system. In the vicinity of those two frequencies which correspond to the absolute maxima of the response, the system also has peaks in the amplitude response whose position depends on the perturbation frequency and time delay as given by expression (3.10).

4. Stability analysis

Returning to the equation (2.6) the characteristic equation for the trivial solution has the form

$$\lambda^2 + 2\zeta\lambda + \kappa^2 + \mu^*\lambda^2e^{-\lambda\tau} + ve^{-\lambda\tau} = 0.$$

Purely imaginary eigenvalues occur when $\lambda = \pm i\psi$, $\psi \neq 0$ with

$$-\psi^2 + 2i\zeta\psi + \kappa^2 - \mu^*\psi^2e^{-i\psi\tau} + ve^{-i\psi\tau} = 0.$$

Separating the last equation into the real and imaginary parts gives

$$\psi_{\pm} = \frac{1}{1 - \mu^{*2}} \left[(2\zeta^2 - \kappa + \mu^*v) \pm \sqrt{(2\zeta^2 - \kappa + \mu^*v)^2 - (\kappa^4 - v^2)(1 - \mu^{*2})} \right], \quad (4.1)$$

and

$$\tau = \frac{1}{\psi_{\pm}} \left[\arccos \frac{\psi_{\pm}^2 - \kappa^2}{v - \mu^*\psi_{\pm}^2} \pm 2\pi n \right], \quad n = 1, 2, \dots$$

Figure 3 shows stability boundary in the parameter plane of the time delay τ and the mass ratio μ^* for different values of κ and ζ . The values of μ^* (i.e. the mass ratio of mass of the beam to the mass of the MSD) do not exceed 1 as for $|\mu^*| > 1$ the trivial steady state is unstable for any delay time (Kyrychko *et al.*

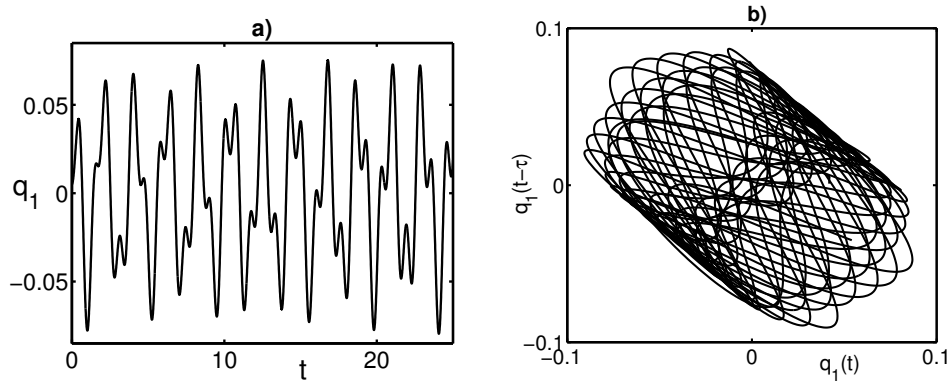


Figure 5. Solution $q_1(t)$ of equation (2.6) with delay time $\tau = 0.9$. a) Temporal dynamics. b) Phase portrait in the plane $(q_1(t), q_1(t - \tau))$.

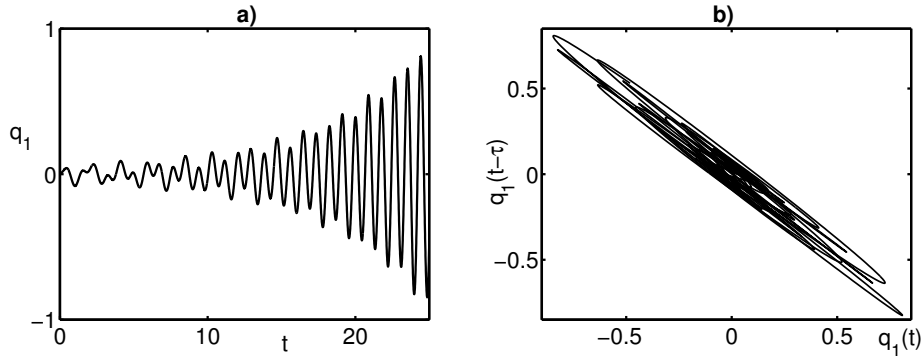


Figure 6. Solution $q_1(t)$ of equation (2.6) with delay time $\tau = 1.2$. a) Temporal dynamics. b) Phase portrait in the plane $(q_1(t), q_1(t - \tau))$.

2006). One can notice that the increase of the spring stiffness leads to the shift of the stability boundary to the left, and also to a slight decrease in the critical value of μ^* . Stable region becomes smaller and narrower. On the other hand, when the damping coefficient increases, the horizontal position of the stability boundary remains unchanged, while both the peaks and the bottoms on the curve grow. These changes in the stability boundary provide important information needed for the design of a stable experiment. Moreover, they indicate how varying the values of parameters of the numerical model can stabilize the system.

For a detailed description of stability switches and extensive stability study of neutral delay differential equations the reader is referred to Kyrchko *et al.* (2006).

5. Numerical simulations

This section is devoted to the numerical simulations of the neutral delay equation (2.6) and then the system (2.4). The equation (2.6) was discretized with an explicit finite difference scheme; to improve numerical stability of the scheme the damping term was approximated by central differences. In all simulations the values of the damping coefficient ζ , rescaled stiffness of the oscillator κ and the rescaled ratio

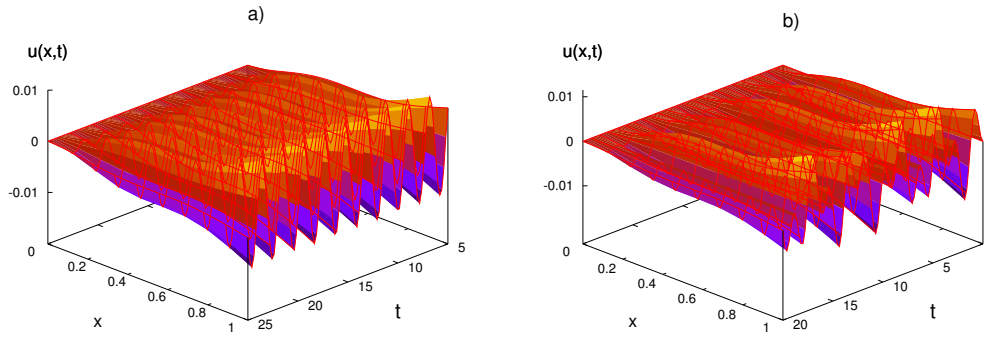


Figure 7. Spatio-temporal dynamics of a three-mode truncation of the beam equation with a) $\tau = 0.2$ and b) $\tau = 0.9$.

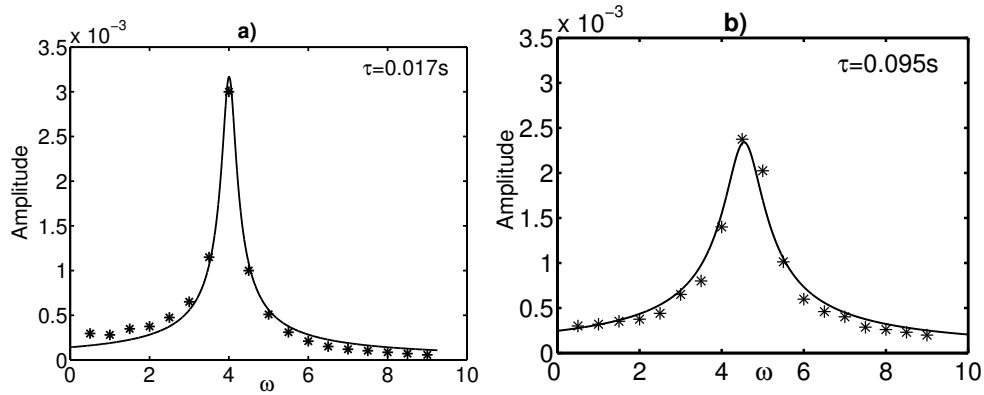


Figure 8. Amplitude response plot. Solid line is the theoretical prediction from (3.9); stars show experimental points.

of mass of the beam to the mass of the MSD μ^* were taken to be constant. The values used in figures 4-6 are $\zeta = 0.003$, $\kappa = 5.4$ and $\mu^* = 0.6$. In principle these values can vary since the parameters of the MSD can easily be changed as they are represented by a numerical model in the experiment. We have tried different combinations of parameters in our numerical simulations and the results are robust and qualitatively similar to those presented in this paper. The equation was forced with an amplitude $\alpha^* = 1$ and frequency $\omega = 5$. The time delay was varied, starting with a value of $\tau = 0.2$ (hence the first stability boundary crossing in figure 3a)). As the trivial steady state of the unforced equation (2.6) is linearly asymptotically stable, external forcing transforms it into a stable periodic orbit. The result of this simulation is shown in figure 4 together with a corresponding phase plane.

For a larger value of the time delay $\tau = 0.9$, the stability of the steady state is lost and the solution develops into a quasi-periodic orbit shown in figure 5. Figure 6 illustrates the solution of the equation (2.6) when the time delay is increased further still. The solution is unstable and it is characterized by a fast growth of the amplitude of oscillations.

To demonstrate the full spatio-temporal dynamics of the beam-MSD system

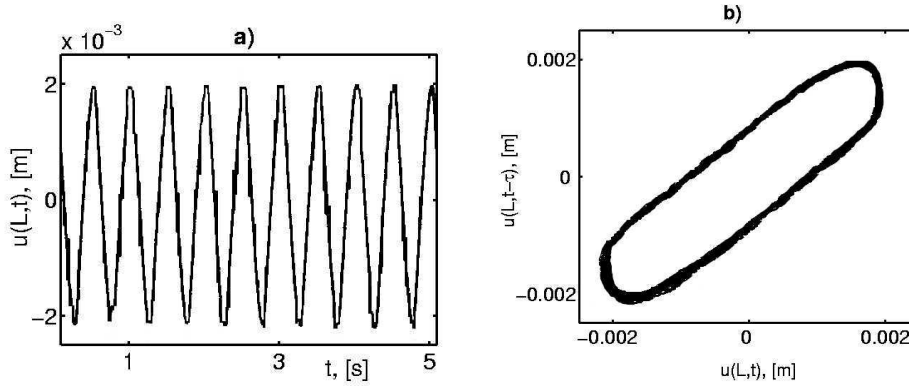


Figure 9. Experimental displacement of the free end of the beam for $\tau = 0.067\text{s}$. a) Temporal dynamics. b) Phase portrait in the plane $(u(L, t), u(L, t - \tau))$.

under external excitation, we solve the system (2.4) numerically. For illustration purposes the number of modes in the expansion is taken to be three. One end of the beam is fixed while the MSD is attached to the other one. The applied external forcing makes the free end of the beam to vibrate. In figure 7a) the value of the delay time is $\tau = 0.2$. One can observe the influence of the higher beam modes which lead to a non-monotonicity of the profile. For $\tau = 0.9$ the beam profile remains qualitatively similar but the quasi-periodic nature of the temporal component induces a complex dynamics as shown in figure 7b).

6. Experimental set-up

In order to confirm our analytical investigations, real-time dynamic substructuring tests are carried out. As discussed earlier, the steel beam is taken to be the physical substructure part, and mass-spring-damper is modelled numerically. The numerical model will be used to calculate the displacement at the interface due to external forcing. This displacement is then applied to the beam in real-time using an electro-mechanical actuator. The beam is mounted on the heavy frame to reduce noise and other unwanted effects. One end of the beam is screwed to the frame, and an actuator is mounted on the free end of the beam. The force acting on the beam is measured using a load cell, an LVDT displacement transducer is used to track and control the movements of the actuator and a digital incremental encoder records the vertical displacement of the beam. In order to implement a real-time testing we use dSpace DS1104 RD Controller board and MATLAB/Simulink are used to program a numerical model.

The beam has a fixed length of 1m with width 5cm and thickness 5mm. The mass, stiffness and damping of the numerical model will be varied during the experiment. First, we take $M = 5\text{kg}$, $C = 1\text{kgs}^{-1}$ and $K = 3500\text{Nm}^{-1}$. The amplitude of the external excitation is $A = 6\text{N}$ and frequency is set to be 2Hz. In figure 8 the amplitude response of the system is shown. The solid line represents an analytical amplitude, and stars are experimental points. Figures 8a) and 8b) are plotted for two different time delays, namely, $\tau = 17\text{ms}$ and $\tau = 95\text{ms}$. The original time delay in the system is 17ms. We can see from figure 8b) that increasing the time delay de-

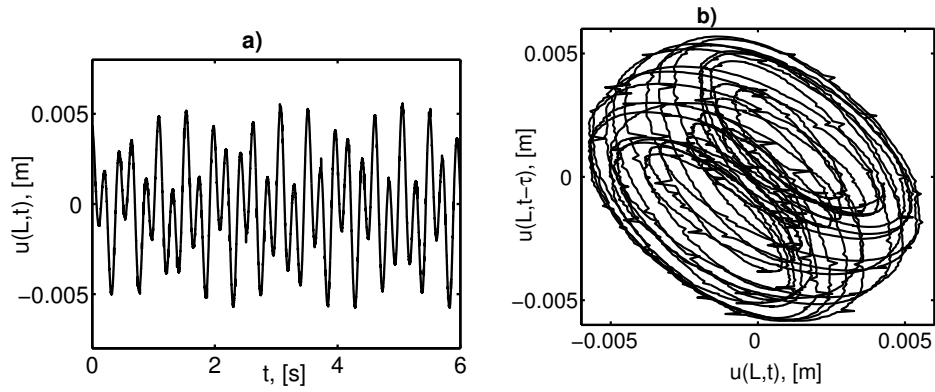


Figure 10. Experimental displacement of the free end of the beam for $\tau = 0.072\text{s}$. a) Temporal dynamics. b) Phase portrait in the plane $(u(L, t), u(L, t - \tau))$.

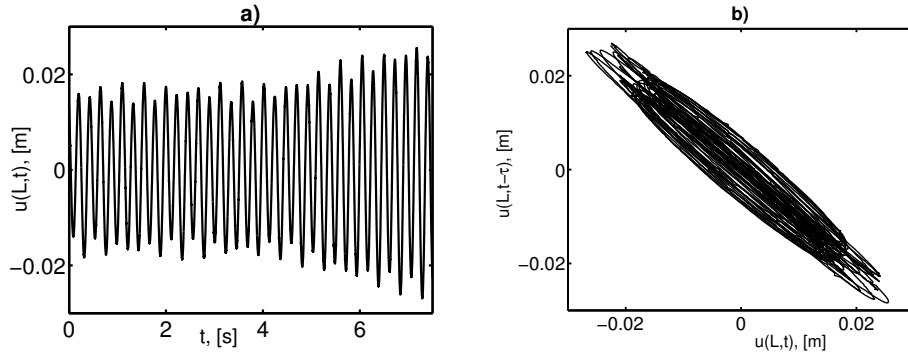


Figure 11. Experimental displacement of the free end of the beam for $\tau = 0.112\text{s}$. a) Temporal dynamics. b) Phase portrait in the plane $(u(L, t), u(L, t - \tau))$.

increases the peak amplitude by around 30%. The experimental points closely follow the theoretical curve which shows a very good accuracy of the analytical predictions.

For experimental results shown in figures 9-11, we fixed all parameters of the system, and increased the time delay. The external force is applied to the numerical model and then the excitation is sent to the free end of the beam. Figure 9 shows periodic oscillations of the beam displacement at the free end for a small time delay. As the time delay is increased, the system undergoes a transition into quasi-periodic motion due to the loss of stability of the trivial steady state as shown in figure 10a). These oscillations are robust against external perturbation, i.e. they do not change to periodic motion if an additional disturbance is applied at the free end of the beam. In this case the dynamics is characterised by large excursions in the phase space (see figure 10b)). As the time delay is increased further still, the quasi-periodic oscillations persist but their amplitude grows. This results in very large transversal displacements of the beam, and eventually the system becomes unstable. This regime is illustrated in figure 11. It is worth noting that experimental beam displacements show dynamics qualitatively very similar to the numerical simulations presented in the previous section.

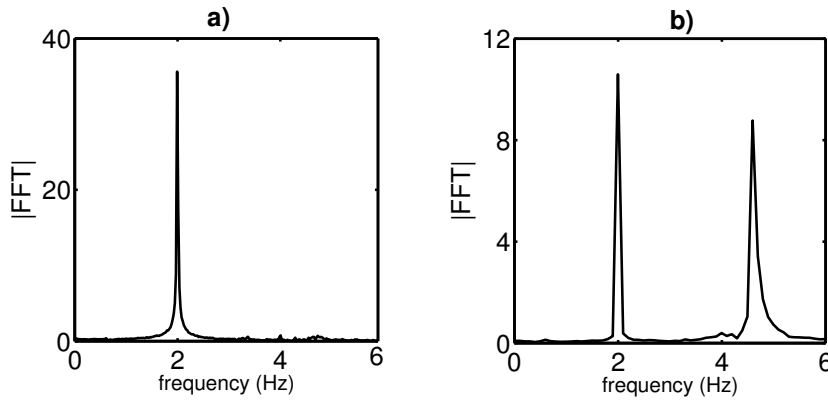


Figure 12. Snapshots of the Fourier spectrum of the experimental displacement of the free end of the beam for different delays. a) $\tau = 0.067\text{s}$, and b) $\tau = 0.072\text{s}$.

In figure 12 we have plotted the Fourier transform of the experimental signals given in figures 9-10. In figure 12a) when $\tau = 0.067\text{s}$, we observe one main frequency in the Fourier spectrum that corresponds to the frequency of external excitation of 2Hz. For a larger time delay $\tau = 0.072\text{s}$, there are two frequencies present in the spectrum as shown in figure 12b). One of them at 2Hz is a forcing frequency, and the second one at $\omega \approx 4.59\text{Hz}$ (analytical value $\omega = 4.65$) is the Hopf frequency that appears when a trivial steady state loses its stability for larger time delay. The Fourier transform of the signal presented in figure 11 is qualitatively similar to the one plotted in figure 12b).

7. Conclusions

This paper deals with modelling of the real-time dynamic experiment of a spatially extended system. The system consists of a steel beam clamped at one end and a mass-spring-damper attached to the free end. This system has been substructured by taking the beam to be the physical structure and a mass-spring-damper replaced by its numerical counterpart. The transfer system used to connect real and 'virtual' parts is an actuator which introduces delays into the experiment. The model equations of motion are given by a partial differential equation with time delay. It is well known that time delays induce instability and analytical treatment of the system allows one to find the regions of system stability and their variation depending on the parameters of the mass-spring-damper. The significant advantage of this experimental technique is that these parameters can easily be varied as they are defined in the numerical part of the experiment. This allows one to switch between different stability regions thereby enhancing the overall stability of the experiment. Despite the fact that time delay is an unwanted effect in real-time dynamic substructuring experiments with stability areas sometimes being very small and narrow what makes them harder to find when running an experiment, the use of the theoretical stability predictions gives insights into their location.

Using the method of multiple scales we find an amplitude response relation of the system depending on the time delay. This is then verified experimentally

giving excellent agreement. We observe that changing time delay helps to reduce the amplitude response peaks. Numerical simulations of an externally forced system demonstrate periodic and quasi-periodic behaviour in different parameter regions. This again gives good correspondence to the experimental observations.

The analytical, numerical and experimental results for the beam-mass-spring-damper system considered in this paper provide important information on the behavioral changes in system dynamics with respect to time delays which are always present in the real-time dynamic experiments. These findings provide much needed insights into the problems and their solutions in the case of larger scale experiments.

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Response to reviewers' comments*Referee 1*

We would like to thank the referee for excellent comments regarding this manuscript. The referee is absolutely correct in saying that the time delay is an unwanted effect in real-time dynamic substructuring experiments. To make this clear, we have amended the appropriate paragraph in the conclusions, as suggested by the referee.

Referee 2

We would like to thank the referee for raising several issues regarding the manuscript. Referee has pointed out that time-varying external load was replaced by a prescribed sinusoid. In fact, this is not the case, as while the external force was indeed chosen to be a sine wave, the mass-spring-damper system was replaced by its numerical model and transfer system.

As stated in the introduction, time delays arise mainly due to the non-instantaneous nature of the transfer system. Of course, unstructured system does not have any delays, however in most cases it is not possible to experimentally test full scale structures and produce real external excitations. Real-time dynamic substructuring is a way forward to overcome the issues of scaling by replacing a part of the system by its numerical counterpart. The main point here is that on this case study system we test mathematical model against experiment in order to make full use of mathematical predictions in more difficult cases such as testing parts of a bridge. In current experiment the time delay is indeed small allowing it to be predicted forward in time. However, as a transfer system becomes more involved, e.g. shaking table or a number of actuators, it is unclear how to deal with different and unknown time delays. This is where the mathematical investigations are the only way to predict the behaviour of the system depending on system parameters before the construction of an actual experimental rig.

Answering to the last comment, we reduced PDDE equation to a system of DDEs, and used a first mode expansion to calculate the amplitude response of the system. The agreement between theoretical and experimental amplitude responses is excellent even with a single mode expansion. We would like to make a comment that this paper is highly multidisciplinary and it contains a comparison between theoretical and experimental results which makes it likely to be of interest to the readers of Proceedings A.

Referee 3

We would like to thank the referee for useful and encouraging comments regarding this work. In specific response to the referee's comments:

- The real-time dynamic substructuring technique is very useful in overcoming the scaling issues involved in a large number of experiments. However, it does have a drawback in the fact that transfer systems (actuator in this paper) are non-instantaneous. Mathematical modelling is used as a way of predicting what will happen to the substructured system under the influence of time delays. We have modelled mathematically a coupled beam-mass-spring-damper system with the inclusion of time delays into the equations to be able to predict delay effects likely to occur in the experiment. The ultimate aim is to apply real-time dynamic substructuring to a more realistic system such as a cable-stayed bridge or a building, and because of the fact that time delays will come from several different actuators, the first step will be to model the full system mathematically, and understand the influence of time delay on the dynamics before conducting an actual experiment.
- The value of the time delay τ in our experimental set-up is 17ms. In order to account for a time delay, we use a prediction forward method, and this gives us a stable experiment with time delay of 17ms for certain values of mass, stiffness and damping of an MSD. However, if prediction forward is not used, the accumulation of delays on each experimental loop leads to desynchronisation of different parts of experiment and eventually to an instability itself.
- For the sake of simplicity of presentation we have used non-dimensional model for theoretical investigations. In section 6 we have presented experimental results and they are dimensional. That is the reason why the values of time delay in Figures 9-11 as opposed to Figures 4-6 are not the same. However, the increase of time delays for numerical simulations was chosen to correspond to the increase of time delay in the experiment. The numerical simulations show behaviour qualitatively similar to the one seen in the experiment.
- We have used a different graphics package and replotted figures 2 and 7. The pictures 2a) and 2b) are now combined in one.