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Supplementary Cementitious Materials: New Sources, Characterization, and Performance Insights

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Abstract

Conventional supplementary cementitious materials (SCMs), such as blast furnace slags or fly ashes, have been used for many decades, and a large body of knowledge has been collected regarding their compositional make-up and their impacts on cement hydration and concrete properties. This accumulated empirical experience can provide a solid, confident base to go beyond the status quo and develop a new generation of low-clinker cements composed of new types and combinations of SCMs. The need for new sources of SCMs has never been greater, as supplies of traditional SCMs are becoming restricted, and the demand for SCMs to reduce CO₂ emissions from concrete production is increasing. In this paper, recent research on emerging SCM sources is reviewed, along with new developments in characterizing and qualifying SCMs for use and improved knowledge of SCMs on long-term concrete performance and durability.

Keywords

(B) Characterization; (C) Durability; (D) Blended Cement; (D) Filler; (D) Pozzolan

1 **1.0 Introduction**

2

3 Supplementary cementitious materials (SCMs) are soluble siliceous, aluminosiliceous, or calcium
4 aluminosiliceous powders used as partial replacements of clinker in cements or as partial replacements
5 of portland cement in concrete mixtures. Many of the materials currently used as SCMs are by-products
6 of other industries, such as fly ash from coal-fired power plants. Some are natural minerals whose
7 energy input in processing is much lower than that used in the manufacturing of portland cement. For
8 example, portland cement clinker has a thermal energy intensity of 3.5 GJ/t clinker [1]. The thermal
9 energy input for fly ash production can be arguably allocated to power production, and not allocated to
10 the fly ash waste product [2]. The calcination of kaolinite clay to make a metakaolin SCM, for example,
11 has a thermal energy input of 0.35 GJ/t clay [1], only 10% of that of portland cement clinker. The energy
12 used in the production of uncalcined natural pozzolans, such as pumicites, is necessarily much lower.
13 Accordingly, life cycle analyses demonstrate a significant reduction in energy use and greenhouse gas
14 emissions associated with concrete mixtures when SCMs are used [3–5].

15

16 SCMs currently serve as one of the primary tools for reducing carbon dioxide emissions associated with
17 concrete production. While in 2009 the Cement Technology Roadmap [6] identified four levers for
18 reducing emissions, including thermal and electric efficiency, alternative fuels, clinker substitution, and
19 carbon capture and storage, it is becoming apparent that the use of SCMs as partial clinker substitutes is
20 the strategy with the lowest economic and performance impacts to cement and concrete production.
21 Indeed, a recent investigation commissioned by the United Nations Environmental Program Sustainable
22 Building and Climate Initiative (UNEP-SBCI) [7] identified clinker substitution and concrete mixture
23 proportioning as the most favorable carbon reduction levers for the industry. Other strategies may
24 become more feasible in a carbon-taxed economy [8].

25

26 The concrete industry also relies on SCMs to improve the long-term mechanical performance and long-
27 term durability of concrete mixtures, both of which play a role in improving efficiency of concrete
28 mixture designs (e.g. increasing strength-to-mass ratios) and increasing longevity, thereby impacting
29 associated emissions over a concrete structure's lifetime [5,7]. The use of SCMs in modern concrete
30 mixtures, therefore, is nearly ubiquitous. Herein lies a problem. The industry has traditionally been
31 dependent on industrial waste streams as the main source for SCMs, e.g. fly ash, ground granulated
32 blast furnace slag, and silica fume. However, in the case of fly ash in particular, the supply chain is
33 threatened. In the US, approximately 40% of coal-fired power plants have closed in the last five years;
34 the UK plans to retire all of its coal-fired power plants by 2025 [9] and the Netherlands by 2030.
35 Furthermore, the demand for cement continues to increase, with demand projected to increase to over
36 6 Gt by 2050 [7]. The industry is changing rapidly, with increases in imports/exports of SCMs between
37 countries [10] and development of new resources for SCMs, the latter of which will be discussed further
38 in this paper.

39

40 Hand in hand with the exploration of new sources of SCMs comes the demand to test and qualify these
41 materials for use. It is becoming increasingly apparent that our traditional, standardized test methods to
42 determine reactivity, among other properties, were developed for sources we already know (e.g. fly ash)
43 and are not appropriate for screening alternative sources of materials with different properties [11,12].
44 Hence, researchers are developing new test methods to characterize novel SCMs, addressed in this
45 paper.

46

47 Research on supplementary cementitious materials is becoming increasingly important. We are moving
48 beyond the testing of small-scale sources for incremental effects on properties and are starting to devise

49 a new system to comprehensively and holistically evaluate any new material to enable its use in
50 concrete materials, thereby expanding our available material resources rather than contracting them. In
51 this paper we explore the new (and re-newed) materials being considered for use as SCMs, new
52 strategies for materials testing and characterization, and new knowledge on the impacts of SCMs on
53 critical aspects of concrete long-term durability.

54

55 **2. New Sources of SCMs**

56

57 A wide variety of materials are available for use as SCMs, including raw and calcined natural minerals,
58 biomass ashes, and industrial by products. Some of these materials are described in Table 1, intended to
59 showcase the variety and availability of SCM resources.

60

61 ***2.1 Natural SCMs***

62

63 To find new SCMs for modern concrete production one can start by looking to the past, at concrete's
64 origins in the Roman cements made from volcanic tuffs. Volcanic materials, including tuffs, ashes,
65 pumicites, perlites and zeolites, can be excellent natural pozzolans, with high amorphous silica and
66 alumina contents, as reviewed recently in a RILEM State-of-the-Art Report [13]. With the increased need
67 for SCMs and shortages in many commonly used materials, natural SCMs are experiencing a sort of
68 renaissance. In the US, a new trade group started in 2017, the Natural Pozzolan Association (NPA), to
69 represent the interests of this growing industry. Natural pozzolans played a strong role in the history of
70 concrete, from ancient times and in the early 20th century [14]. However, they do not have widespread
71 availability and, until recently, they have not been cost-competitive with fly ash and slag. The landscape
72 for natural SCMs is changing, though, and the NPA reports that production capacity is increasing on the

73 order of 500 Kt per year in the US in 2018 and 2019 [15]; this resurgence is accompanied by new
 74 research.

75

76 **Table 1.** Overview of materials used or considered as SCMs [16]. Permission to reprint is provided by a
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78

Material	Chemistry	Used as SCM (Mt/y)	Total volume est. (Mt/y)	Comments
Blast furnace slag	Ca-Si-Al	330	300-360	Nearly fully used, latent hydraulic
Coal fly ash	Si-Al	330-400	700-1100	Subject to limitations on carbon content, reactivity
Natural pozzolans	Si-Al	75	Large accessible reserves	Large variety/variability, often high water demand
Silica fume	Si	0.5-1	1-2.5	Used in high-performance concrete
Calcined clays	Si-Al	3	Large accessible reserves	Metakaolin performs best, often high water demand
Limestone	CaCO ₃	300	Large accessible reserves	Cementitious contribution in combination with reactive aluminates
Biomass ash	Si	0	100-140	Competition with use as soil amendment, high water demand, (for some: high alkali contents)
MSWI bottom ash	Si-Al-Ca	0	30-60	Expansive and corrosive components, leaching issues
Steel slag	Ca-Si-Fe	0 (negligible)	170-250	Various types, can contain expansive components (CaO) or leachable heavy metals (Cr,...). Low reactivity
Copper slag	Fe-Si	0 (some as filler)	30-40	Low reactivity, leaching of heavy metals, more research needed
Other non-ferro slags	Fe-(Si)-(Ca)	0 (some as filler)	5-15 Mt/y each (FeCr, Pb, Ni)	Low reactivity, leaching of heavy metals, more research needed

Bauxite residue	Fe-Al-Si	0	100-150	High alkali content, low reactivity, colour
Waste glass	Si-Na-Ca	0 (or negligible)	50-100	Glass recycling preferable, high alkali content

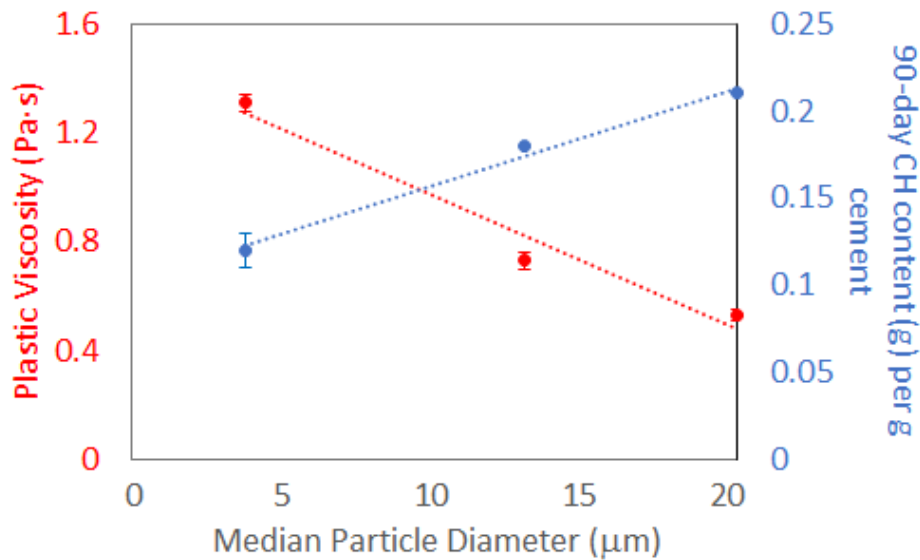
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80 In a paper showcasing new sources of SCMs, Diaz-Loya et al. [17] included natural SCMs, a pumice,
81 perlite, and vitric ash, that all performed similarly to a Class F fly ash, but are in fact more pozzolanic (as
82 indicated by calcium hydroxide consumption in cement pastes). Ghafari et al. [18] also examined perlite
83 and pumice. In spite of frequent concerns that natural SCMs reduce workability, the perlite was not
84 detrimental to fresh state properties. The pumice provided the best strength enhancement, but
85 increased water reducer demand [18]. The impact of pumice and perlite on water demand is related to
86 the particle size distribution; the pozzolanicity of both increases with increased fineness (as measured
87 by calcium hydroxide consumption) [19], but water demand or plastic viscosity also increase (Figure 1)
88 [20–22]. This suggests that the impact of pumice and perlite on reactivity and water demand can be
89 balanced by selection of particle size distribution during grinding or through the use of chemical
90 admixtures. Intergrinding of SCMs with clinker or cement is also possible, and can help when trying to
91 achieve an optimal particle size distribution of the components in blended cement systems [23].

92

93 The impact of natural SCMs on water demand is not only controlled by fineness, but also by specific
94 surface area. Natural zeolites are very porous and have a high internal surface area, thus they have a
95 strong impact on workability [18,24]. It has been shown that calcination of natural zeolites reduces
96 internal surface area, which in turn reduces the viscosity and yield stress of zeolite-cement paste
97 mixtures [24]. Calcination also serves to dehydroxylate smectite clay impurities present in zeolites, also
98 contributing to reduced water demand [24]. Chemical and mechanical treatments have also been shown
99 to modify zeolite reactivity. Burris and Juenger [25] showed that treating natural zeolites with acids de-
100 aluminates them, increasing surface area and thus pozzolanic reactivity (as measured by calcium

101 hydroxide consumption). Likewise, milling zeolites decreases particle size, increasing filler effects and
102 pozzolanic reactivity [26]. The high internal surface area of zeolites also provides unique opportunities
103 for improved concrete performance, such as enhanced internal curing to reduce autogenous shrinkage
104 [27] or self-healing of cracks [28].
105



106
107
108 **Figure 1.** Impact of pumice particle size on the calcium hydroxide (CH) content and plastic viscosity of
109 cement pastes containing 20 wt.% pumice. Higher CH contents suggest less pozzolanic reaction. Higher
110 plastic viscosity indicates higher water demand. Data from [20].

111
112 At the moment, in many regions the cost of quarrying, processing, and transporting natural SCMs may
113 be cost-prohibitive compared to waste-derived SCMs (though this may change as supply/demand shifts).
114 Therefore, there is some interest in exploring natural SCMs derived from waste streams. For example,
115 relatively pure pumicite quarried for other industries may have a layer of overburden, or less pure
116 pumicite. Such overburden pumicite does not have a pure white color and has a lower amorphous

117 content, but is still pozzolanically reactive and suitable as an SCM [12,29]. New material sources should
118 be approached with caution, however, as some mineral waste streams are not suitable as SCMs. For
119 example, quartz and other mineral fines have been shown to pass ASTM C618 [30] specifications for
120 natural pozzolans, even though they are not pozzolanic [12]. This example, along with countless
121 examples from the literature of cement replacements with inert materials, demonstrate the need for
122 better testing, as discussed in Section 3 of this paper.

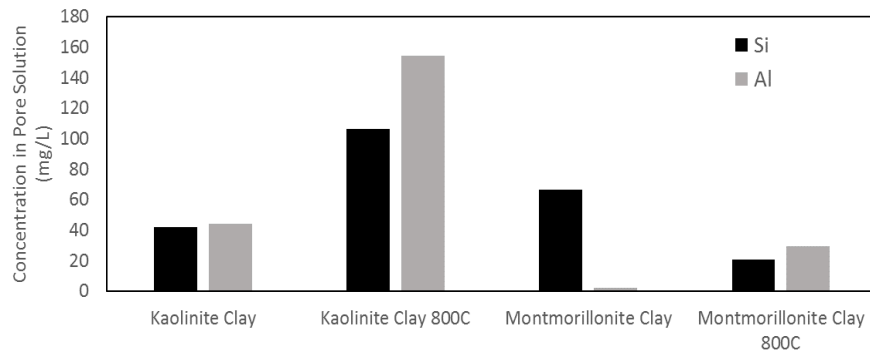
123

124 **2.2 Calcined Natural SCMs**

125

126 Calcined sedimentary materials are also an excellent source of natural SCMs. Calcined kaolinite clay, or
127 metakaolin, has a strong history of use as an SCM in concrete [31]. However, the relatively pure
128 metakaolin that is commonly used as an SCM is also costly because it has limited availability and has
129 value for other industries. Thus, there is interest in calcining other clay minerals and impure clays for use
130 as SCMs. While some clay minerals, such as illite and hectorite, can be dehydroxylated by calcination,
131 they remain inert in cementitious mixtures [32]. Calcined smectite, bentonite, or montmorillonite clays,
132 on the other hand, can be pozzolanically reactive [31–34]. In a study of several purified clay minerals,
133 Hollanders et al. [32] found that calcium-rich smectites are slightly more reactive than sodium-rich ones
134 because they become more amorphous on calcination. Also, smectite clays agglomerate during
135 calcination, reducing specific surface area. With respect to kaolinite, Hollanders et al. [32] found that the
136 degree of ordering of the original kaolinite affects its reactivity after calcination, with higher ordering
137 needing a higher calcination temperature to achieve equivalent amorphization and thus pozzolanicity
138 (as measured by calcium hydroxide consumption). Danner et al. [34] found that calcined kaolinite
139 released more silicon and aluminum into solution than calcined montmorillonite, probably accounting
140 for the increased reactivity of metakaolin (Figure 2).

141



142

143

144 **Figure 2.** Concentrations of silicon and aluminum released into a simulated concrete pore solution by
145 clays containing 47 wt.% kaolinite or 54 wt.% montmorillonite in their original and calcined states. Data
146 from [34].

147

148 Knowledge of the pozzolanicity of smectite clays can be employed to take advantage of low-kaolinite
149 clays as SCM sources. For example, Taylor-Lange et al. [33] tested a clay made by blending kaolinite and
150 sodium-bentonite and also a natural impure clay that contained a high quantity of inert minerals in
151 addition to kaolinite and montmorillonite. They found both to be moderately pozzolanically reactive, as
152 measured by calcium hydroxide consumption. Alujas et al. [35] tested a 40 wt.% kaolinite, 40 wt.%
153 smectite clay and determined that the role of this calcined clay in cementitious systems was part
154 pozzolanic, which increased later age strength, and part filler effect, enhancing early age strength
155 through increased cement hydration. Schulze and Rickert [36] examined 15 different clays of varying
156 purity and compositions. They found that there was no correlation between compressive strength and
157 reactive silica content of the clay, so other characterization methods are necessary to predict reactivity.
158 Clearly, interest in impure clays as SCMs is increasing, and industrial trials have begun to take these from
159 the laboratory into practice [37].

160

161 As with volcanic minerals, it is of interest to reduce cost of calcined clay SCMs by looking to wastes and
162 byproducts of other industries and to underused materials. Snellings et al. [38,39] have looked at clay-
163 rich dredged sediments, for example. Ports, harbors, and waterways are routinely dredged, and the
164 sediments are mechanically de-watered and stockpiled. These sediments may contain clays, so can be
165 calcined and used as SCMs. The calcined sediments from the Port of Antwerp studied by Snellings et al.
166 [39] were more pozzolanic than a fly ash, as measured through heat released from pozzolanic reactions,
167 and the calcined material was found to be free of organic content and have a high amorphous content.
168 Zhou et al. [40], similarly, examined calcined excavated waste clay from London, which contained clay
169 minerals and was shown to be pozzolanic, as measured by the Frattini test. Suraneni et al. [41] and Seraj
170 et al. [42] have found waste fines from the production of lightweight fine aggregates made from
171 calcination of slates and shales to be pozzolanic and suitable as SCMs.

172

173 **2.3 LC³ Materials**

174

175 The worldwide availability of impure clays, their good performance as SCMs, and the observed
176 interaction of aluminosiliceous SCMs with limestone additions to form additional reaction products has
177 resulted in the extensive development of a binder system called LC³ (limestone-calcined clay cement). A
178 recent paper by Scrivener et al. [43] provides an extensive review of this system. The proportions of
179 components can vary, but the most promising combination is one called LC³-50, which contains 50%
180 clinker, 30% calcined clay, 15% limestone, and 5% gypsum. Concrete made with LC³ can achieve
181 improved mechanical properties and permeability compared to concrete with similar mixture
182 proportions containing OPC and 30% fly ash [44]. The calcined clay can be impure, but should have at
183 least 40% kaolinite, since clays with lower amounts of kaolinite are not as pozzolanically reactive (Figure

184 3a). Interestingly, clays with higher kaolinite contents actually cause the hydration of the clinker phases
185 to slow down by 28 days due to pore refinement [45], Figure 3b. Similarly, metakaolin can slow cement
186 hydration by increasing the release of aluminum into solution, inhibiting nucleation of calcium silicate
187 hydrate (C-S-H) [46] or slowing down alite hydration. However, since the reaction of the metakaolin
188 continues in these situations, reacting with limestone to form carboaluminates, strength gain continues.
189 The same effects can be seen when dolomitic limestone is used, since these materials contain reactive
190 calcite [47], and dolomite has been shown to be unreactive at room temperature [48]. Also worth
191 considering is the composition of the cement blended with the limestone and calcined clay. Fernandez
192 et al. [49] showed that in ternary cement-SCM-limestone systems, the C_3A content and alkali content of
193 the cement impact the reaction of the limestone. Carboaluminates and hydrotalcite are only formed at
194 the expense of monosulfoaluminate at early ages when C_3A and alkalis are high, while they form at later
195 ages regardless. This effect influences the rate of reaction and strength gain in these systems.

196

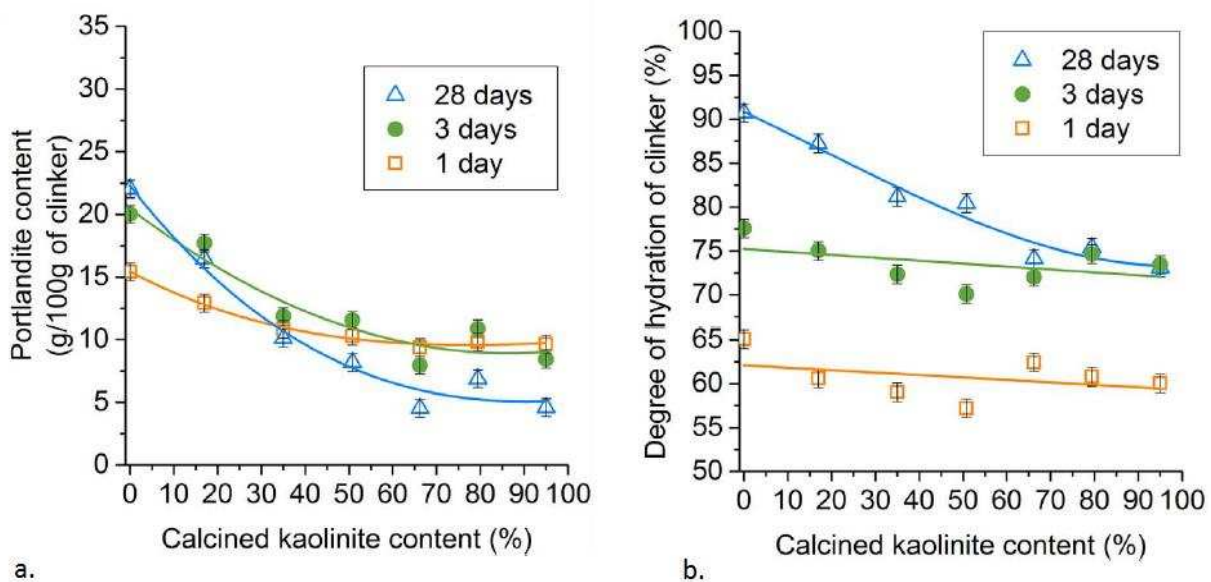
197 Increasing knowledge about LC^3 is helping it move from the laboratory to the field. There have been
198 successful industrial trials of LC^3 in Cuba [50] and India [51], with interest in changing standards to allow
199 classification and implementation of this materials [43].

200

201 Coupled with increasing interest into LC^3 is an increase in research into the role limestone plays in
202 cementitious systems. For example, Bentz et al. [52] found that high volumes of limestone addition have
203 little impact on setting because the particles play an active role in the development of the percolated
204 structure that determines setting. It has also been shown that limestone is a better filler than quartz,
205 not only because of the formation of carboaluminate phases, but because C-S-H sorbs CO_3^{2-} ions that the
206 limestone releases into solution, then the C-S-H releases OH^- into the pore solution through ion
207 exchange, driving C-S-H growth [53]. Additionally, Schöler et al. [54] demonstrated that limestone

208 increases cement hydration kinetics (quantified as an increase in heat flow at the onset of the
 209 acceleratory period) because it increases the undersaturation with respect to alite. The effects on
 210 rheology are very different between limestone and quartz-based fillers, for a similar volumetric
 211 replacement, with a much lower yield stress in limestone-containing mixtures, likely due to increased
 212 inter-particle electrostatic repulsion [52]. This result has implications in additive manufacturing
 213 applications [55]. Similar to systems containing only a single SCM, like pumice, the particle size in a
 214 ternary blended system should be optimized to control and enhance the performance of the
 215 components [56].

216



217

218

219 **Figure 3.** Influence of calcined kaolinite content of impure clays on (a) the portlandite content of cement
 220 pastes determined by thermogravimetric analysis and (b) degree of hydration of clinker determined by
 221 Rietveld quantitative x-ray diffraction. Reprinted from [45] with permission from Elsevier.

222

223 **2.4 By-product Materials: Ashes and Glass**

224

225 Fly ash has been the most commonly used SCM in recent decades, and there continue to be research
226 efforts directed toward understanding its performance, particularly into systems where high volumes of
227 fly ash are combined with fillers and additives to modify performance. Fly ash, like most SCMs,
228 influences strength and setting in complex ways; it behaves like a filler and a pozzolan simultaneously,
229 diluting cement content, and chemically interfering with cement hydration reactions [57]. All of these
230 factors can mask the understanding of fly ash’s hydraulic and pozzolanic contributions to strength gain
231 and performance, and it is important to be able to de-couple these effects through research and
232 improved test methods, as discussed in Section 3. Another avenue of research that is relevant for fly
233 ashes and slag is the investigation of the reactivity of glasses present in these materials. Fly ashes
234 contain a variety of glassy phases, mostly modified aluminosilicates [58]. Both calcium and aluminum
235 are important network modifiers in glasses that affect reactivity [59]. Similar to work examining the role
236 of disorder in clay structures on reactivity, the structural rigidity of glasses in fly ashes has also been
237 shown to affect the rate of dissolution [59,60]. Glass dissolution is complex and dependent on the
238 composition of the surrounding solution, and it has been shown that dissolution is hindered in the
239 presence of calcium and aluminum through the formation of precipitated products on the glass surface
240 [61,62].

241

242 Paris et al. [63] recently reviewed the literature on waste materials that can be used as SCMs. Beyond fly
243 ash and slag, these include a wide variety of sources, such as agricultural waste ashes [64], ashes from
244 biomass used for fuel [65], municipal waste sludge ash [66], oil shale waste [67], and waste glass [68].
245 Most waste ashes need pre- or post-treatment to remove organic matter or metals, but can be suitable
246 for use as SCMs. For example, grinding and classification can reduce surface area and remove quartz

247 from sugar cane bagasse ash, thereby increasing reactivity [69–71]. There are three classes of waste
248 glass: container/bottle, plate/window, and E-glass (fiberglass waste), with E-glass having lower
249 production and a lower alkali content than the others. All are pozzolanic and are suitable as SCMs when
250 ground to sufficient fineness and alkali contents are taken into account in mixture design [68].

251

252 With shortages in fly ash production in many countries and regions, there is interest in examining the
253 performance of fly ashes and other coal combustion residuals that do not meet current specifications
254 and fly ashes that have been landfilled or ponded. Fly ashes that do not meet specifications can be
255 beneficiated or remediated by blending with other materials, classification or separation, chemical
256 passivation, or thermal beneficiation [72]. For example, Al-Shmaisani et al. [29,73] examined fly ashes
257 beneficiated through chemical treatment for high unburned carbon or blending to meet the chemical
258 and physical requirements for a Class F fly ash. All materials performed well in testing for fresh and
259 mechanical properties and concrete durability compared to a production Class F fly ash. In some cases, it
260 may be possible to use an off-spec fly ash as is, accounting for its impact on performance during
261 concrete mixture design. For example, Zunino et al. [74] looked at high-SO₃ fly ashes, those with SO₃
262 contents higher than the ASTM C618 [30] limit of 5.0%. These fly ashes result when systems are installed
263 to control flue gas emissions and can cause delays in setting. Zunino et al. [74] found that the delayed
264 setting can be controlled through washing, limestone addition, or through the use of accelerating
265 admixtures.

266

267 There are increasing research efforts aimed at reclaiming fly ash from landfills and ponds for use as
268 SCMs. In a recent chapter outlining the issues and opportunities for reclaimed fly ash use, McCarthy et
269 al. [9] reported that there could be up to 50 Mt of stockpiled fly ash in the UK and possibly 1 Gt in the
270 US. China could have 2.5 Gt in reserve [75]. Standard specifications for fly ash do not typically prohibit

271 the use of reclaimed material, as long as it meets the necessary chemical and physical characteristics.
272 Depending on the fly ash and the storage conditions, the fly ash needs to be processed, at least
273 minimally, before use since, over time, the ashes agglomerate and acquire moisture. For example, Diaz-
274 Loya et al. [17] have shown that fly ash placed in landfills in the 1970s and 1980s can have similar
275 characteristics to fly ash produced in 2016 after drying and classifying, and mortars blended with them
276 developed comparable compressive strengths compared to fly ashes stored in silos. Low calcium
277 content ashes did not hydrate, and the phase distributions were essentially unchanged after time in the
278 landfill.

279
280 Wirth et al. [76] examined changes in fly ash with weathering and found the presence of hydrated
281 phases in some cases, including calcium sulfates, carbonates and clays, suggesting that compositional
282 changes may occur and should be examined. They also found changes in particle morphology with
283 weathering. Fedorka et al. [77,78] have shown that heating reclaimed fly ash in a proprietary turbulent
284 air system removed organic contaminants, significantly reducing loss on ignition values, making the fly
285 ash suitable for use. Al-Shmaisani et al. [29,73] similarly showed that reclaimed fly ashes perform
286 similarly to production fly ashes, even when moisture contents are slightly higher than allowed by ASTM
287 C618, with differences in performance linked to particle size and shape following grinding or
288 classification.

289

290 ***2.5 By-product Materials: Slags***

291

292 Ground granulated blastfurnace slag (GGBFS) from pig iron production is one of the most commonly
293 used and valuable SCMs. As a latent hydraulic material GGBFS can substitute for clinker to a large extent
294 (up to 95%) and is well-known for enhancing concrete durability. As a consequence, where economics

295 and logistics allow, GGBFS is practically fully used by cement and concrete producers as an SCM. In
296 contrast to GGBFS, other metallurgical slags have found little application as SCMs for a variety of
297 material-related reasons including low reactivity, the presence of incompatible phases such as free CaO
298 or MgO, and environmental and occupational risks related to the heavy metals contained in some slags.
299 Despite these technical challenges, recent years have seen increasing research in the use of other
300 ferrous and non-ferrous metallurgical slags as SCMs. In particular, carbon steel slags generated in Basic
301 Oxygen Furnace (BOF) or Electric Arc Furnace (EAF) steelmaking have received considerable interest.

302

303 Compared to other slags, BOF carbon steel slags contain significantly lower amounts of heavy metals;
304 however, they usually contain free lime or magnesia and non-hydraulic calcium silicate phases.
305 Maturation, a combination of hydration and natural carbonation in open air, is a common practice to
306 reduce the free lime content and avoid volumetric expansion. When added to cementitious mixtures,
307 carbon steel slags have been noted to delay the early hydration of cement and reduce early age strength
308 development [79]. Recent research has shown that this retardation can be mitigated in several ways. For
309 example, Hou et al. [80] showed that precarbonation of the steel slag improved early cement hydration,
310 mitigated volume stability issues, and resulted in a large increase in strength. Elevated temperature
311 curing [79] or carbonation curing [81,82] have also been shown to activate steel slag, resulting in
312 enhanced material performance. Other studies have described activation of carbon steel slag by
313 extensive milling or particle size classification. Shi et al. [83] and Qiang et al. [84] reported clear (latent)
314 hydraulic activity intermediate between fly ash and GGBFS for carbon steel slags of particle sizes below
315 20 μm . Another approach is to modify the steel slag properties by rapid cooling of the slag or using by
316 oxidizing conditions to enhance the stabilization of the alpha and beta polymorphs of C_2S by Fe^{3+} [85].

317

318 Ferrous slags derived from production of stainless steel, alloyed steel, or ferroalloys have somewhat
319 different compositions than carbon steel slags in that they are enriched in Ca-Mg-silicate phases and
320 contain less free lime or magnesia. Heavy metal concentrations and stability are usually more
321 challenging, but depend strongly on the metallurgical process. High-temperature modification of the
322 Ca/Si ratio of stainless steel slag composition can result in beneficial formation of hydraulic phases such
323 as C_3S and alpha or beta C_2S , as shown by Kriskova et al. [86]. Other researchers have looked into using
324 ferronickel and ferrochromium slags. As in case of carbon steel slags, rapid cooling and fine grinding
325 positively influenced the mainly pozzolanic reactivity [87,88] and provide interesting opportunities for
326 application if the environmental quality can be ensured.

327

328 Non-ferrous slags are derived from the production of copper, lead, or other metals in pyrometallurgical
329 processes. Most non-ferrous slags are rich in iron, usually present as oxides or silicates. Recent research
330 by Edwin et al. [89,90] has shown that copper slags have only limited pozzolanic reactivity and mostly
331 acted as fillers in concrete. Fine grinding of the slag [89,91] or introduction of calcium in the high
332 temperature slag processing stage [92] have been shown to increase the reactivity of the slag and
333 provide inroads to enabling the use as an SCM. The presence of significant concentrations of heavy
334 metals, such as residual Zn or Pb, in some slags has been noted to delay the early hydration of cement
335 and poses challenges in terms of meeting environmental quality requirements.

336

337 **3. Advances in SCM Characterization and Testing**

338

339 To tackle the challenge of a diversifying range of SCMs and to overcome shortcomings in existing
340 standardized test methods, there is a clear need for new SCM characterization and testing methods to
341 achieve a better generic understanding of SCM behavior and to more universally and more reliably

342 predict concrete performance and durability [11]. Over the past few years, this need has been
343 addressed on two distinct levels: 1) advancement of in-depth characterization techniques to determine
344 fundamental SCM properties that, for example, can be used as input for microstructural modelling to
345 predict performance and durability, and 2) development of more reliable, relevant and rapid screening
346 tests that aim to judge SCMs on performance, not on origin, and that can be widely used in research,
347 development and manufacturing control of SCMs.

348

349 ***3.1 Advances in SCM Characterization Techniques***

350

351 To build physics-based microstructural models of blended cements, the fundamental material properties
352 of SCMs need to be known in considerable detail. This includes data on the chemical and phase
353 composition and on physical properties such as particle size distribution, density and specific surface
354 area. This information can be obtained from established and widely available methods such as X-ray
355 fluorescence (XRF) spectrometry, quantitative X-ray powder diffraction (XRD), thermogravimetry, laser
356 diffractometry, He pycnometry and BET N₂ adsorption, respectively [93]. Even though these analytical
357 techniques may be routinely used and operate on well-known principles, it is crucial to use adapted,
358 well-calibrated measurement procedures and careful interpretation to obtain accurate and precise
359 results. This was clearly demonstrated by recent work of RILEM TC 238-SCM that investigated the
360 applicability of measurement protocols for a wide range of SCMs and formulated recommendations for
361 routine analysis of particle size, surface area and shape [94], or measurement of loss on ignition by
362 thermal analysis [95]. It is clear that adjustments in analytical procedures are required depending on the
363 SCM type and properties, even for seemingly robust techniques. Table 2 summarizes for common SCM
364 characterization techniques what information can be obtained and what are typical assets and
365 limitations for these techniques.

366

367 Solid state NMR spectroscopy has contributed much to our understanding of cement hydration by
368 delivering quantitative structural information on cement hydrates. Recent contributions using ^{27}Al and
369 ^{29}Si magic-angle spinning (MAS) NMR added to this by shedding light on relationships between
370 composition, local atomic structure and reactivity of SCMs. Thomsen et al. [96] showed the contrasting
371 impact of alkali versus calcium network modifiers on aluminosilicate glass structure where alkali cations
372 preferentially charge balance AlO_4^{3-} tetrahedra, while calcium ions are more associated to
373 depolymerized SiO_4^{4-} tetrahedra. The resulting local structural heterogeneities in the aluminosilicate
374 glass structure were shown to positively affect reactivity [97]. Similarly, Kinnunen et al. [98] found un-
375 mixing or phase separation of aluminosilicate glasses by rapid cooling to increase glass reactivity in
376 alkaline solutions. Also, for calcined clays, detailed NMR studies contributed to explaining reactivity
377 differences between calcined clays based on differences in local structural ordering and bonding
378 environment of the thermally activated materials [34,99]. In an original approach, Ruiz-Santaquiteria
379 and Skibsted [100] used NMR to characterize the residues of dilute dissolution experiments and identify
380 reactive sites in calcined clays. In the future, advanced solid-state NMR methods such as multiple-
381 quantum magic-angle spinning (MQMAS) could be instrumental in SCM characterization by offering
382 increased spectral resolution for quadrupolar nuclei such as ^{27}Al or ^{23}Na [101]. This was demonstrated
383 recently by the identification of different structural sites of Al in ettringite or ye'elimite [102,103].

384

385 Unfortunately NMR spectroscopy cannot be used to study materials containing significant
386 concentrations of paramagnetic ions such as iron. Instead Pair Distribution Function (PDF) analysis of
387 neutron and X-ray scattering data can be used to acquire information on interatomic distances and infer
388 changes in local structure as a function of compositional changes or hydration [104]. Natali et al. [105],

389 for instance, studied different types of Class F fly ash using PDF, showing major changes in fly ash glass
 390 atomic structure for high concentrations of aluminum, iron or carbon (graphite).

391

392 **Table 2.** Common SCM characterization techniques summarizing the information probed, assets, and
 393 limitations.

394

Technique	Information	Assets	Limitations
Laser diffractometry	Particle size distribution – size range 0.1 to 1000 μm	Common, rapid, reproducible measurement	Optical model and sample preparation sensitive (disagglomeration, dispersion,...)
He pycnometry	Density	Common, rapid, reproducible, straightforward	Dry sample required, in case of closed porosity not accessible to He an apparent density is measured
BET N₂ sorption	Specific surface area, size and shape of pores	Common, includes internal porosity	Degassing/drying procedure may lead to changes in sample properties, not for coarse materials
Blaine air permeability	Fineness	Reference method for cement, standardised	Not easily amenable to very fine or rough powders
X-Ray Fluorescence spectrometry (XRF)	Chemical composition	Rapid, straightforward, accurate	Calibration with similar reference materials is required to account for matrix effects Preparation of sample beads leads to loss of volatile elements, e.g. alkalis
Thermogravimetric analysis (TGA)	Loss On Ignition, volatiles/thermally unstable phases	Accessible, straightforward, quantitative	Interference between weight changing events (e.g. oxidation, decarbonation,...), dependence on sample size and instrument geometry
X-Ray powder Diffraction (XRD)	Mineralogy, phase composition	Accessible, rapid, multiple phase identification and quantification	Quantitative analysis requires extensive knowledge Amorphous phases give diffuse signal, complex analysis
Solid state Nuclear Magnetic Resonance (NMR)	Local structural order, element specific (²⁷ Al, ²⁹ Si,...)	Atomic structure information, applicable to	Accessibility of instruments, expert support required

spectroscopy		amorphous phases	
Scanning Electron Microscopy (SEM)	Spatially resolved phase information (size, shape, texture, distribution)	Can be combined with microchemical analysis	Accessibility, sample preparation of polished sections and data interpretation requires expertise, time and resources

395

396

397 One of the most important properties of an SCM is its chemical reactivity in a hydrating portland cement
398 environment. However, it is not straightforward to measure this. In cement pastes, the reaction of the
399 SCM interferes with the hydration of clinker minerals and can be affected by the formulation of the
400 cement [106,107]. To isolate the reaction of the SCM, a common approach is to use simplified model
401 systems. The most elementary approach is to use highly dilute systems in which the rate of dissolution
402 of the SCM can be measured, avoiding precipitation of reaction products [108]. The rate of dissolution
403 can be calculated from the solution composition, using Si or Al dissolved from the SCM in solution as a
404 tracer. These studies demonstrated the close correlation between the atomic structure of the SCM and
405 its reactivity [99]. In particular, the connectivity of the aluminosilicate glass structure was found to
406 correlate with dissolution rates and reactivity in blended cements [59,108,109]. For calcined kaolinite,
407 the dissolution rate using Al as tracer was found to correlate well with reactivity in cement pastes [110].
408 In addition, dissolution studies have shed new light on the interaction between solution chemistry and
409 SCM reactivity, such as the accelerating effect of pH or the inhibiting effect of dissolved Ca or Al
410 [61,111,112].

411

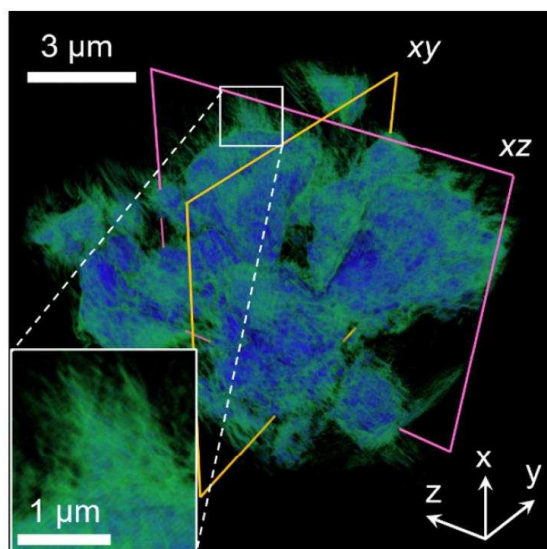
412 For heterogeneous materials, it is of interest to determine physico-chemical properties down to the
413 level of individual components of the SCM. This can be done using electron microscopy combined with
414 EDX microchemical analysis [113]. Recent studies have shown that image processing enables
415 identification of subpopulations of components based on chemical composition [114]. Complementary
416 information, such as a particle size distribution or averaged reactive surface area, can be measured and

417 attributed to these subpopulations. This method was recently applied to explain the reaction rates of
418 chemically distinctive fly ash components in both model systems [115] and cement pastes [109].
419 Moreover, the proportions of different compositional groups in a range of fly ashes have been shown to
420 correlate to compressive strength development in mortar bars [116]. Blast furnace slags [117] and
421 calcined dredging sediments [38] have also been studied by this method. The application of the
422 technique may very well find its way to other systems as well. Drawbacks are that the technique is
423 inherently limited by the spatial resolution of the EDX interaction volume (1-2 μm) and that the
424 optimization of the image processing routines can be time-consuming if done manually. Further
425 progress on the latter may be made by adopting approaches rooted in artificial intelligence such as
426 supervised machine learning for object recognition [118].

427
428 Advances in sub-micrometer scale imaging may well deliver more insight into SCM reaction mechanisms
429 and features such as lattice defects that control reactivity. Detailed characterization of SCMs and their
430 reaction products down to the nanoscale has been pursued, taking advantage of the continuous
431 development of advanced characterization techniques accessible in specialized research facilities.
432 Combined with focused ion beam (FIB) milling to prepare sample foils, scanning transmission electron
433 microscopy has enabled observation of hydration product morphologies and unambiguous
434 determination of their composition in blended cements that contain sub-micrometer SCMs such as silica
435 fume or metakaolin [119]. FIB was also used to prepare thin slits in a blast furnace slag surface. These
436 were used as so-called microreactors to observe the early hydration of the slag in alkaline solutions by
437 SEM [112]. More quantitatively, 3D images of dissolving SCM surfaces have been acquired to calculate
438 dissolution rates using nanoscale surface topography measurement techniques such as atomic force
439 microscopy (AFM) or vertical scanning interferometry (VSI) [60,120]. These techniques not only measure
440 bulk dissolution rates, but, by visualizing the retreating surface, can also track the effect of lattice

441 defects, inclusions and other nanoscale features in crystalline materials. 3D images of entire hydrating
442 C_3S and C_3A particles at nanoscale resolution has been made possible by recent developments in
443 synchrotron imaging techniques. Nanoscale X-ray tomography, ptychographic imaging and X-ray
444 fluorescence microscopy were recently used to study the early hydration of C_3S [121] and C_3A [122] in
445 3D at unprecedented resolution, as shown in Figure 4. These techniques may also be applied to observe
446 how individual SCM particles react.

447



448

449

450 **Figure 4.** 3D X-ray nanotomography reconstruction of C_3A particles hydrated in the presence of gypsum
451 for 143 min. C_3A is shown in blue, hydration products in green. Reprinted from [122] with permission
452 from Elsevier.

453

454 **3.2 Advances in SCM Reactivity Testing**

455

456 In the long run, advances in in-depth characterization techniques are key to fundamentally understand,
457 and eventually predict, the behavior and performance of blended cements based on fundamental

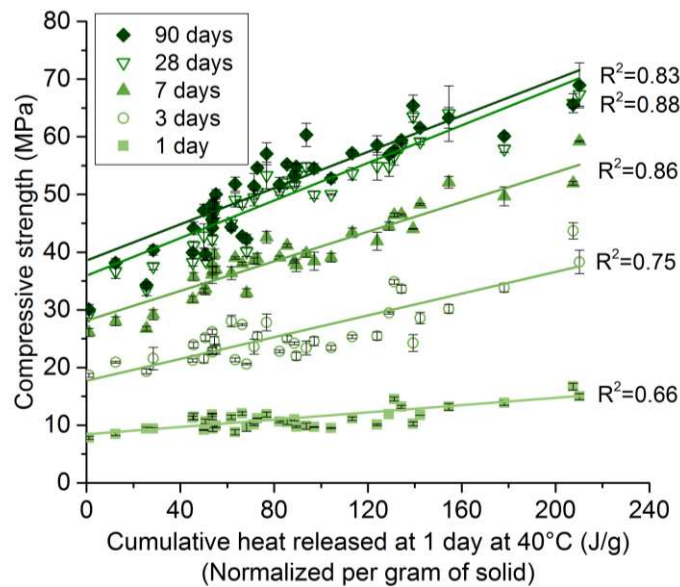
458 structure-property relationships [123]. However, in the short term, more practical tests are needed to
459 expedite the development of the next generation of high performance, durable and sustainable
460 cements. In this respect, a SCM screening reactivity test should be practical, reproducible, applicable to
461 a wide range of SCMs and deliver a relevant result to evaluate the performance of the SCM in cement.
462 Unfortunately, most of the existing test methods are found to fall short on one or more of the test
463 method requirements, in particular regarding the range of SCMs covered and correlation to
464 performance. In case of methods that measure “reactive silica” or consumption of $\text{Ca}(\text{OH})_2$ as a proxy for
465 SCM reactivity, such as the Chappelle or the Frattini tests, correlation to the compressive strength
466 development of benchmark mortar bars is not clearly apparent and cannot be extended to cover both
467 pozzolanic and latent hydraulic materials [11,124,125]. In addition, a common weakness is the
468 robustness of test methods due to deficiencies in protocol descriptions or reagent specifications [126].
469 On the other hand, test methods based on strength development, such as the ASTM C 311 strength
470 activity index, often require 28 to 90 days before the contribution of the SCM becomes evident and
471 cannot distinguish between physical packing or filling, and chemical reactivity effects, which can
472 eventually lead to ambiguity as regards to material classification [12,127].

473

474 In a concerted effort to make progress on improving SCM reactivity test methods, both RILEM and ASTM
475 initiatives were set up to evaluate standardized and newly-proposed SCM reactivity tests against criteria
476 such as correlation to strength development or interlaboratory reproducibility. In the work of the RILEM
477 TC 267-TRM, a wide range of SCMs was considered, including both pozzolanic and latent hydraulic
478 materials. The results indicated that measurements of heat release or bound water in so-called R^3 model
479 pastes performed best to quantify both the pozzolanic and latent hydraulic reactivity simultaneously
480 [124]. The R^3 model paste supplies an excess of $\text{Ca}(\text{OH})_2$ and water to measure the unconstrained
481 reaction of the SCM. Additional alkali, sulfate and calcite are added to simulate a typical portland

482 cement reaction environment. The pastes are cured at 40°C to accelerate the SCM reaction. Elimination
 483 of portland cement enables measurement of the reaction of the SCM directly by robust, accessible
 484 properties such as heat release or bound water. Moreover, mutual interference between clinker and
 485 SCM reactions can be avoided. The R³ test method was first applied to calcined kaolinitic clays by Avet et
 486 al. [128], showing good correlation to compressive strength for cumulative heat, bound water and also
 487 chemical shrinkage results. This is illustrated for the R³ model paste cumulative heat at 1 day and
 488 compressive strength at various ages of standard EN 197-1 mortar bars in which portland cement was
 489 replaced by 35% of calcined clay and 15% of limestone in Figure 5.

490



491

492

493 **Figure 5.** Correlation between mortar strength (20°C) and cumulative heat released at 1 day of hydration
 494 for isothermal calorimetry run at 40 °C for calcined clay-portlandite-limestone R³ model systems.

495 Reprinted with permission from [129].

496

497 Applied to a broader range of conventional SCMs of comparable particle size distribution as portland
498 cement, the correlation of the test results to the mortar compressive strength at 30 wt% cement
499 replacement was shown to hold, first by Snellings and Scrivener [11] and subsequently by the more
500 extensive work of the RILEM TC 267-TRM [124]. To distinguish between pozzolanic and latent hydraulic
501 materials, Suraneni and Weiss [130] suggested to complement the results with Ca(OH)_2 consumption as
502 measured by thermogravimetry. The phenomenological explanation of the strong correlations between
503 the measured heat release, chemical shrinkage and bound water in the R^3 test is that these are all direct
504 manifestations of the formation of hydration products. The solidification of water by formation of
505 common cement hydrates is accompanied by a remarkably constant reaction enthalpy [131], which is
506 proposed to be behind the convergence of trends for bound water and isothermal calorimetry.

507

508 **4. Understanding the Impact of SCMs on Concrete Durability**

509

510 As discussed in previous sections, SCMs modify the type of hydration products forming in portland
511 blended cements and can also improve particle packing, pore refinement and reduce connectivity of
512 pores in concrete produced with them [132]. It is well known that the combined response to aggressive
513 agents of hydration products present in blended cements, for example their chloride binding capacity
514 [133–135], along with improved transport properties, enhances the long term performance and
515 durability of SCM-containing concrete [136]. Detailed information about the characteristics, including
516 permeability and chemical durability, of concrete with conventional SCMs such as blast furnace slag, fly
517 ash, natural SCMs and alternative SCMs, has been recently published in the state-of-the-art report of the
518 RILEM Technical Committee (TC) 238-SCM [137]. Here, a summary of the recent advances on durability
519 of concrete containing natural or new sources of SCMs is presented.

520

521 ***4.1 Durability of Concrete Containing Natural SCMs***

522

523 The durability of concrete with natural SCMs, such as calcined clays [138] and volcanic ashes and tuffs
524 [139], has been extensively assessed, as outlined in the RILEM TC 238-SCM state-of-the-art report [137].
525 It is well-established that natural SCMs contribute to the long-term compressive strength development
526 of concrete and overall can significantly improve durability, which is one of the main incentives for their
527 use in concrete production [140]. There is a good consensus that concrete containing natural SCMs can
528 present low chloride permeability [141,142], high sulfate resistance [143], and in most cases reduction
529 of alkali-silica reaction [144]. However, due to the variability in chemical composition and reactivity of
530 natural SCMs, their effects on the properties of cement and concrete can vary significantly depending on
531 the source and amount of the natural SCMs used.

532

533 There are a limited number of reports on durability of concrete containing new sources of natural SCMs
534 [63], as most of these materials have previously been used for other purposes (pumice [145] and several
535 agricultural wastes [146] have been used as aggregates, for example), or their applicability as SCMs was
536 not identified until recent years (e.g. dredged sediments [38]). In spite of this, the existing knowledge
537 about the correlations between the type of hydration products forming in cements blended with
538 conventional SCMs and their response to different aggressive agents may be transferred to concrete
539 containing new sources of SCMs. Hence, more studies focusing on detailed evaluation of the
540 microstructural features, including phase assemblage evolution, pore structure, and chemical resistance
541 of cement containing natural or new sources of SCMs are urgently needed, to elucidate how concrete
542 containing these materials might perform in service. In addition, more studies determining the corrosion
543 mechanism of steel reinforcing bars embedded in concrete with natural SCMs are urgently needed,
544 particularly evaluating the effect of carbonation and/or chlorides on the service life of reinforced

545 concrete. A recent study by De Weerd et al. [147] demonstrated that carbonation reduces the alkali
546 metal concentration in the pore solution of concrete, coupled with an increase in chlorine and sulfur
547 concentrations, which can further accelerate corrosion in carbonated concrete.

548

549 *4.1.1 Natural Zeolites and Pumice*

550

551 Clinoptilolite is one of the most common zeolite minerals and, as such, also one of the most widely used
552 zeolites in production of concrete. Addition of 10 -20% improves chloride chemical binding in concrete
553 via formation of Friedel's salt [148], which explains the reduced chloride penetration reported in
554 concrete containing this natural SCM [149]. In combination with chemical admixtures (e.g.
555 superplasticizers and/or air entraining agents), clinoptilolite-type zeolites decrease the water
556 penetration, while increasing freeze-thaw resistance of concrete [150,151]. Good chemical resistance
557 has also been reported [152] for clinoptilolite-containing concrete when exposed to $MgCl_2$, NH_4Cl ,
558 Na_2SO_4 , and HCl , when compared with plain portland cement systems. Their effect when concrete is
559 exposed to H_2SO_4 is strongly dependent on the amount of zeolite added [153]. Concrete containing this
560 SCM also exhibits high susceptibility to carbonation when tested under accelerated (5% CO_2) conditions.
561 However, under natural carbonation conditions, carbonation resistance was much better, and a
562 carbonation front was solely identified at high zeolite contents (25 wt.% replacement) after 15 months
563 of exposure [153].

564

565 The addition of ground pumice as an SCM in concrete reduces early age compressive strength, along
566 with a decrease of water absorption, sorptivity, and volume of permeable voids at extended curing
567 times [154]. Madani et al. [155] identified in blended concrete with high volumes of pumice (up to 60
568 wt.%) negligible differences in chloride diffusivity, water absorption and volume of permeable voids,

569 compared with plain portland cement concrete, after 28 days of curing; however, improvements in all
570 these properties were observed after one year of curing. The addition of low contents of silica fume (e.g.
571 3 wt.%) significantly improves the chloride resistance of ternary mixtures of portland cement-pumice-
572 silica fume at early ages. This is associated with a reduced water absorption and refined microstructure
573 development [155].

574

575 An increase in the durability against magnesium sulfate attack has been observed in pumice-containing
576 systems [153,154,156], as well as a reduction in shrinkage strain at longer drying durations. Good
577 resistance to sulfuric acid attack has also been reported for pumice-containing concrete [153]. Concrete
578 with less than 10% addition of pumice also presents a carbonation resistance comparable to that of
579 concrete without SCMs, when tested under accelerated (5% CO₂) and natural testing conditions [153].
580 Increasing the pumice content significantly reduced the carbonation resistance of these materials.

581

582 *4.1.2 Non-Kaolinitic Clays and Sediments*

583

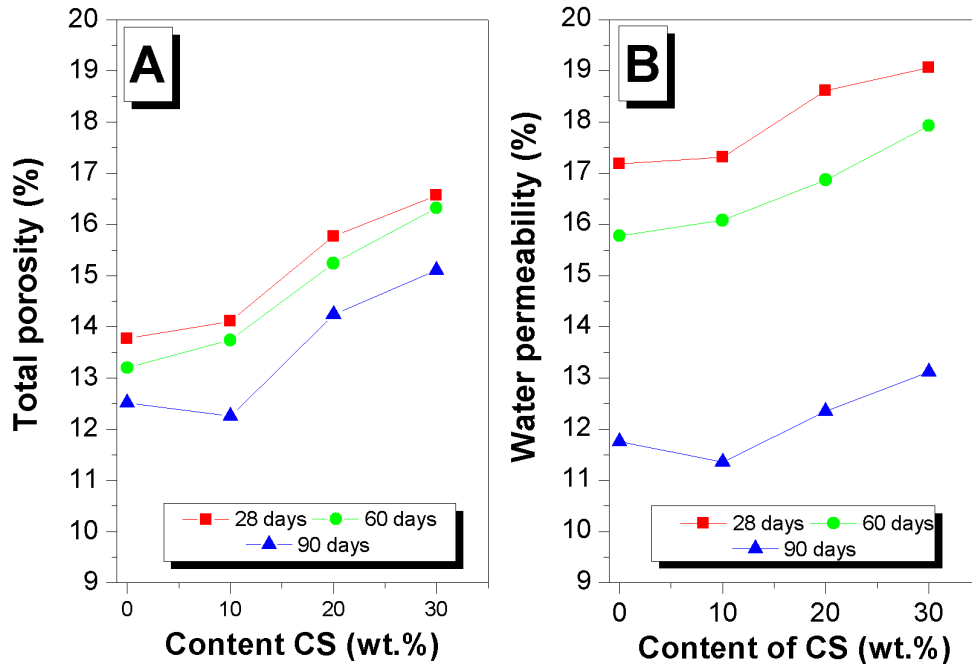
584 The majority of studies on non-kaolinitic clays have centered on determining the best pre-processing
585 treatments to improve their reactivity and the phase assemblages forming upon blending with portland
586 cement, as was discussed in previous sections. Studies of durability of concrete containing uncalcined,
587 non-kaolinitic clays are mainly centered on determining their effects when present as a contaminant in
588 aggregates [157,158], although in recent years, these materials have also attracted interest as
589 nanomaterials with the potential to enhance strength and durability of concrete [159]. For example,
590 Langaroud et al. [160] identified that small additions of a montmorillonite-based nano-clay (i.e. 95%
591 purity and maximum size of 1-2 nm) (< 3%) in self-consolidating concrete, with and without inclusion of

592 other SCMs (e.g. rice husk ash, silicate fume, blast furnace slag or fly ash), promotes significant
593 reductions of the water and chloride permeability, particularly at 3% addition.

594

595 There are a limited number of durability studies of concrete containing calcined sediments. Amar et al.
596 [161] identified that mortars with 10% portland cement substitution by calcined dredged sediments
597 present a similar freeze-thawing, chloride permeability and sulfate attack performance compared with
598 materials without SCMs. Higher contents of this SCM led to a significant reduction in durability
599 performance of these materials. This was associated with increased porosity and water permeability at
600 high replacement levels (Figure 6). Safer et al. [162] observed in concrete with up to 20% calcined
601 dredged sediment a comparable resistance to H_2SO_4 attack to that of concrete without SCMs. Snellings
602 et al. [39] reported a comparable hydration products phase assemblage in plain cement systems and
603 those with up to 30% of a flash calcined dredged sediment. Van Bunderen et al. [163] also identified for
604 concrete with 30% calcined sediment an increased resistance to chloride ingress compared to portland
605 cement without SCMs, consistent with a refined pore structure as a result of the pozzolanic reactions;
606 however, the scaling resistance was reduced when evaluated in presence of de-icing salts.

607



608

609

610 **Figure 6.** Total porosity determined by mercury intrusion porosimetry, and water permeability of
 611 mortars containing calcined dredged (CS) sediment, as a function of the time of curing. Data from [161].

612

613 The differences in performance reported among the few durability studies of calcined sediment SCMs
 614 when testing concrete with different amounts of this SCM are likely a consequence of combined
 615 variations in the chemical composition of sediments from different sources and the calcination
 616 processes used, which modifies their physical properties and their reactivity. Further studies about the
 617 factors governing the durability of these systems are required to identify strategies to mitigate their
 618 potential damage, particularly when exposed to freeze-thaw conditions.

619

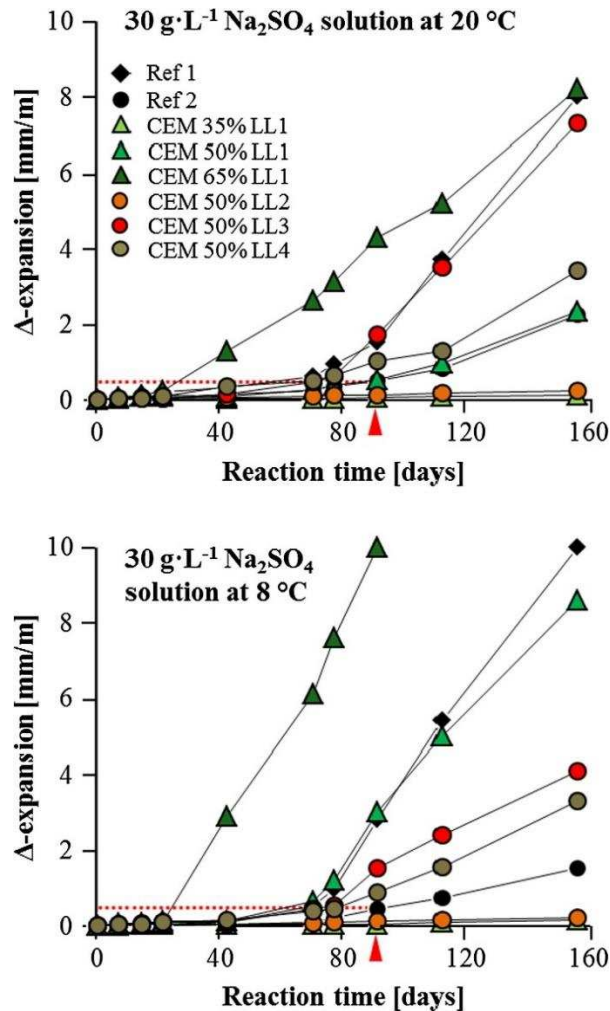
620 *4.1.3 Portland Cement-Limestone-SCMs*

621

622 For decades, portland limestone cements have been widely used globally. Limestone powder has a
623 relative low cost, low energy consumption and reduced global warming potential. The filler, dissolution,
624 and chemical effects of limestone powder in concrete are dependent on the particle size, dosage,
625 dissolution, and mineral composition of the cement, among other factors that have been extensively
626 documented, as outlined in a recent review by Wang et al. [164]. The mechanical and durability
627 properties of portland limestone cements are also discussed in that review, where it is highlighted that a
628 high content of limestone in the concrete increases susceptibility to sulfate attack, particularly
629 thaumasite formation [165]. For this reason, the amount of limestone addition in Europe is limited up to
630 20% limestone for CEM II/A-L and up to 35% for CEM II/B-L, and blended portland cements containing
631 up to 15% limestone have recently been introduced into Canada and the US [166].

632
633 Palm et al. [167] identified that low-water concrete (e.g. water/cement ratio of 0.35) with higher
634 contents of limestone than those allowed in European and American standards (between 35 wt.% - 65
635 wt.% of CEM I replacement) is comparable to concrete designed according to EN 206 [4]. A lower
636 chloride permeability was obtained in concrete with the highest compressive strength, consistent with a
637 reduced content of air in the mixes. However, freeze-thaw resistance was reduced at low air contents.
638 Concrete with up to 50 wt.% limestone presents good sulfate resistance (Figure 7), if designed with low
639 water contents, and optimized packing density [168]. This is true even in samples tested at 8°C (Figure
640 7b), where thaumasite precipitation could be expected in the presence of high concentrations of
641 SO_4^{2-} (up to $30 \text{ g}\cdot\text{L}^{-1}$), Ca^{2+} , CO_3^{2-} , and silicic acid, which are the conditions prevailing in the tested
642 systems. Sulfate resistance can also be improved by addition of other SCMs. It has also been reported
643 that the use of limestone with high CaCO_3 content particularly facilitated thaumasite formation. Rezvani
644 et al. [169] demonstrated that the content, chemistry (e.g. varying content of CaO , SiO_2 , Al_2O_3 , Na_2O ,
645 among others), and mineralogy (e.g. 97.8 – 66.9 wt.% CaCO_3) of the limestone influences the drying

646 shrinkage-related microcracking of concrete with up to 70 wt.% limestone. Rezvani et al. [169]
 647 developed an analytical model, integrating mechanics of porous materials and thermodynamics, to
 648 predict drying shrinkage of these materials.
 649



650
 651
 652 **Figure 7.** Strain development of mortars after 158 days of storage in a Na₂SO₄ solution. Expansion is
 653 reported as Δ-expansion, relative to the mortars immersed in a Ca(OH)₂ reference solution at 20 °C. The
 654 SVA¹-guideline limit for a high sulfate resistance of the test specimens is ≤0.5 mm/m after 91 days

¹ SVA is the sulfate resistance testing method of the committee of experts of the German Institute for Civil Engineering described in [170].

655 (marked with the dotted red line and the red triangle) [170]. LL corresponds to limestone with CaCO_3 of
656 97.5% (LL1), 79.7% (LL2), 77.5% (LL3), and 77.4% (LL4), respectively. Reprinted from [168] with
657 permission from Elsevier.

658

659 As mentioned earlier, the LC^3 system is attracting particular attention from academia and industry.
660 Scrivener et al. [43] reported that limestone calcined clay cement (LC^3)-based concrete presents
661 excellent resistance to chloride penetration, mitigation of alkali-silica reaction, and durability in the
662 presence of sulfates. This is associated with a high degree of pore refinement, which is dependent on
663 the calcined kaolinite content [45]. More pore refinement is observed in systems produced with clays
664 with up to 50 wt.% kaolinite. This pore refinement particularly enhances the chloride resistance, as it
665 has been observed that for some LC^3 formulations, their total binding capacity is comparable to that of
666 portland cement systems in spite of some Friedel's salt formation [171]. Shi et al. [172,173] identified
667 that in LC^3 systems the chloride binding capacity is actually improved with the addition of metakaolin
668 due to an increased formation of Friedel's salts, compared with plain portland cement systems. This is a
669 consequence of metakaolin acting as an additional source of aluminum. The differences in results
670 between these studies are most likely due to the differences in purity of the clays or calcination process
671 used to produce the metakaolin. The content of soluble calcium in these systems also seems to be a
672 determining factor in the chloride binding capacity of materials with limestone, independently of
673 inclusion of metakaolin [173]. It is interesting that studies reveal that the use of high-grade clays is not
674 necessary to obtain significant improvement in chloride penetration resistance in LC^3 systems [174], due
675 to the higher resistivity [44,174] and refined pore structure.

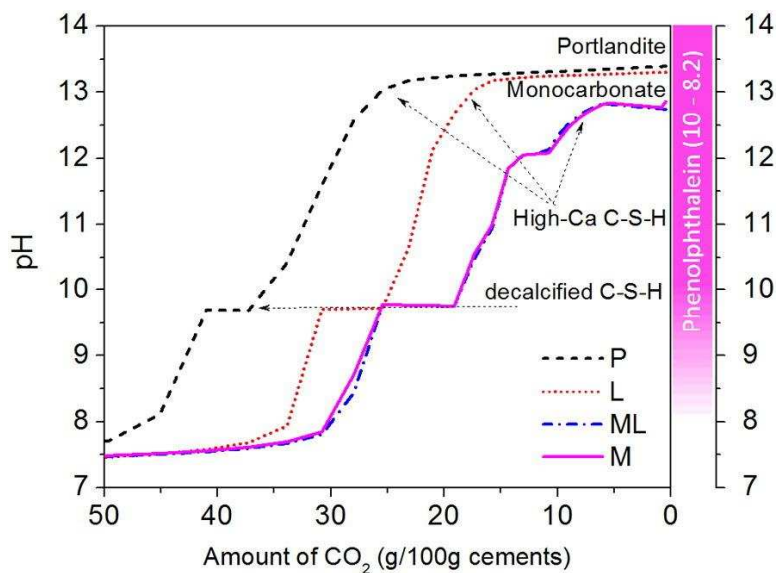
676

677 A high resistance to sodium sulfate attack has also been identified for these materials [175], produced
678 with either calcined kaolinite or calcined mortmorillonite. This was attributed to the consumption of

679 portlandite by the pozzolanic reactions of the calcined clays and the dilution of the portland cement
 680 clinker, leading to a lower amount of calcium available for the secondary formation of gypsum and
 681 ettringite. Formation of thaumasite was not observed in limestone-metakaolin-portland cement systems
 682 tested at 5°C, consistent with the observations reported by Mirvalad et al. [176].

683
 684 Shi et al. [177] observed higher carbonation resistance of LC³ materials, comparable with those solely
 685 blended with limestone, consistent with the refined pore structure reported for LC³ systems [45]. In
 686 these materials, the changes in pH detected by phenolphthalein indicator are associated with a two-step
 687 carbonation of the C-S-H phase. Upon carbonation of high-Ca C-S-H, an initial depletion of Ca²⁺ ions in
 688 the interlayer space occurs. As carbonation progresses, the resultant C-S-H with a lower calcium content
 689 than the original C-S-H degrades to form a silica gel and calcium carbonate (Figure 8) [177]. LC³-based
 690 concrete showed a comparable performance to that observed for concrete containing fly ash, but
 691 presented higher carbonation rates than concrete blended with blast furnace slag [178].

692



693
 694 **Figure 8.** pH profiles related to the changes in phase assemblages predicted by thermodynamic
 695 modeling. The right bar indicates the gradual color change of phenolphthalein from fuchsia to colorless

696 upon pH changes from 10 to 8.2. *P* corresponds to 100 wt.% white portland cement; *L* – 68.1 wt.% white
697 portland cement + 31.9 wt.% limestone; *ML* to 68.1 wt.% white portland cement + 25.5 wt.% metakaolin
698 + 6.4 wt.% limestone; and *M* – 68.1 wt.% white portland cement + 31.9 wt.% metakaolin. Reprinted
699 from [177] with permission from Elsevier.

700

701 Global scarcity of high-grade limestone has motivated investigation on other carbonate sources for
702 production of concrete. Dolomite ($\text{CaMg}(\text{CO}_3)_2$) in particular has been identified as a potential substitute
703 of limestone. Zajac et al. [178] demonstrated that the reaction of dolomite in portland cement pastes
704 leads to formation of comparable phase assemblages to those of limestone added cements. Dolomite
705 acts as a magnesium source aiding formation of hydrotalcite, which contributes to the compressive
706 strength development of these systems. Hydrotalcites can also play an important role in improving
707 durability performance of concrete. Ke et al. [179] showed that hydrotalcites can take up chloride from
708 highly alkaline solutions with different initial $[\text{Cl}^-]/[\text{OH}^-]$ ratios, via a combined surface adsorption and
709 ion exchange mechanism. Machner et al. [135] assessed the chloride binding capacity of materials with
710 40 wt.% portland cement replacement by either limestone or dolomite, in combination with metakaolin
711 (5 wt.%), and identified that formation of hydrotalcite in dolomite-containing systems has a positive
712 effect on the chloride binding capacity, to a similar extent as the Friedel's salt formed in systems
713 containing limestone. It has also been observed [180] that hydrotalcite forming in dolomite-metakaolin-
714 portland cement systems, withstands leaching and carbonation, which can potentially be beneficial for
715 the durability of the resulting concrete.

716

717 In a recent study [47] evaluating the phase assemblage evolution of calcium and magnesium carbonate-
718 metakaolin-portland cements, it was identified that these carbonates have a comparable effect on
719 hydration, as both minerals used in that study contained calcite. The amount of calcite present was

720 sufficient to promote hemicarbonates formation, and significant differences in pore refinement and
721 distribution of portlandite and silicates were observed in the carbonate-containing specimens,
722 compared with reference samples containing quartz. This indicates changes in the pozzolanic reaction of
723 metakaolin in presence of carbonates, which might impact performance evolution of these composite
724 cements.

725

726 Other ternary systems that have attracted significant interest are blends of portland cement containing
727 blast furnace slag and limestone, and recent studies have been focused on determining factors
728 influencing their phase assemblage evolution [181] and their impact in performance of these materials
729 [182]. Proske et al. [183] identified for these systems comparable carbonation rates to those reported
730 for CEM III/A, a good resistance against chloride migration, and a significant decrease in drying shrinkage
731 and creep deformation compared with concrete without SCMs. However, a higher susceptibility to
732 damage due to freeze-thaw was observed. The freeze-thaw performance significantly improved when
733 the water content of these composite concrete was reduced. Similar results were observed by Zhang et
734 al. [184], who identified a reduction in the carbonation and freeze-thaw resistance of blends with higher
735 volumes of slag replacement, although addition of limestone slightly enhanced the performance of
736 these materials. In pre-carbonated specimens exposed to freeze-thaw cycles, a reduced scaling
737 resistance and higher internal damage was identified compared with non-carbonated specimens
738 [184,185].

739

740 Durability of portland cement-SCM-limestone based concrete requires further investigation, so that
741 mitigation strategies can be adopted to enhance their performance particularly against carbonation and
742 freeze-thaw.

743

744 **4.2 Durability of Concrete Containing By-Product SCMs**

745

746 The recent reviews by Mo et al. [186] and Martirena et al. [187] provide a state-of- the-art overview
747 about concrete produced with SCMs derived from farming waste residues, including ashes from
748 bamboo, wheat, banana leaf, and elephant grass, among others. It is evident from these reviews that
749 there is little or no systematic understanding about the effects of these potential SCMs on the durability
750 of concrete containing them. This is consistent with their few practical applications as SCMs.

751

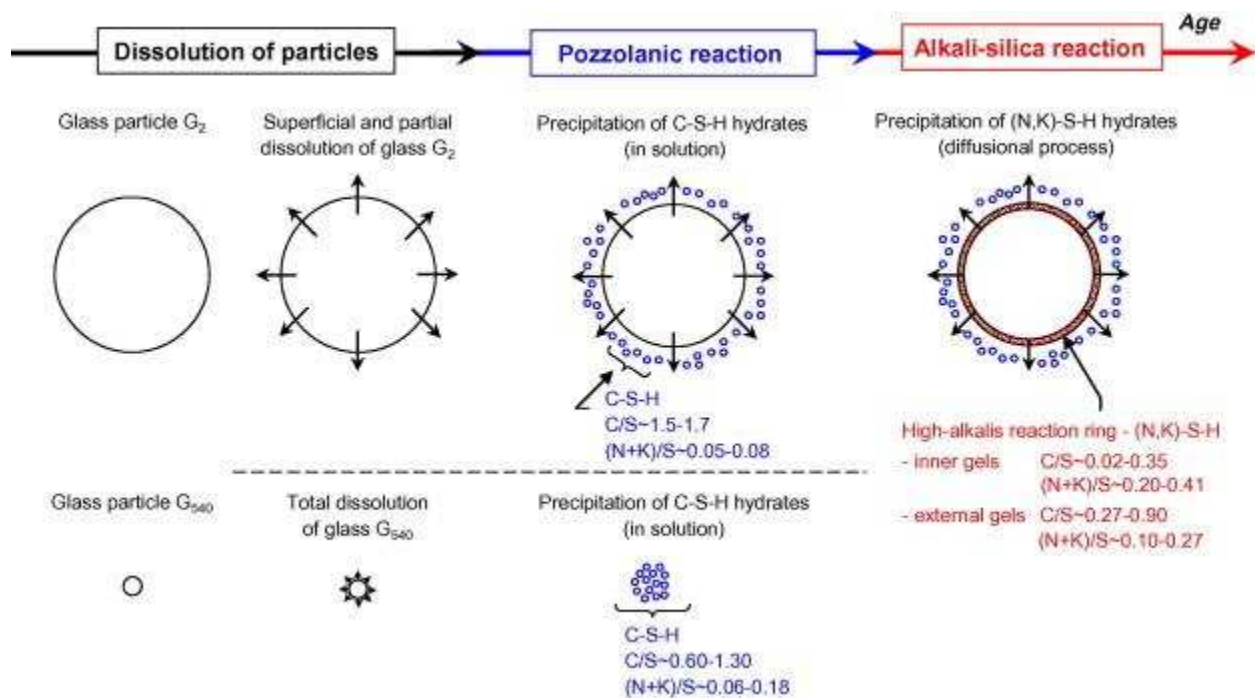
752 However, some promising results have been published. For example, Bahurudeen et al. [188] identified
753 that the addition of sugar cane bagasse ash in concrete increased the resistance to chloride ingress and
754 gas penetration in concrete. This positive effect has also been observed in ultra-high strength concrete
755 (160 MPa) with up to 15 wt.% replacement of cement by sugar cane bagasse ash, consistent with
756 significant reduction of permeability, particularly when high temperature (90°C) steam curing is applied
757 during the first 24 h after mixing, although steam curing would usually lead to a more porous structure
758 [189]. An improved resistance to alkali-silica reaction in materials containing sugar cane bagasse ash has
759 also been identified [190].

760

761 Czarnecki et al. [191] evaluated the carbonation resistance of concrete produced with different contents
762 of a calcareous fly ash from non-standard circulating fluidized-bed combustion (FFACa) from coal
763 combustion in a circulating fluidized bed boiler. Using a lower water/binder ratio reduced the
764 carbonation susceptibility of the concrete, so that concrete designed with a water/binder of 0.45 and
765 15% of FFACa presented comparable carbonation depth to concrete without SCMs.

766

767 Paul et al. [192] published a recent review on materials containing waste recycled glass and showed that
 768 most durability studies of concrete containing this SCM have been centered on alkali-silica reaction
 769 (ASR), which is strongly dependent on the particle size of the waste glass used, so that the pozzolanic
 770 reaction can prevail over ASR for finely ground glass cullet (Figure 9). In most studies, a reduced
 771 expansion has been reported in materials blended with powdered waste glass. A low water absorption,
 772 along with a high resistance to freeze-thaw and low chloride ingress, have also been reported for
 773 concrete with both powdered and sludge waste glass.
 774



775
 776 **Figure 9.** Successive reactions of coarse and fine glass particles. Reprinted from [193] with permission
 777 from Elsevier.

778
 779 **4.3 General remarks about Durability Testing of Concrete with New SCMs**

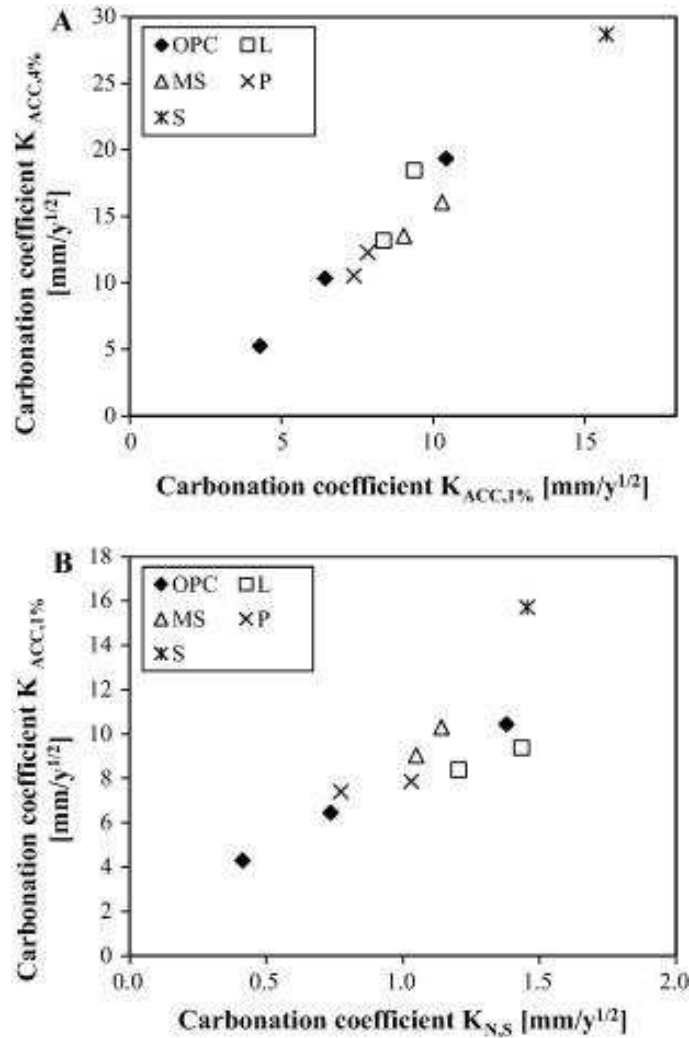
780

781 Although new sources of SCMs have been identified, and, in some cases, excellent durability
782 performance has been reported, there are some barriers limiting their immediate uptake, including the
783 question of whether standardized testing methodologies are applicable to evaluate durability
784 performance under accelerated conditions. For instance, Auroy et al. [194] in a recent study concluded
785 that, considering the changes in phase assemblage induced when cements blended with slag or fly ash
786 are exposed to 3% CO₂ or 0.04 % CO₂, accelerated carbonation testing at 3% CO₂ is sufficiently
787 representative of long-term carbonation when adding these SCMs. However, Leemann et al. [195]
788 identified significant differences in carbonation coefficients determined under different exposure
789 conditions for mortars and concrete containing slag and silica fume, which underperform when tested
790 under accelerated carbonation conditions (1% and 4% CO₂) compared with field observations (Figure
791 10). This is mainly a consequence of the level of saturation of the concrete, so that at a moderate
792 relative humidity (RH) (57% RH) no significant influence of CO₂ concentration has been identified
793 regarding the carbonation ranking of concrete produced with nine different portland blended cement
794 types [196]. This is not the case when accelerated carbonation of concrete is induced at 80% RH due to
795 an increased volume of pores filled with water. Consequently, an increase of RH in accelerated testing
796 does not improve the transferability of the results to natural carbonation. Such observations are of
797 particular importance as concrete with SCMs might perform significantly better in conditions inducing
798 rebar corrosion, than indicated by accelerated carbonation tests. These discrepancies highlight the fact
799 that phase assemblage, pore solution chemistry, and transport properties play a significant role in how
800 degradation mechanisms proceed. Hence, to determine durability of concrete, such factors need to be
801 coupled with reactive transport models so meaningful results from accelerated tests can be obtained.
802 There have been significant advancements in the application of thermodynamic modeling of phase
803 assemblages to the understanding of durability in SCM-blended materials when exposed to sulfates
804 [197], carbonation [147], and chlorides [198], and in linking these models to reactive transport

805 simulations [199–201], However, this remains an area in need of further development and insight for
 806 concrete produced with conventional SCMs and more so for those with new SCMs.

807

808



809

810

811 **Figure 10.** (A) Carbonation coefficient K_{ACC} of mortars at 4% CO₂ versus carbonation coefficient K_{ACC} at
 812 1% CO₂ ($R^2 = 0.96$). (B) Carbonation coefficient K_{ACC} at 1% CO₂ versus carbonation coefficient K_{NS} in
 813 sheltered, outdoor exposure ($R^2 = 0.69$). OPC correspond to mortars based on CEM-I, L mortars with

814 added limestone, *MS* with added microsilicate, *P* with added portlandite, and *S* mortars based on CEM
815 III/B. Reprinted from [195] with permission from Elsevier.

816

817 The approach for assessing durability of concrete with new sources of SCMs has mainly centered on
818 developing parametric studies for given formulations at specific exposure conditions, without much
819 insight explaining the mechanisms governing the performance of those materials considering their
820 chemistry and/or transport properties. Hence, it is difficult to transfer the decades of knowledge gained
821 on durability performance of concrete with conventional SCMs. In order to elucidate the mechanisms
822 leading to different types of degradation in concrete with SCMs of any nature, a deeper understanding
823 of the chemical and transport phenomena taking place at different scales needs to be developed. This
824 will enable the creation and adoption of the suitable testing methodologies for the reliable assessment
825 of durability performance of concrete with alternative SCMs, which will underpin the development of
826 more accurate models for their long-term performance prediction.

827

828 Considering the diversity of new sources of SCMs, the most tractable approach is probably to develop
829 and validate the core models for materials containing conventional SCMs, with a focus on relating
830 performance to hydrate assemblage and pore structure. The application of such models to concrete
831 with new sources of SCMs will then be much more straightforward on the basis of readily-determined
832 material parameters, e.g. the nature and fractions of the hydrate phases formed and the pore geometry
833 and pore fluid chemistry, rather than fully re-building a model specific to each different SCM.

834

835 The lack of in-service history to validate the observations made from accelerated durability tests is
836 another challenge for the uptake of new SCMs. The prediction of the long-term performance and
837 durability of modern and future concrete containing new or alternative SCMs, within time-scales

838 relevant to their immediate application, is one of the main scientific challenges that needs to be
839 resolved to bring the necessary confidence for their use in practice. Hence, for blended cements
840 containing new or alternative SCMs, whose phase assemblage evolution and pore structure fall within
841 well-established trends for the known historical record of conventional SCMs, it is sensible to think that
842 they will perform at least equally to those materials that have been used for several decades.

843

844 **5. Insights, Recommendations, and Conclusions**

845

846 SCMs have been used in concrete mixtures for decades to improve performance such as increased long-
847 term compressive strength, enhanced long-term durability, reduced cost, and reduced environmental
848 impact. The cement and concrete industries have become reliant on these materials, particularly on
849 those with low cost and high availability from industrial waste streams. However, as demand for
850 conventional SCMs increases and supply decreases, there is a great need to find new sources of
851 materials that provide comparable or superior properties to the highly utilized fly ash and slag sources.
852 At the same time, it is important to consider maximizing resource efficiency and sustainability, making
853 use of wastes from different industries that currently have little or no commercial value.

854

855 It is clear that research into new sources of SCMs, their reactivity, and their impact on concrete
856 properties has been increasing in recent years and has great likelihood to further increase as the
857 demand for these materials grows. Ideally, we could directly translate the extensive body of knowledge
858 on commonly-used SCMs to new material sources, but it is becoming apparent that new research is
859 necessary, especially in some areas. Despite the great technical potential of some of the new SCMs,
860 there are some barriers limiting their uptake such as reduced reactivity, low production speed, scarcity
861 of resources, and more quality control needs, compared to conventional SCMs. More studies are

862 required to identify potential solutions to these barriers, as well as to quantify the environmental and
863 economic savings that utilization of new SCMs can offer. This will underpin changes in policy so that
864 incentives for their use can be implemented in future infrastructure projects. The inclusion of new SCMs
865 in existing or new standards is imperative to enable their widespread use in the years to come. This
866 might be achieved more promptly for natural SCMs such as pumices and calcined clays, due to their
867 homogeneity and known chemistry.

868

869 The implementation of some SCMs is contingent on a better understanding of their reactivity,
870 particularly in the case of impure calcined clays. An improved understanding of the reactivity of the
871 minerals in these complex, heterogeneous materials is needed. Likewise, when calcined clays and other
872 SCMs are combined with limestone and cement, the ensuing reactions, microstructural development,
873 and their impacts on durability need further investigation. These so-called LC³ and similar systems hold
874 great promise as low-cost, high-availability SCMs, and further research will facilitate their
875 implementation. Some new SCMs derived from wastes may potentially contain heavy metals (e.g.
876 municipal solid waste bottom ashes or some non-ferrous slags) that might limit their applicability as
877 infrastructure materials. It is then important that more studies are focused on understanding the
878 potential mobility of those species within concrete to ensure the safe use of new SCMs.

879

880 Alongside the exploration of new materials is the need for new standardized tests and standards
881 governing their use. Existing test methods for pozzolanic reactivity have repeatedly been found to be
882 either ineffective at predicting performance or lack robustness. New test methods are being developed,
883 and we expect to see these find their way into practice in the coming years. Likewise, methods to
884 characterize the impact of SCMs on durability, such as resistance to carbonation, need to be revisited
885 and revised in the context of new materials and systems. Further research on test methods, for SCM

886 characterization and for concrete durability, will facilitate the development of new standards allowing
887 for a greater diversity of allowed SCMs in our evolving concrete mixtures.

888

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890

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896

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